


ESTIMATE OF QUANTITIES

| BID ITEM NUMBER | ITEM | QUANTITY | UNIT |
| :---: | :---: | :---: | :---: |
| O09E0010 | Mobilization | Lump Sum | LS |
| 110 E7700 | Remove Drop Inlet Frame and Grate Assembly for Reset | 9 | Each |
| 320E0402 | Asphalt Repair Mastic Type 2 | 137,794 | Lb |
| 320E1200 | Asphalt Concrete Composite | 300.0 | Ton |
| 380E5030 | Nonreinforced PCC Pavement Repair | 8,496.7 | SqYd |
| 380E6000 | Dowel Bar | 9,556 | Each |
| 380E6110 | Insert Steel Bar in PCC Pavement | 18,623 | Each |
| 380E6200 | Tie Bar Retrofit, Stitching | 335 | Each |
| 380E6302 | Reseal PCC Pavement Joint - Hot Pour | 123,887 | Ft |
| 380E6310 | Seal Random Cracks in PCC Pavement | 1,239 | Ft |
| 380E6510 | Grinding PCC Pavement | 744.4 | SqYd |
| 390E0200 | Repair Type A Spall | 22.0 | SqFt |
| 410E2600 | Membrane Sealant Expansion Joint | 52.0 | Ft |
| 460E0700 | Joint Nosing Material | 10 | SqFt |
| 633E0225 | Preformed Thermoplastic Pavement Marking, 24" | 816 | Ft |
| 633E0235 | Preformed Thermoplastic Pavement Marking, Arrow | 29 | Each |
| 633E0240 | Preformed Thermoplastic Pavement Marking, Combination Arrow | 1 | Each |
| 633E1200 | High Build Waterborne Pavement Marking Paint, White | 155 | Gal |
| 633E1205 | High Build Waterborne Pavement Marking Paint, Yellow | 93 | Gal |
| 633E5015 | Grooving for Cold Applied Plastic Pavement Marking, 24" | 816 | Ft |
| 633E5025 | Grooving for Cold Applied Plastic Pavement Marking, Arrow | 29 | Each |
| 633E5030 | Grooving for Cold Applied Plastic Pavement Marking, Combination Arrow | 1 | Each |
| 634E0010 | Flagging | 500.0 | Hour |
| 634E0020 | Pilot Car | 250.0 | Hour |
| 634E0110 | Traffic Control Signs | 3,221.8 | SqFt |
| 634E0120 | Traffic Control, Miscellaneous | Lump Sum | LS |
| 634E0275 | Type 3 Barricade | 32 | Each |
| $634 E 0330$ | Temporary Raised Pavement Markers | 21,500 | Ft |
| 634E0420 | Type C Advance Warning Arrow Board | 12 | Each |
| 634E0600 | 4" Temporary Pavement Marking Tape Type I | 2,544 | Ft |
| 634E0900 | Portable Temporary Traffic Control Signal | 2 | Unit |
| 634E1215 | Contractor Furnished Portable Changeable Message Sign | 6 | Each |
| 650E9000 | Repair Concrete Curb and/or Gutter | 255 | Ft |
| 670E7000 | Reset Drop Inlet Frame and Grate Assembly | 9 | Each |

## SPECIFICATIONS

Standard Specifications for Roads and Bridges, 2015 Edition and Required Provisions, Supplemental Specifications and Special Provisions as included in the Proposal

## ENVIRONMENTAL COMMITMENTS

The SDDOT is committed to protecting the environment and uses Environmental Commitments as a communication tool for the Engineer and Contractor to ensure that attention is given to avoid, minimize, and/or mitigate an environmental impact Environmental commitments to various agencies and the public have been mad to secure approval of this project. An agency with permitting authority can delay project if identified environmental impacts have not been adequately addressed. Uness otherwise designated, the Contractor's primary contact regarding matters associated with these commitments will be the Project Engineer. During Environmental Commitment rgirents. Thi Lhat the Contractor has mat not subject to change without prior written approval from the SDDOT Environmental Office.

Additional guidance on SDDOT's Environmental Commitments can be accessed through the Environmental Procedures Manual found
<https://dot.sd.gov/media/documents/EnvironmentalProceduresManual.pdf >
For questions regarding change orders in the field that may have an effect on an Environmental Commitment, the Project Engineer will contact the Environmental Engineer at 605-773-3180 or 605-773-4336 to determine whether an environmental analysis and/or resource agency coordination is necessary.
Once construction is complete, the Project Engineer will review all environmental commitments for the project and document their completion.

## COMMITMENT E: STORM WATER

Construction activities constitute less than 1 acre of disturbance

## Action Taken/Required:

At a minimum and regardless of project size, appropriate erosion and sedimen control measures must be installed to control the discharge of pollutants from the

## COMMITMENT H: WASTE DISPOSAL SITE

The Contractor will furnish a site(s) for the disposal of construction and/or demolition debris generated by this project.

## Action Taken/Required:

Construction and/or demolition debris may be disposed of within the Public ROW
The waste disposal site(s) will be managed and reclaimed in accordance with the following from the General Permit for Construction/Demolition Debris Disposal of Agriculture and Natural Resources. Agiculure and Natral Resource.

The waste disposal site(s) will not be located in a wetland, within 200 feet of surface water, or in an area that adversely affects wildlife, recreation, aesthetic value of an area, or any threatened or endangered species, as approved by the Environmental Office and the Project Engineer.

The waste disposal site(s) is located such that it is within view of any ROW, the wing additional requirements will apply

1. Construction and/or demolition debris consisting of concrete, asphalt concrete, or other similar material will be buried in a trench separate from will consist of a minimum of 1 for of soil capable of supporting vegetation Waste disposal sites provided outside of the Public ROW will be seeded in accordance with Natural Resources Conservation Service recommendations. The seeding recommendations may be obtained through the appropriate County NRCS Office. The Contractor will control the access to waste disposal sites not within the Public ROW with fences, gates, and placement of a sign or signs at the entrance to the site stating, No Dumping Allowed.
2. Concrete and asphalt concrete debris may be stockpiled within view of the ROW for a period not to exceed the duration of the project. Prior to projec , he waste disposal sit reclaind as nobe.

The above requirements will not apply to waste disposal sites that are covered by an individual solid waste permit as specified in SDCL 34A-6-58, SDCL 34A-6-1.13 and ARSD 74:27:10:06

Failure to comply with the requirements stated above may result in civil penalties in accordance with South Dakota Solid Waste Law, SDCL 34A-6-1.31.

Cost associated with furnishing waste disposal site(s), disposing of waste, maintaining control of access (fence, gates and signs), and reclamation of the waste disposal site(s) will be incidental to the various contract items

## COMMITMENT I: HISTORIC PRESERVATION OFFICE CLEARANCES

The SDDOT has obtained concurrence with the State Historic Preservation Office (SHPO or THPO) for all work included within the project limits and all department sources and designated option material sources, stockpile sites, storage areas, and waste sites provided within the plans.

## Action Taken/Required:

All earth disturbing activities not designated within the plans require a cultural esource review prior to scheduling the pre-construction meeting. This work processing sites, stockpile sites, storage areas, plant sites, and waste areas.

The Contractor will arrange and pay for a record search and when necessary, a cultural resource survey. The Contractor has the option to contact the state Archaeological Research Center (ARC) at 605-394-1936 or another qualified record search might bether a records search or a cultural resourcusly surveyed; however, a cultural resources survey may need to be conducted by a qualified archaeologist.
The Contractor will provide ARC with the following: a topographical map or aerial view in which the site is clearly outlined, site dimensions, project number, and PCN If applicable, provide evidence that the site has been ivities with a landowner .
he Contractor will submit the cultural resources survey report to SDDOT Environmental Office, 700 East Broadway Avenue, Pierre, SD 57501-2586. SDDOT will submit the information to the appropriate SHPO/THPO. Allow 30 Days from the date this information is submitted to the Environmental Engineer for SHPO/THPO review

In the event of an inadvertent discovery of human remains, funerary objects, or if vidence of cultural resources is identified during project construction activities,


Project Engine will cont SDOT Environmental Office, who will cod Prpropriate SHPO/THPO within 48 hours of the discovery to determine an appropriate course of action

SHPO/THPO review does not relieve the Contractor of the responsibility for obtaining any additional permits and clearances for Contractor furnished material sources, material processing stes, stockpile sites, storage areas, plant stes, and waste areas that affect wetlands, threatened and endangered species, or waterways. The Contractor will not utilize a site known or suspected of having contaminated soil or water. The Contractor will provide the required permits and clearances to the Project Engineer at the preconstruction meeting

## SCOPE OF WORK

This project consists of full depth replacement of Nonreinforced Concrete Pavement (NRCP) in areas where concrete pavement blowups or major failures have occurred. Joints will be sawed and sealed where sealant has failed.

## COORDINATION BETWEEN CONTRACTORS

A separate contract for Project IM 0909(91)394-PCN 065D has been awarded to another Contractor for bridge repair over 190 located 2.1 miles west of 129 .

In addition, another separate contract for Project IM-B 2291(01)10-PCN 01QA has been awarded to another Contractor for bridge replacement on $60^{\text {th }} \mathrm{St}$. N. over 1229.

The Contractor will schedule work so as not to interfere with or hinder the progress of the work performed by other Contractors on the projects listed above.

## UTILITIES

The Contractor will contact the involved utility companies through South Dakota One Call (1-800-781-7474) prior to starting work. It will be the responsibility of the Contractor to coordinate work with the utility owners to avoid damage to existing facilities.

Utilities are not planned to be affected on this project. If utilities are identified near the improvement area through the SD One Call process as required by South Dakota Codified Law 49-7A and Administrative Rule Article 20:25; the Contractor will contact the Project Engineer to determine if project changes are necessary to avoid utility impacts.

The Contractor will be aware that the existing utilities shown in the plans were surveyed prior to the design of this project and might have been relocated or replaced by a new utility facility prior to construction of this project, might be relocated or replaced by a new utility facility during the construction of this project or might not require adjustment and may remain in its current location. The new utility facilities. The each utility owner and confirm the status of all existing and plans or bidding documents.

## EXISTING NON-REINFORCED CONCRETE (NRC) PAVEMENT

## SD38 in Hartford, SD

The existing pavement is $8^{\prime \prime}$ NRC PCC Pavement. Existing contraction joints are spaced at approximately $20^{\prime}$. Longitudinal joints are reinforced joints are reinforced with $11 / 4^{\prime \prime} \times 18^{\prime \prime}$ plain round dowel bars spaced $12^{\prime \prime}$ center to center. The aggregate in the existing NRC Pavement is quartzite.

SD38 over Skunk Creek
The existing pavement is 8 " NRC PCC Pavement. Existing contraction joints are spaced at approximately 15 '. Longitudinal joints are reinforced with No. $5 \times 30^{\prime \prime}$ deformed tie bars spaced $48^{\prime \prime}$ center to center. Transverse joints are reinforced with $11 / 4^{\prime \prime} \times 18^{\prime \prime}$ plain round dowel bars spaced $12^{\prime \prime}$ center to center. The aggregate in the existing NRC Pavement is quartzite.

## EXISTING NON-REINFORCED CONCRETE (NRC) PAVEMENT (CONTINUED)

SD38E/W at West $60^{\text {th }} \mathbf{S t} \mathbf{N}$ in Sioux Falls, SD
The existing pavement is 9" NRC PCC Pavement. Existing contraction joints are spaced at approximately $20^{\prime}$. Longitudinal joints are reinforced with No. $5 \times 30^{\prime \prime}$ deformed tie bars spaced $48^{\prime \prime}$ center to center. Transverse center to center. The aggregate in the existing NRC Pavement is quartzite

## 190E/W near Buffalo Ridge

The existing pavement is $11.5^{\prime \prime} \times 20^{\prime}$ NRC PCC Pavement. Existing contraction joints are spaced at approximately $20^{\prime}$. Longitudinal joints are Transverse joints are reinforced with $11 / 4$ x $18^{\prime \prime}$ plain round dowel bars spaced 12 " center to center. The aggregate in the existing NRC Pavement is quartzite.

129N near 190 interchange in Sioux Falls, SD The existing pavement is $11^{\prime \prime}$ NRC PCC Pavement. Existing contraction joints are spaced at approximately $20^{\prime}$. Longitudinal joints are reinforced with No. $5 \times 30^{\prime \prime}$ deformed tie bars spaced $48^{\prime \prime}$ center to center. Transverse
joints are reinforced with $11 / 4^{\prime \prime} \times 18^{\prime \prime}$ plain round dowel bars spaced $12^{\prime \prime}$ joints are reinforced with $11 / 4 \times 18^{\prime \prime}$ plain round dowel bars spaced $12^{\prime \prime}$
center to center. The aggregate in the existing NRC Pavement is quartzite.

## SD34 in Howard, SD

The existing pavement is 8 " NRC PCC Pavement. Existing contraction joints are spaced at approximately $20^{\prime}$. Longitudinal joints are reinforced with No. $5 \times 30^{\prime \prime}$ deformed tie bars spaced $48^{\prime \prime}$ center to center. Transverse joints are reinforced with $11 / 4^{\prime \prime} \times 18^{\prime \prime}$ plain round dowel bars spaced 12

1229S near 190 interchange in Sioux Falls, SD
The existing pavement is $12^{\prime \prime}$ NRC PCC Pavement. Existing contraction joints are spaced at approximately $20^{\prime}$. Longitudinal joints are reinforced with No. $5 \times 30^{\prime \prime}$ deformed tie bars spaced $48^{\prime \prime}$ center to center. Transverse joints are reinforced with $11 / 4^{\prime \prime} \times 18^{\prime \prime}$ plain round dowel bars spaced $12^{\prime \prime}$ center to center. The aggregate in the existing NRC Pavement is quartzite.

190E/W near 1229 interchange in Sioux Falls, SD The existing pavement is $11.5^{\prime \prime}$ NRC PCC Pavement. Existing contraction joints are spaced at approximately $15^{\prime}$. Longitudinal joints are reinforced with No. $5 \times 30^{\prime \prime}$ deformed tie bars spaced $48^{\prime \prime}$ center to center. Transverse
joints are reinforced with $1^{1 / 4^{\prime \prime}} \times 18^{\prime \prime}$ plain round dowel bars spaced $12^{\prime \prime}$ center to center. The aggregate in the existing NRC Pavement is quartzite.

NONREINFORCED PCC PAVEMENT REPAIR - GENERAL
NRC Pavement Repair will be done prior to Grinding PCC Pavement
New pavement thickness will equal existing pavement thickness ( $\mathrm{T}_{\mathrm{N}}=\mathrm{T}$ ).
Locations and size (length or width) of concrete repair areas are subject to change in the field, at the discretion of the Engineer, at no additional cost to the state. Payment will be based on actual area replaced.
Existing concrete pavement will be sawed full depth at the beginning and end of he NRCP repair areas. When either the beginning or end of a NRCP repair area位 Where possible, new working Saw custhater shrinkage mortar mix at the Contractor's expense.

Existing concrete pavement in the replacement areas will be removed by the lift out method or by means that minimize damage to the base and sides of remaining in place concrete. Removed material will be removed from within the right-of-way by the end of the workday. Damage to adjacent concrete caused by the Contractor's operations will be removed and replaced at the Contractor's expense

If the pavement replacement area is entirely on either side of the existing contraction joint, the location of one of the working joints will be at the original location. Any existing dowel bar assemblies/steel bars will be sawed off and removed

At full roadway width repairs and when specified, a working joint will be reconstructed at both ends of each pavement replacement area as shown in these plans.
Concrete placed adjacent to gravel and asphalt concrete shoulders will be formed full depth to match the width of existing concrete pavement. Asphalt concrete shoulders adjacent to concrete pavement replacements will be repaired with new

At repair locations where the new working joint is not opposite the existing working joint, the Contractor will place a $1 / 4^{\prime \prime}$ preformed asphalt expansion joint material along the longitudinal joint from the existing working joint to the new working joint. The expansion joint material will meet the requirements of AASHTO M33. Cost for Nonreinforced PCC Pavement Repai

The initial contraction joint sawing will be performed as soon as practical after placement to avoid random cracking.
Joints (longitudinal and transverse) through and around the repair areas will be sawed and sealed in accordance with the details shown in these plans. Refer to Saw and Seal Joints notes.

## SAW AND SEAL JOINTS (NRCP)

Longitudinal and transverse joints at concrete repair areas will be sawed and

Joint sealing will conform to Section 380.3 P
Longitudinal and transverse joints in urban and rural sections will be sealed with Hot Poured Elastic Joint Sealer

Hot Poured Elastic Joint Sealer will be based on visual inspection by the Engineer
Cost for sawing and sealing for both longitudinal construction and transverse joints will be incidental to the contract unit prices per square yard for Nonreinforced PCC Pavement Repair.

## NONREINFORCED PCC PAVEMENT REPAIR

Concrete will meet the requirements stated in Section 380 of the specifications, except as modified by the following notes:

The fine aggregate will be screened over a one-inch square-opening the Engineer.

The slump requirement will be limited to $3^{\prime \prime}$ maximum after water reduce is added and the concrete will contain $4.5 \%$ to $7.0 \%$ entrained air. The concrete will contain a minimum of $50 \%$ coarse aggregate by weight. Coarse aggregate will be crushed ledge rock, Size No. 1 unless an alternative gradation is approved by the Concrete Engineer as part of the mix design submittal. The mix design will contain between 650 and 800 28 -day compressive strength will be $4,000 \mathrm{psi}$. The Contractor esponsible for the mix design used

The Contractor will submit a mix design and supporting documentation for approval at least 2 weeks prior to use.

The use of a water reducer at manufacturer's recommended dosage will be required.
Concrete will be cured with white pigmented curing compound (AASHTO M148 Type 2) applied as soon as practical at a rate of 125 square feet per gallon. Concrete will be cured for a minimum of 48 hours before opening to trafic. The 48 the cure period. If the concrete temperature falls below $60^{\circ} \mathrm{F}$, the cure time will be extended, or other measures taken, at no additional cost to the State. A streng of 3,000 psi must be attained prior to opening to traffic.

Upon placement of the concrete, repair areas will be straight edged to ensure a smooth riding surface and will be textured longitudinally with the pavement by finishing with a stiff broom. Repair areas will then be checked with a 10 ' foot straight edge. The permissible longitudinal and transverse surface deviation will be $1 / 8^{\prime \prime}$ in 10

## NONREINFORCED PCC PAVEMENT REPAIR (CONTINUED)

Concrete will be covered with suitable insulation blanket consisting of a layer of closed cell polystyrene foam protected by at least one layer of plastic. Insulation blanket will have an $R$-value of at least 0.5 , as rated by the manufacturer. Insulation blanket will be left in place, except for joint sawing operations, until the $3,000 \mathrm{psi}$ is attained. Insulation blanket will be overlapped on to the existing concrete by 4 . during periods of hot weather upon approval of the Engineer
Cost for performing the aforementioned work including sawing and removing concrete, furnishing and placing concrete, sawing and sealing joints, repairing in the contract unit price per square yard for Nonreinforced PCC Pavement Repar.

## STEEL BAR INSERTION (NRCP)

Steel bars will conform to Section 1010.
Locations and quantities of concrete repair are subject to change in the field at the discretion of the Engineer. The Contractor will be responsible for ordering the actual quantity of steel bars necessary to complete the work

For existing pavement thickness greater than or equal to $10.5^{\prime \prime}$ ( $T>=10.5^{\prime \prime}$ ) The Contractor will insert the steel bars ( $11 / 2^{\prime \prime} \times 18^{\prime \prime}$ epoxy coated plain round dowel bars and No. $11 \times 18$ epoxy coated deformed tie bars for transverse joints and No. $5 \times 24$ epoxy coated deformed tie bars for longitudinal joints) into drilled holes in the existing concrete pavement. An epoxy resin adhesive must be used to anchor the steel bar in the drilled hole as per Section 380.3 C.1.
For existing pavement thickness greater than or equal to $8.5^{\prime \prime}$ and less than 10.5" ( $\gg=8.5^{"}$ and T < $10.5^{")}$ :
The Contractor will insert the steel bars ( $11^{\prime \prime} \times 18^{\prime \prime}$ epoxy coated plain round dowe bars and No. $9 \times 18^{\prime \prime}$ epoxy coated deformed tie bars for transverse joints and No $5 \times 24^{\prime \prime}$ epoxy coated deformed tie bars for longitudinal joints) into drilled holes in the existing concrete pavement. An epoxy resin adhesive must be used to anchor the steel bar in the drilled hole as per Section 380.3 C.1.

For existing pavement thickness less than $8.5^{\prime \prime}\left(T<8.5^{\prime \prime}\right)$.
The Contractor will insert the steel bars (1" x 18" epoxy coated plain round dowel bars and No. $8 \times 18$ " epoxy coated deformed tie bars for transverse
joints and No. $5 \times 24$ " epoxy coated deformed tie bars for longitudinal joints) into drilled holes in the existing concrete pavement. An epoxy resin adhesive must be used to anchor the steel bar in the drilled hole as per Section 380.3 C .1

Steel bars will be inserted in the transverse joint on 18 " centers. The first steel bar in the transverse joint will be placed 9 " from the edge of the slab closest to centerline. Steel bars will be inserted in the longitudinal joint on 30 " centers and will be a minimum of 15 " from either transverse joint. A typical one-lane patch ${ }^{12}$ wide and 6 long will require 18 steel bars ( 8 in each transverse joint and 2 in the longtudinal joint). It will be necessary to laterally adjust the location of some of the locations.

A rigid frame or mechanical device will be required to guide the drill to ensure proper horizontal and vertical alignment of the steal bars in the drilled holes

## TIE BAR RETROFIT, STITCHING

Drilling of holes and epoxy resin adhesive will conform to Section 380. Steel bars will conform to Section 1010.

Tie Bar Retrofit, Stitching will be done prior to Grinding PCC Pavement. If this sentence is applicable.

Tie Bar Retrofit, Stitching will be done on longitudinal joints and random cracks as marked out by the Engineer.
The Contractor will insert No. 5 epoxy coated deformed tie bars into drilled holes in the existing concrete pavement. An epoxy resin adhesive must be used to anchor the steel bar in the drilled hole. A rotary drill or other approved drill will be used that will not damage the concrete surface. The diameter of the disturbed surface from will be less than 2 inches. A rigid frame or mechanical device will be to guide the drill to ensure the proper angle of the steel bars in the drilled holes.

The diameter of the drilled holes in the existing concrete pavement for the steel bars will not be less than $1 / 8$ inch nor more than $3 / 8$ inch greater than the overall diameter of the steel bar. The holes will be drilled at an angle alternating from opposite sides of the joint to produce a cross-stitching pattern.
Fill the drilled holes sufficiently with epoxy prior to the insertion of the tie bar such that the epoxy will be level with the top of the concrete pavement after insertion of the tie bar. Rotate the steel bar during insertion to eliminate voids and ensure complete bonding of the bar. Insertion of the bars by the dipping method win not be allowed. The top of the drilled hole will be filled with epoxy or excess epoxy removed such that the epoxy is level with the existing pavement.

No bars will be inserted within 15 " of an existing transverse contraction joint. Any bars not functioning or damaged will be repaired or replaced at the Contractor's expense.
Cost for the epoxy resin adhesive, tie bars, drilling of holes, debris or loose material removal, applying the adhesive, inserting the tie bars into the drilled holes and cidentals necessary fie Retrofit, Stitching

## GRINDING PCC PAVEMENT

PCC Pavement Repair will be done prior to Grinding PCC Pavement. Grinding PCC Pavement will be done prior to sawing and sealing joints. Approximately 4' of grinding is expected on either side of the joint line in the tie bar retrofit stitching areas of SDerchange in Sioux Falls.

## RESEAL PCC PAVEMENT JOINT

Existing transverse and longitudinal joints will be cleaned and resealed for the full width of the joints with Hot Poured Elastic Joint Sealer; except on SD34, where the joints will be cleaned and resealed with Asphalt Repair Mastic Type 2.

Joints will not be sealed unless they are thoroughly clean and dry. Cleaning will be accomplished by sandblasting and other tools as necessary. Sand blasting of both sides of the vessel will be accomplished simultaneously with a mechanical device approved by the Engineer. Just prior to sealing, each joint will be blown out using a jet of compressed air to remove all traces of dust. Final joint width is to be kept joint will not be widened more than $1 / 8$ inch if sawing is utilized to prepare the join for sealant.

If sawing
the joint
In certain areas the joint may be wider than the original construction. It may be necessary to provide backer rod in the wide areas. Any additional cost to perform this work will be at no additional cost to the State. The Contractor will be responsible to verify joint widths prior to establishing the contract unit price.
It is not essential that all of the sealant be removed. Remaining sealant adhering to the sides may remain in place if the Engineer determines that it is not detrimental to the joint.

Cost for cleaning and resealing transverse and longitudinal joints will be included in the contract unit price per foot for Reseal PCC Pavement Joint - Hot Pour.

## SEAL RANDOM CRACKS IN PCC PAVEMENT

Random cracks will be repaired in accordance with the detail for Sealing Random Cracks. Reservoir dimensions may vary slightly from the details, due to the nature of this operation. However, any variance due to Contractor negligence will be repaired at the Contractor's expense

Only those random cracks in the existing concrete pavement that are open and accept water and incompressible material as selected
prepared and sealed with Hot Poured Elastic Joint Sealer
Prior to sealing, each random crack will be routed and thoroughly cleaned with compressed air or by other methods satisfactory to the Engineer. Routing will be performed with a saw designed for that purpose

Random cracks narrower than $1 / 2$ inch will be routed and sealed $1 / 2$ inch wide by $1 / 2$ inch deep

Random cracks wider than $1 / 2$ inch may require the placement of a backer rod prior to sealing. Use of backer rod should be limited to locations where, once placed the top of the backer rod will be a minimum of $21 / 4$ inches below the top surface of the pavement. The hot pour in cracks wider than $1 / 2$ " should be placed 2 inch thick with the final surface of the hot pour remaining recessed $1 / 4$ inch below the top surface of the pavement
Sealant will be placed in the routed reservoir with equipment and by methods that insure complete and uniform filling. Hot Poured Elastic Joint Sealer will be placed level with the driving surface of the concrete for cracks $1 / 2^{\prime \prime}$ or narrower. Any excess or overrun of sealant will be removed by the Contractor at no additional cost to the State.

## SEAL RANDOM CRACKS IN PCC PAVEMENT (CONTINUED)

Seal Random Cracks in PCC Pavement will be measured by the foot to the nearest 0.1 foot of random cracks sealed and accepted and will be paid for at the contract unit price per foot measured for payment. Payment will be full compensation for labor, equipment, material and incidentals required for crack routing, cleaning, funnishing and installng backer wod when nesesary, furnishing and placing sealant and removing routed and foreign material from the roadway.

## SD34 JOINT SEALING WITH ASPHALT REPAIR MASTIC TYPE 2

Pavement joints on SD34 specified for joint sealing will be cleaned and resealed with Asphalt Repair Mastic Type 2.

Pavement joints with be cleaned in accordance with the notes for RESEAL PCC PAVEMENT JOINT.
The Special Provision for Asphalt Concrete Crack Leveling will apply except that use of a compressed air heated lance will not be required for surface preparation, and references to placing mastic material on pavement surfaces (other than in the joint itself) do not apply.

## 5" SAW CUT OF NRC AT TERMINAL ANCHOR LOCATIONS

A 5 " opening saw cut is required in the NRC at these terminal anchor locations and may require a multi-step process if the existing sealant joint is fully compressed. If the existing joint is fully compressed, the Contractor will be required to cut the joint to a $3^{\prime \prime}$ opening across the right lane. As soon as possible and on the same day, the Contractor will be required to adjust traffic control to install a lane closure to provide a $3^{\prime \prime}$ opening saw cut across the left lane. Traffic will not be permitted across any expansion joint wider than 3 ". Installation of the 5 " joint membrane will not be permitted for 1 -week.

Upon completion of the 1 -week waiting period, the contractor will be required to set up traffic control in the right lane, resaw the joint to the required 5 " and install the new 5 " membrane joint sealant in the right lane. As soon as possible and on the same day, the Contractor will be required to adjust traffic control to install a lane closure to resaw the joint in the left lane to the required 5 " and install the new 5 " membrane joint sealant in the left lane

See the traffic control details in the plans for lane closure details for this work


## REPAIR TYPE A SPALLS

Spall repair work will be done prior to Grinding PCC Pavement
Concrete Patch Material will be Type III conforming to Section 390.2 B.3. SF
As an alternative, the Contractor may remove concrete by milling, provided it produces results similar to the sawing and chipping process described in the Specifications
It is anticipated that a number of locations scheduled for Type A Spall Repair will have deteriorated to the point of needing full depth repair. Additional Quantities have deteriorated to the point of needing full depth repair. Additional Quantities will determine these locations on construction.

Spalls which are repaired according to plans and specifications and exhibit partial respalling or cracking, will be repaired to the satisfaction of the Engineer at no additional cost to the State.

## NOSING MATERIAL FOR CONCRETE REPAIR

A quantity of Nosing Material has been set up for use if spalling occurs, an existing crack is too close to the new saw cut, or a crack intersects the new saw cut for the Membrane Sealant Expansion Joint. The following quantity of 35 square feet is and 0.3 feet wide at full 11.5 " depth Actual depth may vary from full 11.5 " depth to 2" partial depth spalls. The Engineer will determine if and where this nosing material is to be used

The nosing material used must be one of types from the approved product list for Nosing Material. The nosing material will be furnished from one source and must be installed in accordance with the manufacturer's recommendations as approved by the Engineer.

The nosing material will be measured to the nearest 0.1 square foot. The Engineer will make measurements on the driving surface to the nearest 0.1 foot. Joint nosing material repairs will be paid for at the contract unit price per nearest square foot equipment tools and any incidentals necessary to prop furnish and install the nosing material will be incidental to the contract unit price per square foot for Joint Nosing Material.

## REPAIR CONCRETE CURB AND/OR GUTTER

The existing concrete curb and gutter is Type B68 in Hartford, SD. The existing curb and gutter will match in place.
Refer to the repair tables and details for locations of removal and replacement. These locations will be designated by the Engineer during construction.

If the end of any section to be removed does not fall on an existing joint, a sawed joint (4" deep) must be made to provide a vertical face for the new joint.

Existing foundation material will be shaped and compacted to a firm, uniform bearing surface, conforming to the existing section or established grades as set by the Engineer

Unsuitable foundation material will be removed and replaced as directed. Gravel cushion material will be furnished by the Contractor.
Cost for labor, equipment, material, and incidentals required for excavation and providing cushion material will be incidental to the contract unit prices for the various items
Curb and Gutter will be tied to existing PCC pavement with drilled in No. $5 \times 24^{\prime \prime}$ epoxy coated deformed tie bars spaced 30 " center to center or by salvaged in place tie bars. Also, two No. $5 \times 24^{\prime \prime}$ epoxy coated deformed tie bar will be drilled into the for STEEL BAR INSERTION.
Cost for this work will be included in the contract unit price per each for Insert Steel Bar in PCC Pavement
The Contractor will satisfactorily restore disturbed areas adjacent to the new concrete placement to the satisfaction of the Engineer. Cost for this restoration work will be incidental to the contract unit prices for the various items.

Standard specifications for sawing, removing and replacing concrete curb and/or gutter, and material composition will apply except that the cost for such will be included in the contract unit price per foot for Repair Concrete Curb and/or Gutter


## REMOVE DROP INLET FRAME AND GRATE ASSEMBLY FOR RESET

The Contractor will reset drop inlet frame and grates on drop inlets that are in place on the $60^{\text {th }} \mathrm{St} \mathrm{N}$ location along SD38. The elevations of the frame and grate will be flush with the adjacent concrete at each location.

| Hwy | DMI | L/R | Class <br> Concrete (CuYd) | Quantity |
| :--- | :--- | :--- | :--- | :--- |
| SD38E | 364.68 | R | 0.05 | 1 |
| SD38E | 364.64 | R | 0.05 | 1 |
| SD33E | 364.55 | R | 0.05 | 1 |
| SD38W | 364.72 | R | 0.05 | 1 |
| SD38W | 364.68 | R | 0.05 | 1 |
| SD38W | 364.64 | R | 0.05 | 1 |
| SD38W | 364.52 | R | 0.05 | 1 |
| SD38W | 364.50 | R | 0.05 | 1 |
| SD38W | 364.48 | R | 0.05 | 1 |
| Grand Totals |  | 0.45 | 9 |  |

*The quantity of Class M6 Concrete will be incidental to the contract unit price per each for Reset Drop Inlet Frame and Grate Assembly

## ASPHALT CONCRETE COMPOSITE

If damage to the shoulder occurs from driver behavior, not to the fault of the Contractor as determined by the Engineer, the damaged shoulder areas will be repaired with Asphalt Concrete Composite. Removal of damaged asphalt on the shoulders prior to placement will be incidental to the contract unit price for Asphalt Concrete Composite. The Asphalt Concrete Composite item is not intended for repairs/fill adjacent to repair areas due to forms or Contractor operations

Asphalt concrete composite will be placed at a thickness of 3 " and will not require asphalt for flush seal or sand for flush seal.

## TABLE FOR SUMMARY OF PCC REPAIRS

|  |  |  |  |  |  |  |  | NRCP | REPAIR | INSERT | INSERT | REPAIR | TIE BAR |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
|  |  |  |  |  |  |  |  | REPAIR | CONCRETE | STEEL BAR | DOWEL | TYPE A SPALL | RETROFIT |
|  | BEGIN |  | END |  | EXCEPTION |  |  |  | CURB/GUTTER | IN NRCP |  | IN NRCP | STITCHING |
| HWY | MRM | DISP | MRM | DISP | LENGTH | Beg DMI | End DMI | (SqYd) | (Ft) | (Each) | (Each) | (SqFt) | (Each) |
| SD38 | 356.00 | 0.135 | 356.69 | 0.018 | 0 | 356.135 | 356.708 | 2017.6 | 255 | 4325 | 2336 | 22 |  |
| SD38 | 360.00 | 0.138 | 360.22 | 0.114 | 0 | 360.138 | 360.334 | 170.4 |  | 358 | 190 |  | 70 |
| SD38E | 364.26 | 0.116 | 364.75 | 0.000 | 0 | 364.376 | 364.750 | 512.6 |  | 1068 | 614 |  |  |
| SD38W | 364.26 | 0.116 | 364.75 | 0.000 | 0 | 364.376 | 364.750 | 1160.5 |  | 2419 | 1390 |  |  |
| 190E | 390.37 | 0.041 | 397.00 | 0.252 | 0 | 390.411 | 397.252 | 60.4 |  | 186 | 109 |  | 77 |
| 190W | 390.29 | 0.136 | 397.00 | 0.252 | 0 | 390.426 | 397.252 | 397.3 |  | 1220 | 717 |  |  |
| 129 N | 83.70 | 0.096 | 85.35 | 0.116 | 0 | 83.796 | 85.466 | 129.9 |  | 401 | 194 |  |  |
| SD34 | 364.00 | 0.987 | 366.37 | 0.012 | 0 | 364.987 | 366.382 | 2589.8 |  | 5191 | 2846 |  |  |
| 12295 | 8.28 | 0.584 | 10.84 | 0.000 | 0 | 8.864 | 10.840 | 732.5 |  | 1297 | 540 |  | 188 |
| 190E | 401.00 | 0.464 | 401.61 | 0.143 | 0 | 401.464 | 401.753 | 139.9 |  | 170 | 100 |  |  |
| I90W | 401.00 | 0.468 | 401.61 | 0.142 | 0 | 401.468 | 401.752 | 585.8 |  | 1988 | 520 |  |  |
|  |  |  |  |  | GRAND TOTALS |  |  | 8,496.7 | 255 | 18,623 | 9,556 | 22 | 335 |

TABLE FOR SUMMARY OF GRINDING PCC PAVEMENT

|  |  |  |  |  |  |  |  |  |  |  | GRINDING |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
|  |  |  |  |  |  |  | EXCEPTION | WIDTH OF | LENGTH OF | GENERAL | PCC |
|  | BEGIN |  | END |  |  |  | LENGTH | GRINDING | GRINDING | AREAS | PAVEMENT |
| HWY | MRM | DISP | MRM | DISP | Beg DMI | End DMI | (Ft) | (Ft) | (Ft) |  | (SqYd) |
| SD38 | 360.00 | 0.138 | 360.22 | 0.114 | 360.138 | 360.334 | 859.9 | 8 | 175.0 | Tie Bar Retrofit Stitching | 155.6 |
| I90E | 390.37 | 0.041 | 397.00 | 0.252 | 390.411 | 397.252 | 36,024.2 | 8 | 96.3 | Tie Bar Retrofit Stitching | 85.6 |
| 190W | 390.29 | 0.136 | 397.00 | 0.252 | 390.426 | 397.252 | 35,945.0 | 8 | 96.3 | Tie Bar Retrofit Stitching | 85.6 |
| I229S | 8.28 | 0.584 | 10.84 | 0.000 | 8.864 | 10.840 | 9,963.3 | 8 | 470.0 | Tie Bar Retrofit Stitching | 417.8 |
|  |  |  |  |  |  |  |  |  | GRAND TOTALS |  | 744.4 |

TABLE FOR JOINT RESEALING

|  |  |  |  |  |  |  |  | RESEAL | RESEAL | RESEAL | RESEAL |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
|  |  |  |  |  |  |  |  | LONGITUDINAL | LONGITUDINAL | TRANSVERSE | TRANSVERSE |
|  | BEGIN |  | END |  | EXCEPTION |  |  | JOINTS | JOINTS | JOINTS | JOINTS |
| HWY | MRM | DISP | MRM | DISP | LENGTH | Beg DMI | End DMI | (Ft) | TYPE | (Ft) | TYPE |
| SD38 | 356.00 | 0.135 | 356.69 | 0.018 | 0 | 356.135 | 356.708 | 5,446 | Hot Pour | 14,704 | Hot Pour |
| SD38 | 360.00 | 0.138 | 360.22 | 0.114 | 0 | 360.138 | 360.334 | 310 | Hot Pour | 621 | Hot Pour |
| SD38E | 364.26 | 0.116 | 364.75 | 0.000 | 0 | 364.376 | 364.750 | 1,580 | Hot Pour | 3,318 | Hot Pour |
| SD38W | 364.26 | 0.116 | 364.75 | 0.000 | 0 | 364.376 | 364.750 | 1,975 | Hot Pour | 4,147 | Hot Pour |
| 190E | 390.37 | 0.041 | 397.00 | 0.252 | 0 | 390.411 | 397.252 | 3,612 | Hot Pour | 4,696 | Hot Pour |
| I90W | 390.29 | 0.136 | 397.00 | 0.252 | 0 | 390.426 | 397.252 | 3,604 | Hot Pour | 4,685 | Hot Pour |
| 129N | 83.70 | 0.096 | 85.35 | 0.116 | 0 | 83.796 | 85.466 | 882 | Hot Pour | 1,146 | Hot Pour |
| SD34 | 364.00 | 0.987 | 366.37 | 0.012 | 0 | 364.987 | 366.382 | 22,097 | Hot Pour | 40,879 | Hot Pour |
| 12295 | 8.28 | 0.584 | 10.84 | 0.000 | 0 | 8.864 | 10.840 | 3,130 | Hot Pour | 5,947 | Hot Pour |
| 190E | 401.00 | 0.464 | 401.61 | 0.143 | 0 | 401.464 | 401.753 | 153 | Hot Pour | 407 | Hot Pour |
| 190W | 401.00 | 0.468 | 401.61 | 0.142 | 0 | 401.468 | 401.752 | 150 | Hot Pour | 400 | Hot Pour |
|  |  |  |  |  |  | GRAND | tals | 42,938 |  | 80,949 |  |
|  |  |  |  |  |  |  |  | RESEAL | RESEAL |  |  |
|  |  |  |  |  |  |  |  | LONGITUDINAL | LONGITUDINAL |  |  |
|  | BEGIN |  | END |  | EXCEPTION |  |  | JOINTS | JOINTS |  |  |
| HWY | MRM | DISP | MRM | DISP | LENGTH | Beg DMI | End DMI | (lb) | TYPE | DESCRIPTION |  |
| SD34 | 364.00 | 0.987 | 366.37 | 0.012 | 0 | 364.987 | 366.382 | 137,794 | Type II Mastic | Centerline Joint |  |
|  |  |  |  |  |  | GRAND TOTAL |  | $137,794$ |  |  |  |

## SD38 IN HARTFORD



* Cost for this work will be included in the contract unit price per foot for Repair Concrete Curb and/or Gutter


## SD38 IN HARTFORD



* Cost for this work will be included in the contract unit price per foot for Repair Concrete Curb and/or Gutter


## SD38 IN HARTFORD



## SD38 IN HARTFORD



SD38 OVER SKUNK CREEK


## SD38 AT W 60TH ST N IN SIOUX FALLS

SD38 AT W0TH ST N IN SIOUX FALLS


## SD38 AT W 60TH ST N IN SIOUX FALLS

## SD38 AT W 60TH ST N IN SIOUX FALLS

| JointNo. | WBDRIVING LANE LANE |  | $\stackrel{\text { WB }}{\text { PASSING }}$ LANE |  | LEFT TURN LANE |  | $\begin{gathered} \text { EB } \\ \text { PASSING } \\ \text { LANE } \end{gathered}$ |  | EBDRIVING LANE LANE |  | NRCP <br> REPAIR <br> SqYds | NEW JOINT CONFIG. (NRCP) | INSERT STEEL BAR IN PCC PAVEMENT (NRCP) |  |  | DOWEL BAR Each |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
|  |  |  | No. $9 \times 18$ " DEFORMED TIE BARS Each | No. $5 \times 24$ DEFORMED tie bars Each |  |  | INSERT <br> STEEL <br> BAR IN NRCP total Each |  |  |  |  |  |
|  | L | w |  |  | L | w |  | L | w |  |  |  | w | L | w |  |
|  | Ft | Ft |  |  | Ft | Ft |  | Ft | Ft | Ft |  |  | Ft | Ft | Ft |  |
| TOTALS: |  |  |  |  |  |  |  |  |  |  | 1393.1 |  | 1898 | 1009 | 2907 | 1674 |
| AdDITIONALQUANTITIES: |  |  |  |  |  |  |  |  |  |  | 280.0 |  | 380 | 200 | 580 | 330 |
| GRAND |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |
| totals |  |  |  |  |  |  |  |  |  |  | 1673.1 |  | 2278 | 1209 | 3487 | 2004 |

NRC PAVEMENT REPAIR AREA TYPES
$\mathrm{W}=$ Two Working Joints (Use only if repair is full roadway width and uniform length (across all lanes))
$\mathrm{T}=\mathrm{T}$ wo Tied Joints
$\mathrm{B}=$ O One Working \&
$B=$ One Working \& One Tied Joint
$R=$ Two Tied Joints with Original Joint Restored with Dowel Bar Assembly

## 190 NEAR BUFFALO RIDGE



## I90 NEAR BUFFALO RIDGE



## I90 NEAR BUFFALO RIDGE



NRC PAVEMENT REPAIR AREA TYPES
$\mathrm{W}=$ Two Working Joints (Use only if repair is full roadway width and uniform length (across all lanes))
$T=T w o ~ T i e d ~ J o i n t s ~$
$\mathrm{R}=\mathrm{T} w \mathrm{~T}$ Tied Joints with Original Joint Restored with Dowel Bar Assembly

I29N NEAR I90 INTERCHANGE IN SIOUX FALLS






## SD34 IN HOWARD



## SD34 IN HOWARD


$\frac{\text { NRC PAVEMENT REPAIR AREA TYPES }}{W}$ = Two Working Joints (Use only if repair is full roadway width and uniform length (across all lanes))
$T=$ Two Tied Joints
$B=$ One Working \& One Tied Joint $\quad$ Rectred with Dowe Bar Assemb

## I229S NEAR I90 INTERCHANGE IN SIOUX FALLS



I229S NEAR I90 INTERCHANGE IN SIOUX FALLS


NRC PAVEMENT REPAIR AREA TYPES
(Use only if repair is full roadway width and uniform length (across all lanes)
= One Working \& One Tied Join
= One Working \& One Tied Joint

## I90E NEAR I229 INTERCHANGE



NRC PAVEMENT REPAIR AREA TYPES
$\mathrm{W}=$ Two Working Joints (Use only if repair is full roadway width and uniform length (across all lanes))
$\mathrm{T}=\mathrm{Two} \mathrm{Tied}$
$T=T w o$ Tied Joints
$B=$ One Working \& One Tied Joint
$R=$ Two Tied Joints with Original Joint Restored with Dowel Bar Assembly

## 190W NEAR 1229 INTERCHANGE



## 190W NEAR 1229 INTERCHANGE



INSERT STEEL BAR IN
INCC PAVEMENT (NRCP)


STEEL BAR IN
NRCP
TOTAL Each

| Each |
| :---: |
| 4 |
| 16 | ${ }_{2}$

## I90W NEAR I229 INTERCHANGE



NRC PAVEMENT REPAIR AREA TYPES
$\mathrm{W}=\mathrm{Two}$ Working
$\mathrm{T}=\mathrm{Two}$ Tied Joints
$\mathrm{B}=$ One Working \&
$R=$ Two Tied Joints with Original Joint Restored with Dowel Bar Assembly

## NONREINFORCED PCC PAVEMENT REPAIR

# ANY SINGLE LANE ROADWAY (RAMPS, ETC.) 

 TYPICAL REPAIR AREAS

KEY

## $\square$ <br> PCC Pavement Repair Area

PCC PAVEMENT REPAIR AREA TYPES:
W Two Working Joints (Use only if repair is full roadway W) width and uniform length (across entire driving surface))
(T) Two Tied Joints
(B) One Working \& One Tied Joint

R Two Tied Joints with Original Joint Restored with Dowel Bar Assembly

Steel Bars for Transverse Joints
Pavement Thickness >= $10.5^{\prime \prime}$

- Drilled in $11 / 2^{\prime \prime} \times 18^{\prime \prime}$ epoxy coated plain round dowel bars spaced 18" center to center.
Drilled in No. $11 \times 18^{\prime \prime}$ epoxy coated deformed tie bars spaced $18^{\prime \prime}$ center to center.
Pavement Thickness >=8.5" and < $10.5^{\prime \prime}$ _Drilled in $11^{\prime \prime} \times 18^{\prime \prime}$ epoxy coated plain round dowel bars spaced $18{ }^{\prime \prime}$ center to center.
Drilled in No. $9 \times 18^{\prime \prime}$ epoxy coated deformed
tie bars spaced $18^{\prime \prime}$ center to center.
Pavement Thickness < 8.5"
- Drilled in 1" $\times 18^{\prime \prime}$ epoxy coated plain round dowel bars spaced $18^{\prime \prime}$ center to center. tie bars spaced 18" center to center
㔽 Dowel Bar Assembly

Steel Bars for Longitudinal Joints
No. $5 \times 30$ " epoxy coated deformed tie bars Sawed Joint - spaced 48" center to center. Construction Joint - spaced 48" center to center.

No $5 \times 24$ " epoxy coated deformed tie bars. Drilled In - spaced 30" center to center

NOTES: Saw around repair areas full depth for removal.
(1) Where possible, transverse joints will be constructed/maintained full roadway width
(2) Edges of repair areas will be formed to match the width of the existing concrete pavement.
(3) Need for bars in small repair areas on/near the shoulder to be determined on a case-by-case basis, on construction by the Engineer

## UP TO TWO LANE ROADWAY WITH CENTER LANE OR UP TO SIX LANE DIVIDED ROADWAY TYPICAL REPAIR AREAS



KEY
$\square$ PCC Pavement Repair Area

PCC PAVEMENT REPAIR AREA TYPES:
W) Two Working Joints (Use only if repair is full
W) roadway width and uniform length (across all lanes))
(T) Two Tied Joints
(B) One Working \& One Tied Joint

R Two Tied Joints with Original Joint Restored with
Rowel Bar Assembly
Longitudinal Keyway Joints Without Bars

- K - Where a repair area intersects an existing longitudina keyway joint without tie bars, the newly constructed joint should also be a keyway without tie bars.

Steel Bars for Transverse Joints
Pavement Thickness >= $10.5^{\prime \prime}$ - Drilled in $11 / 2^{\prime \prime} \times 18^{\prime \prime}$ epoxy coated plain round dowel bars spaced 18" center to center.
Drilled in No. $11 \times 18$ " epoxy coated deformed tie bars spaced $18^{\prime \prime}$ center to center.
Pavement Thickness >=8.5" and < 10.5" _Drilled in $11_{4}^{\prime \prime} \times 18^{\prime \prime}$ epoxy coated plain round dowel bars spaced $18^{\prime \prime}$ center to center.
Drilled in No. $9 \times 18$ " epoxy coated deformed
tie bars spaced $18^{\prime \prime}$ center to center.
Pavement Thickness $<8.5^{\prime \prime}$
Pavement Trilled in $1^{\prime \prime} \times 18^{\prime \prime}$ epoxy coated plain round dowel bars spaced 18" center to center.
Drilled in No. $8 \times 18^{\prime \prime}$ epoxy coated deformed tie bars spaced 18 " center to center
囟 Dowel Bar Assembly

Steel Bars for Longitudinal Joints
No. $5 \times 30$ " epoxy coated deformed tie bars Sawed Joint - spaced 48" center to center. Construction Joint - spaced 48" center to center

No $5 \times 24$ " epoxy coated deformed tie bars Drilled In - spaced 30" center to center

NOTES: Saw around repair areas full depth for removal.
(1) Where possible, transverse joints will be constructed/maintained full roadway width
(2) Edges of repair areas will be formed to match the width of the existing concrete pavement.
(3) Need for bars in small repair areas on/near the shoulder to be determined on a case-by-case basis, on construction by the Engineer

NONREINFORCED PCC PAVEMENT REPAIR

# uP to four lane roadway or up to eight lane divided roadway 

 TYPICAL REPAIR AREAS

KEY:
$\square$ PCC Pavement Repair Area

PCC PAVEMENT REPAIR AREA TYPES:
W) Two Working Joints (Use only if repair is full W) roadway width and uniform length (across all lanes))
(T) Two Tied Joints
(B) One Working \& One Tied Joint

R Two Tied Joints with Original Joint Restored with Dowel Bar Assembly

Longitudinal Keyway Joints Without Bars
K - Where a repair area intersects an existing longitudinal keyway joint without tie bars, the newly constructed joint should also be a keyway without tie bars.

Steel Bars for Transverse Joints
Pavement Thickness >= $10.5^{\prime \prime}$ Drilled in $11 / 2^{\prime \prime} \times 18^{\prime \prime}$ epoxy coated plain round dowel bars spaced 18" center to center.
Drilled in No. $11 \times 18$ " epoxy coated deformed tie bars spaced $18^{\prime \prime}$ center to center.
Pavement Thickness >=8.5" and < $10.5^{\prime \prime}$ - Drilled in $11^{\prime \prime} \times 18^{\prime \prime}$ epoxy coated plain round dowel bars spaced $18{ }^{\prime \prime}$ center to center.
Drilled in No. $9 \times 18$ " epoxy coated deformed tie bars spaced 18" center to center.

Pavement Thickness $<8.5^{\prime \prime}$

- Drilled in 1" $\times 18^{\prime \prime}$ epoxy coated plain round dowel bars spaced 18" center to center.
Drilled in No. $8 \times 18$ " epoxy coated deformed tie bars spaced 18 " center to center.
㔽 Dowel Bar Assembly

Steel Bars for Longitudinal Joints
No $5 \times 30$ " epoxy coated deformed tie bars Sawed Joint - spaced 48" center to center. Construction Joint - spaced 48" center to center.

No. $5 \times 24$ " epoxy coated deformed tie bars Drilled In - spaced 30" center to center

NOTES: Saw around repair areas full depth for removal.
(1) Where possible, transverse joints will be constructed/maintained full roadway width
(2) Edges of repair areas will be formed to match the width of the existing concrete pavement.
(3) Need for bars in small repair areas on/near the shoulder to be determined on a case-by-case basis, on construction by the Engineer

## NONREINFORCED PCC PAVEMENT REPAIR

DEFORMED TIE BAR INSERTION
TYPE T - (TWO TIED JOINTS)

$$
\begin{aligned}
& \text { Sawed joint to be filled with: } \\
& \text { Low Modulus Silicone Sealan }
\end{aligned}
$$

$$
\begin{aligned}
& \text { Low Modulus silicone Sealant } \\
& \text { in Rural Section. }
\end{aligned}
$$

Hot Poured Elastic Joint Sealer

$$
\begin{aligned}
& \text { Hot Uoured Elastic Joint Sealer } \\
& \text { in Urban Section }
\end{aligned}
$$


-

PLAIN ROUND DOWEL BAR INSERTION TYPE W - (TWO WORKING JOINTS)


DEFORMED TIE BAR AND PLAIN ROUND DOWEL BAR INSERTION TYPE B - (ONE TIED JOINT AND ONE WORKING JOINT)

Sawed joint to be filled with:
Low Modulus Silicone Sealant
in Rural Section,
Hot Poured Elastic Joint Sealer
in Urban Section
in Urban Section Tied

L
 Working
Joint


Sawed joint to be filled with: Backer Rod and Low Modulus
Silicone Sealant in Rural Sectio Silicone Sealant in Rural Section
Hot Poured Elastic Hot Poured Elastic Joint Sealer

DEFORMED TIE BAR INSERTION WITH DOWEL BAR ASSEMBLY DEFORMED TIE BAR INSERTION WITH DOWEL BAR ASSEMBLY
TYPE R - (TWO TIED JOINTS AND ONE WORKING JOINT - ORIGINAL JOINT RESTORED)


| Existing <br> Pavement <br> Thickness | Epoxy Coated <br> Deformed <br> Tie Bar Size | Epoxy Coated <br> Plain Round <br> Dowel Bar Size |
| :---: | :---: | :---: |
| $\mathrm{T}>=10.5^{\prime \prime}$ | No. $11 \times 18^{\prime \prime}$ | $11 / 2^{\prime \prime} \times 18^{\prime \prime}$ |
| $\mathrm{T}>=8.5^{\prime \prime} \&$ | No. $9 \times 18^{\prime \prime}$ | $1^{11 / 4} \times 18^{\prime \prime}$ |
| $\mathrm{T}<105^{\prime \prime}$ |  |  |
| $\mathrm{T}<8.5^{\prime \prime}$ | No. $8 \times 18^{\prime \prime}$ | $1^{\prime \prime} \times 18^{\prime \prime}$ |

## $T=$ Existing pavement thickness $T_{N}=$ New pavement thickness.

Bar embedded to a minimum depth of 9 inches into the existing pavement by utilizing an epoxy resin adhesive.
Cost for furnishing and inserting steel bars (deformed tie and plain round dowel) will be included in the contract unit
price per each for insert Steel Bar in PCC Pavement.
Cost for furnishing and installing dowel bar assembly will be included in the contract unit price per each for Dowel Bar.

| $T_{N}=T$ |
| :--- |
| (top of new pavement |
| will be flush with top |
| of existing pavement) |

## NONREINFORCED PCC PAVEMENT REPAIR


$T_{N}=$ New pavement thickness.
Cost for furnishing and inserting tie bars will be incidental to the contract
unit price per square yard for Nonreinforced PCC Pavement Repair
unit price per square yard for Nonreinforced PCC Pavement Repai

LONGITUDINAL CONSTRUCTION JOINT WITH DRILLED IN TIE BARS

$T$ = Existing pavement thickness

Bar embedded a minimum depth of 9 inches into the existing pavement by utilizing an epoxy resin adhesive
Bars will be placed a minimum of 15 inches from existing transverse contraction joints.
Cost for furnishing and inserting drilled in tie bars will be included in the contrac unit price per each for Insert Steel Bar in PCC Pavement

$T_{N}=$ New pavement thickness.
The first saw cut to control cracking will be a minimum of $1 / 3$ the depth of the pavement. Additional sawing for widening the saw cut will be necessary Cost for furnishing and inserting tie bars will be incidental to the contrac unit price per square yard for Nonreinforced PCC Pavement Repair.

LONGITUDINAL CONSTRUCTION JOINT WITH DRILLED IN TIE BARS

$T$ = Existing pavement thickness.
Bar embedded a minimum depth of 9 inches into the existing pavement by utilizing an epoxy resin adhesive.
Bars will be placed a minimum of 15 inches from existing transverse contraction joints.
Cost for furnishing and inserting drilled in tie bars will be included in the contract unit price per each for Insert Steel Bar in PCC Pavement.

## NONREINFORCED PCC PAVEMENT REPAIR

LONGITUDINAL SHOULDER CONSTRUCTION JOINT WITH TIE BARS \& KEYWAY


LONGITUDINAL SHOULDER JOINT WITH DRILLED IN TIE BARS

$\mathrm{T}=$ Existing shoulder pavement thickness.
Bar embedded a minimum depth of 9 inches into the existing pavement by utilizing an epoxy resin adhesive.

Bars will be placed a minimum of 15 inches from existing transverse contraction joints.
Cost for furnishing and inserting drilled in tie bars will be included in the contract
Cost for furnishing and inserting drilled in tie bars will be in
unit price per each for Insert Steel Bar in PCC Pavement.

SAWED LONGITUDINAL SHOULDER JOINT


The first saw cut to control cracking will be a minimum of $1 / 3$ the depth of th pavement. Additional sawing for widening the saw cut will be necessary Cost for furnishing and inserting tie bars will be incidental to the contract unit price per square yard for Nonreinforced PCC Pavement Repair.

LONGITUDINAL SHOULDER JOINT WITH DRILLED IN TIE BARS

$\mathrm{T}_{\mathrm{s}}=$ New shoulder pavement thickness.
Bar embedded a minimum depth of 9 inches into the existing pavement by utilizing
an emoxy resin adhesive.
Bars will be placed a minimum of 15 inches from existing transverse contraction joints.
Cost for furnishing and inserting drilled in tie bars will be included in the contract Cost for furnishing and inserting driled in tie bars will be in
unit price per each for Insert Steel Bar in PCC Pavement.

SAW \& SEAL TRANSVERSE JOINTS

** The saw cut to control cracking will be a minimum of $1 / 4$ the thickness of the pavement.


## REPAIR OF TYPE A SPALLS

NRC SPALL PATCHES (PLAN VIEW)


CRC SPALL PATCHES (PLAN VIEW)


TIE BAR RETROFIT (STITCHING)


TIE BAR RETROFIT (STITCHING)


Stitch Bar Spacing 24" Max


## TYPICAL PCC PAVEMENT REPAIR AROUND MANHOLES

BOX-OUT DETAIL IN PCC PAVEMENT


Where the utility access is offset from the longitudinal and transverse joints


Where the utility access is intersected by the longitudinal and transverse joints


[^0] present or at Longitudinal or Transverse joint.

REBAR LAYOUTS IN PCC PAVEMENT WITH BOX-OUT


Where the utility access is offset from the longitudinal and transverse joints


Where the utility access is intersected by the longitudinal and transverse joints


Where no Longitudinal or Transverse joints are present or at Longitudinal or Transverse joint.

REBAR LAYOUT
IN PCC PAVEMENT WITHOUT BOX-OUT


The rebar will not cross any joint in the concrete pavement. If manhole is next to a joint in the concrete pavement the Engineer will approve a revised layout of the rebar


* Rebar will be placed at the midpoint depth of the PCC Pavement. Cost for furnishing \& installing rebar and constructing box-outs will be incidental to the contract unit price per square yard for Nonreinforced PCC Pavement Repair and/or Fast Track Concrete for PCC Pavement Repair

* If sawing is required, the cost will be incidental to the contract unit price per foot for Repair Concrete Curb and/or Gutter

- 9" Minimum 23" Maximum

44 No. $5 \times 24$ " Epoxy Coated Deformed Tie Bar Drilled 9" into in place pavement **

4A4 No. $5 \times 24$ " Epoxy Coated Deformed Tie Bar Drilled 9" into in place curb \& gutter **

See standard plate(s) for Type B, D and F Concrete Curb and Gutter and Type P Concrete Gutter for construction and forming details.


## LAYOUT FOR REPAIR CONCRETE CURB AND/OR GUTTER ADJACENT TO DROP INLET

REMOVAL


## Remove Concrete Curb and/or Gutter

* If sawing is required, the cost will be incidental to the contract unit price per foot for Repair Concrete Curb and/or Gutter

- 9" Minimum 23" Maximum

44 No $5 \times 24$ " Epoxy Coated Deformed Tie Bar Drilled 9" into in place pavement **

44 No. $5 \times 24$ " Epoxy Coated Deformed Tie Bar Drilled 9" into in place curb \& gutter **



## RESEAL LONGITUDINAL JOINT

 WITH HOT POURED ELASTIC JOINT SEALER

Additional sawing for widening the saw cut to provide the width for the installation of the Hot Poured Elastic Joint Sealer will be necessary

The first saw cut to control cracking will be a minimum of $1 / 4$ the depth of the pavement. Additional sawing for widening the saw cut to provide the width for the installation of the Hot Poured Elastic Joint Sealer will be necessary.

## SEQUENCE OF OPERATIONS

1. Install Traffic Control devices per the details in these plan 2. Complete all concrete repair work
2. Grind areas listed in the plans
3. Reseal joints
4. Install permanent pavement marking

If the Contractor requests to deviate from the sequence of operations will be submitted in writing to the Engineer for review. Approval of an alternate sequence of operations will only be allowed when the proposed changes meet with the Department's intent for traffic control and sequencing of the work. An alternate implementation.

## GENERAL TRAFFIC CONTROL

Existing guide, route, informational logo, regulatory, and warning signs will be temporarily reset and maintained during construction. Removing, relocating, covering, salvaging, and resetting of existing traffic control devices, including delineation, will be the responsibility of the Contractor. Cost for this work will b incidental to the contract unit prices for the various items unless otherwise specified in the plans. Any delineators and signs damaged or lost will be replaced by th

All temporary traffic control sign locations will be set in the field by the Contractor and verified by the Engineer prior to installation.

All temporary speed limit signs will have a minimum mounting height of 5 feet in rural locations, even when mounted on portable supports.

Portable sign supports will not be located on sidewalks, bicycle facilities, or other areas designated for pedestrian or bicycle traffic.
All construction operations will be conducted in the general direction of traffic movement

If there is a discrepancy between the traffic control plans, standard plates, and the
MUTCD, whichever is more stringent will be used, as determined by the Engineer. darkness.

Fixed location signing placed more than 4 calendar days prior to the start of construction will be covered or laid down until the time of construction. The covers must be approved by the Engineer prior to installation. The cost of material, labor, and equipment necessary to complete this work will be incidental to other contrac items. No separate payment will be made.

All fixed location signs, sign posts, and breakaway bases will be removed within 7 calendar days following pavement marking.

All haul trucks will be equipped with an additional flashing amber light that is visible to the contract unit prices for the various related items.

## GENERAL TRAFFIC CONTROL (CONTINUED)

Traffic will be maintained on the driving lanes only. Use of the shoulder as a driving lane will not be permitted. Any damage to the shoulder due to rerouted traffic or Contractor's equipment will be repaired at no expense to the Department.

A Type 3 Barricade will be installed at the end of a lane closure taper as detailed in these plans. Additional Type 3 Barricades will be installed facing traffic within the closed lane at a spacing of $1 / 4$ mile.
Lane closures will be limited to 5 miles in length. The distance between the closest points of any two-lane closures will be at least 3 miles, excluding tapers.

Construction vehicles will exit or enter the construction work zone at locations identified by the Engineer. At no time will construction vehicles utilize the maintenance crossovers or the Interstate median to exit or enter Interstate traffic.

On Interstate projects with more than one construction site, slow moving equipment that operates at a speed less than 40 MPH may mobilize between sites if the equipment travels on the shoulder. The slow-moving equipment will also display a flashing amber light and a slow-moving sign.

## LANE CLOSURES

Interstate lane closures shorter than 5 miles will be used if 5 miles is greater than the length of work that can be accomplished in one day's production. More than one lane closure may be permitted; however, there will be a minimum of a threemile section between lane closures, excluding the tapers.

Interstate lane closures will be removed when work will not be occurring for a period of 3 or more calendar days. Activities that do not involve workers being present, such as curing time for concrete, constitute work. Lane closures will not be set up on a Friday if no work will be occurring on Saturday or Sunday. In these cases, the lane closure will be installed on Monday

## FLAGGING

Operations will be conducted so that the traveling public will not have to wait longer than 15 minutes at the flagger station

Additional flagger warning signs and flagger hours have been included in the Estimate of Quantities for use on intersecting roads. These flaggers will be used as directed by the Engineer and will be used primarily during daytime hours. Also included in the Estimate of Quantities are WAIT FOLLOW PILOT CAR signs for use on low volume intersecting roads as determined by the Engineer. WAIT FOLLOW PILOT CAR signs will not block the view of the stop sign


## FLAGGING (CONTINUED)

It is required that the flaggers and pilot car operators be able to communicate with one another. If an emergency vehicle needs to pass through the project the Contractor will be required to expedite traffic movement. Cost associated with this will be incidental to the contract unit price per hour for Flagging

## WORK ZONE SPEED REDUCTION

The Department is required to obtain a speed reduction resolution prior to the installation of any SPEED LIMIT (R2-1) signs shown on standard plate 634.63 or as shown in the plans. To provide adequate time for the resolution to be enacted, installation of any work zone speed reduction signs on the project. The information provided by the Contractor will include the anticipated date of sign installation, the newly reduced speed limit, the location of the work zone, and the anticipated completion date of work requiring the speed reduction

## TEMPORARY PAVEMENT MARKING

Temporary flexible vertical markers (tabs) will be used to mark dashed centerline, No Passing Zones, and applicable lane lines. Paint will not be allowed for temporary pavement marking on the asphalt concrete wear course.

Temporary flexible vertical markers (tabs) may be used as detailed in the specifications.

Covers on the tabs will be sufficiently secured to prevent traffic from dislodging the cover and when removed, the covers will be properly disposed of. The Contractor will remove and properly dispose of the tabs after permanent pavement marking is accomplished within one week of completion of the permanent pavement marking.
Full reflectivity of all temporary flexible vertical markers (tabs) is required at all times. The Contractor will be required to replace any missing or non-reflective tabs at no additional cost to the State

Prior to nightfall, tabs will be required to mark centerline on segments of roadway where existing centerline marking has been removed and new marking has not been installed.

## PERMANENT PAVEMENT MARKING

The Contractor will be required to repaint all existing pavement marking including centerline, edge line, lane lines, turn arrows, stop bars, and pedestrian crossings. This list is approximate. The Contractor will be required to document and be able to relocate for replacement of the existing, turn arrows, stop bars, pedestrian crossings, etc. before such marking is obliterated. Additional quantities are included in the estimate of quantities to paint the additional pavement marking. unit prices for the various contract items.

## TRAFFIC CONTROL FOR PCCP REPAIR

Each mainline concrete repair location, from which the in-place concrete has been removed, will be marked with a minimum of two reflectorized drums. In areas installed at a spacing of 660 feet alternating with the Type 3 Barricades.

Construction workspaces on undivided roadways will be limited to 1000 feet in length. The distance between the closest points of any two construction workspaces, including channeling devices, will not be less than 3 miles. Drivers in beyond the work zospas be able . length.

Construction workspaces in urban areas will be limited to 3 blocks in length. The minimum distance between workspaces will be 3 blocks.

## traffic.

The Contractor will use Flaggers during peak traffic hours and at times specified by the Engineer to supplement the stop conditions. Peak traffic hours are assumed to be 6:30 am to 8:30 am, 11:30 am to 1:00 pm and 4:30 pm to $6: 00 \mathrm{pm}$. It is possible that Flagging will be required during all daytime hours. Advance warning Flagger signs will be

Holes adjacent to centerline in the lane open to traffic created during removal and replacement of PCC pavement repair areas will be filled with gravel cushion material and cold-mix asphalt concrete prior to opening the lane to traffic. Grave cushion material and cold-mix asphalt concrete will be furnished by the Contractor.

Holes in the gravel and asphalt concrete shoulders created during removal and replacement of PCC pavement repair areas will be filled with gravel cushion material and hot-mix asphalt concrete (to match the shoulder surfacing) prior to opening the lane to traffic. Gravel cushion material and hot-mix asphalt concrete will be furnished and installed by the Contrator

Cost for furnishing, hauling, and placing gravel cushion material and asphalt concrete will be incidental to the contract unit price per square yard for Nonreinforced PCC Pavement Repair
not be allowed.
Extra care will be taken to protect the in place asphalt concrete shoulders on 190 and 129 locations. In all workspaces in these areas, flexible delineators will be required on the shoulders and will also be placed in locations to adequately keep traffic completely off the shoulders. Continuous maintenance will be required to

Type B warning lights will be placed on top of FLAGGER (W20-7) symbol signs.
Joints in approaches to signalized intersections containing vehicle detector loops will not be sawed, sealed, or otherwise disturbed.

The Contractor will be required to contact the Engineer two weeks in advance so that the Region Traffic Engineer can arrange for signal timings to be adjusted to accommodate traffic when a lane is closed near a signalized intersection.

## TRAFFIC CONTROL FOR PCCP REPAIR (CONTINUED)

The Contractor will maintain pedestrian access at crosswalk locations. Additional traffic control devices will be used as necessary to accommodate the pedestrian traffic if work activities block an existing crosswalk.

Reflectorized drums or Type 2 Barricades will be used to maintain a minimum of two-way traffic at intersecting roads or streets. The Contractor will mark and maintain alternating one-way access to businesses and residences along the project with cones, drums, or Type 1 Barricades. The Contractor will advise affected businesses before a restriction to the business is installed, as well as the anticipated duration of the restriction.

PORTABLE TEMPORARY TRAFFIC CONTROL SIGNAL
Bridge over Skunk Creek on SD38

| PHASING AND SEQUENCING |  |  |  |  |  |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| INTERVAL <br> SIGNAL HEADS | 1 | 2 | 3 | 4 | 5 | 6 | FLASH DISPLAY |
| SD38EB $\quad \phi$ A | G | Y | R | R | R | R | R |
| SD38 WB ${ }^{\text {d }}$ | R | R | R | G | Y | R | R |
| TIMINGS BASED ON MAXIMUM 1400 F ${ }^{*}$ dISTANCE between opposing stop lines and traffic speed OF 35 MPH . |  |  |  |  |  |  | $\begin{aligned} & \text { FLASH } \\ & \text { TIME } \end{aligned}$ |
| CYCLE LENGTH $=112$ Seconds |  |  |  |  |  |  | FAILURE OR EMERGENCYONLY |
| PHASES |  | $\phi$ A |  |  | ¢ B |  |  |
| MOVEMENTS |  | $\rightarrow$ |  |  | - |  |  |
| MIN. GREEN (SEC) |  | 10 |  |  | 10 |  |  |
| EXTENSION GREEN (SEC) |  | 5 |  |  | 5 |  |  |
| MAX GREEN(SEC) |  | 25 |  |  | 25 |  |  |
| YELLOW (SEC) |  | 4 |  |  | 4 |  |  |
| ALL RED (SEC) |  | 27 |  |  | 27 |  |  |

- The timings may be adjusted if the length between the stop lines varies from the 1400 ft value used in calculation The all red times may be recalculated as follows:

$$
\text { All Red }=t+\frac{V}{2 a}+\frac{W+L}{V}-Y
$$

Where $\mathrm{W}=$ The distance between stop lines ( ft ).
Green times may be adjusted accordingly, however the total cycle length shall not exceed 120 seconds.

The portable temporary traffic control signal will be set up to dwell in red
All vehicle signal heads will have backplates with retroreflective border. The vehicle signal head backplates will have a factory applied 3-inch wide yellow retroreflective border. Sheeting for the border will be Type IX or Type XI in conformance with ASTM D4956.

## PORTABLE TEMPORARY TRAFFIC CONTROL SIGNAL (CONTINUED)

Signal backplates will be polycarbonate, aluminum, or aluminum-composite Minimum material thicknesses are:

> Polycarbonate, 0.10 -inch
> Aluminum, 0.06 -inch
> Aluminum-Composite, 0.08 -inch

Signal backplates will extend not less than 5 inches from the edge of the signal head at the top, bottom, and sides
All traffic signal equipment and material will meet the requirements of Sections 635 and 985 of the Specifications except the controller requirements.

Cost involved with constructing the portable temporary traffic control signal as specified above and on the plans, will be included in the contract unit price per unit for Portable Temporary Traffic Control Signal.

## CONTACTOR FURNISHED PORTABLE CHANGEABLE MESSAGE SIGN

One week prior to starting work affecting the traveling public, portable changeable message signs (PCMS) will be installed at locations detailed in the plans to notify drivers of the upcoming construction. The Contractor will program the portable changeable message signs with the following messag

ROAD WORK
DATE
When work begins that will affect traffic patterns, the Contractor will re-program the PCMS with the messages below:

ROAD

BE PREPARED
TO STOP

## INCIDENTS

An incident is an emergency road user occurrence, a natural disaster, or other unplanned event that affects or impedes the normal flow of traffic such as a crash hazardous material spill, or other event.

The Contractor will set up a meeting prior to start of work to plan and coordinate responses to an incident. The Contractor will invite the Department of Transportation, the South Dakota Highway Patrol, the County Sheriffs, and local emergency response entities to the meeting
The Contractor will assist to maintain traffic as required by these plan notes and as agreed to at that meeting

Emergency vehicle access through the project will be considered and discussed at the meeting.

The Contractor may be required to modify messages on portable changeable message signs or relocate portable changeable message signs, and to provide flaggers to direct or detour traffic. The Contractor should be prepared to relocate advance warning signs if determined to be necessary for a major traffic inciden lasting more than two hours. Fixed location ground mounted signs may be covered and additional portable signs provided
No additional payment will be made for the modification of portable changeable message sign messages or the relocation of portable changeable message signs.

Cost for the relocation of an advance warning sign due to an incident will be $50 \%$ of the designated sign rate. Flaggers will be paid for at the contract unit price per hour for Flagging.

## TEMPORARY PAVEMENT MARKING TAPE, TYPE I

Temporary pavement marking for stop lines will consist of 4" Temporary Pavement Marking Tape Type I. Placement of each $24^{\prime \prime}$ white stop line will be accomplished by placing six pieces of 4 " $\times 12^{\prime}$ tape adjacent to one another. Each workspace requires two stop lines which is an equivalent of approximately 144 of 4 tape required for centerline marking shown on standard plate 634.25. Temporary tape will be removed upon completion of the project.

## TEMPORARY RAISED PAVEMENT MARKERS

Temporary raised pavement markers will be used for marking edge lines, lane lines, and tapers and centerlines. Temporary raised pavement markers will be used on all new permanent surfacing sections of roadway and on existing surfacing where temporary marking locations are different than existing marking locations, unless noted or as directed by the Engineer.
Temporary raised pavement markers will be attached to the roadway surface with a flexible non-permanent bituminous adhesive capable of being removed from the roadway surface or with an adhesive approved by the Engineer.

Cost to furnish, install, replace if necessary, and remove the markers will be Markers.

## GROOVING FOR COLD APPLIED PLASTIC PAVEMENT MARKING

The Contractor will establish a positive means for the removal of the grinding and/or grooving residue. Residue from dry grooving will be vacuumed. Solid residue will be removed from the pavement surfaces before being blown by traffic action or wind. The Contractor will conduct this work to control and minimize airborne dust and similar debris that may become a hazard to motor vehicle operation or nuisance to property owners. Residue from wet grooving will not be permitted to flow across lanes being used by public traffic or into gutter or drainage facilities. Residue, whether in solid or slurry form, will be disposed of in a manner that will prevent it from reaching any waterway in a concentrated state. The cleaning of the residue for grooving will be to the satisfaction of the Engineer and may require more grooving residue will be included in the contract unit price per square foot for Grooving for Cold Applied Plastic Pavement Marking contract items.

## PREFORMED THERMOPLASTIC PAVEMENT MARKING

## General

- Made of prefabricated retroreflective, resilient thermoplastic material,
- Contains glass beads uniformly distributed through the entire crosssectional area;
- Capable of being affixed to bituminous or concrete pavement by heating,
- Resistant to deterioration due to exposure to sunlight, water, salt, and adverse weather conditions;
- Under traffic wear, shows no appreciable fading in accordance with the color requirements, lifting, or shrinkage throughout the life of the marking; - Capable of conforming to pavement contours, breaks, and faults through
the action or trafic at nomal pavistics, sura
Possesses resealing characteristics, such that it is capable of fusing with
- Protected during shipment and in storage

Apply the preformed thermoplastic pavement marking as recommended by the manufacturer to provide a neat, durable marking that will not flow, distort, or crack due to temperature if the pavement surface remains stable. Use equipment and application methods specified by the manufacturer. Primer as required by the manufacturer will be provided with the material.
Application of the marking will include the use of any manufacturer recommended sealers. Sealers may be required on concrete pavements, inside grooves, or on older asphalt pavements. Prior to placing any marking on new concrete, the other standard industry methods.

Any required primers or sealers will be included in the contract unit price for the various preformed thermoplastic pavement marking items.

Provide precut messages and symbols meeting the requirements of the MUTCD and the Standard Signs Manual in custom kits. Use separate pieces or segments to form individual letters or symbols only to the extent supplied by the manufacturer. Provide shapes, sizes, and colors as required by the contract.

## Color

Will meet the color specification limits and luminance factors for Cold Applied Plastic Pavement Marking and Legends (Section 983.2 D, Tables 1 and 2).

## PREFORMED THERMOPLASTIC PAVEMENT MARKING (CONTINUED)

## Glass Beads

- Ensure the preformed thermoplastic pavement marking contains a minimum $30 \%$ intermixed glass beads by weight and a minimum $80 \%$ true

Ensure preformed thermoplastic pavement marking contains only clear beads.

## Skid Resistance

- Ensure the surface of the preformed thermoplastic pavement marking provides a skid resistance value of at least 45 British Pendulum Number (BPN) when tested in accordance with ASTM E303


## Retroreflectivity

- Provide preformed thermoplastic pavement marking meeting the minimum initial pavement marking retroreflectivity values using 30 m geometry and meeting the testing procedures of ASTM E1710:

| Minimum Initial Pavement Marking Retroreflectivity |  |  |
| :--- | :--- | :--- |
|  | White | Yellow |
| Thermoplastic | $400 \mathrm{mcd} / \mathrm{sq} . \mathrm{ft} / \mathrm{ft}$. | $250 \mathrm{mcd} / \mathrm{sq} . \mathrm{ft} / \mathrm{ft}$. |
| Thermoplastic, <br> enhanced skid <br> resistance (ESR) | $250 \mathrm{~d} / \mathrm{sq} . \mathrm{ft} / \mathrm{ft}$. | $150 \mathrm{~d} / \mathrm{sq} . \mathrm{ft} / \mathrm{ft}$. |

## Thickness

- A longitudinal marking is a minimum 90 mils thick at the edges, and a maximum 125 mils thick at the center of the stripe.
- Transverse marking and symbols are a minimum 125 mils thick at the edges, and a maximum 160 mils thick at the center.


## Sample

- Prior to application, the Contractor will provide a sample of the preformed thermoplastic pavement marking to be used on the project to the Region thermoplastic pavement marking to be used
Traffic Engineer for inspection and approval.
- Do not begin application of the preformed thermoplastic pavement marking prior to obtaining the Region Traffic Engineer's approval of the preformed thermoplastic pavement marking material. The Region Traffic Engineer's approval of the preformed thermoplastic pavement marking does not void other preformed thermoplastic pavement marking requirements specified.


## HIGH BUILD WATERBORNE PAVEMENT MARKING PAINT

Material will be applied as per manufacturer's recommendations. Reflective media consisting of glass beads as well as bonded core reflective elements will be adhered to the paint.

No further testing of this material will be required. Reflective media consisting of glass beads as well as bonded core reflective elements will be adhered to the paint.
The bonded core reflective elements will contain either clear or yellow tinted microcrystalline ceramic beads bonded to the outer surface. The bonded core reflective elements will provide a $50 / 50$ blend of dry to wet ratio of reflective a minimum index of refraction of 1.8 for dry retroreflectivity and 2.4 for wet retroreflectivity when tested using the liquid oil immersion method.

Pavement marking not conforming to the retroreflectivity requirements will be removed and replaced. If replacement of marking cannot be applied within the same year, the Contractor will schedule subject work to be completed no later than June $15^{\text {th }}$ in the following year

Upon replacement, the retroreflectivity testing process will be done again requiring new readings.
The Department will randomly select one test location per mile of each edge line including ramps and one test location per mile of centerline (solid and/or skip line will be considered as one centerline). Three retroreflectivity readings will be taken at each test location. The three readings will be averaged and become the reading for that test location.

## Initial readings:

| Pavement Marking Color | Minimum Value |
| :--- | :--- |
| White | $350 \mathrm{mc} / \mathrm{m}^{2} / \mathrm{lux}$ |
| Yellow | $275 \mathrm{mc} / \mathrm{m}^{2} / \mathrm{lux}$ |

All pavement marking not conforming to the requirements provided in these plans will be considered deficient and will be removed and replaced. Additional retroreflectivity readings will be taken by the Department to determine the limits of retroreflectivity readings will be taken by the Department to determine the limits of
removal. The removal will be accomplished using suitable sand blasting or grinding removal. The removal will be accomplished using suitable sand blasting or grinding remove at least $90 \%$ of the deficient line, with no excessive scarring of the existing pavement. The removal width will be one inch wider all around the nominal width of the pavement marking to be removed. Removal and replacement of the pavement marking will be at the Contractor's expense, with no cost incurred by the State.
High Build Waterborne Pavement Marking Paint applied after October 15 must be formulated as cold-weather waterborne paint. Cold weather waterborne paint will meet the requirements of Section 980.1 B.

## RATES OF MATERIALS FOR HIGH BUILD WATERBORNE PAVEMENT

 MARKING PAINTSolid $4^{\prime \prime}$ line $=27.8$ Gals/Mile
Dashed $4^{\prime \prime}$ line $=7.6 \mathrm{Gal} /$ Mile
Glass Beads = 5.3 Lbs/Gal.
moste Reflective Elements = 2.1 Lbs/Gal
Cost for material, labor and equipment necessary to furnish and install the pavement marking will be incidental to the contract unit price for the respective High Build Waterborne Pavement Marking Paint items.

PAINTING FOR PCC REPAIRS

## PAVEMENT MARKING

Typical pavement marking as shown on this sheet will be
applied throughout the entire length of the roadway
Traffic Control will be incidental to the cost of application. The striper and advance or trailing warning vehicle will be equipped with flashing amber lights and advance warning arrow board.

Left Arrows, in sets of two, spaced 8 ' or $16^{\prime}$ arrow tip to arrow tip, (when two are required) will be positioned in the center turn lane at 300 ' spacing at a frequency of one set of arrows per block or at existing arrow locations.

Application rates will be as follows:


Solid Are
Arrows
Left Arrows
Right Arrows
Straight Arrows
Combo Arrows
Lane Drop Arrows
$\frac{\text { Messages }}{\text { STOP }}$
STOP AHEAD
RXRw/ Stop Li
RXRW/Stop Lin
SCHOOL X-ING
Additional White


SD38 IN HARTFORD

|  |  | CONVENTIONAL ROAD |  |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: |
| SIGN CODE | SIGN DESCRIPTION | NUMBER | SIGN SIZE | $\begin{gathered} \text { SQFT } \\ \text { PER SIGN } \end{gathered}$ | SQft |
| R1-1 | STOP | 8 | $48^{\prime \prime}$ | 13.3 | 106.4 |
| R3-1 | RIGHT TURN PROHIBITION (symbol) | 8 | 24" $\times 24$ " | 4.0 | 32.0 |
| R3-2 | LEFT TURN PROHIBITION (symbol) | 8 | 24" $\times 24$ " | 4.0 | 32.0 |
| R4-7 | KEEP RIGHT (symbol) | 8 | $24^{\prime \prime} \times 301$ | 5.0 | 40.0 |
| W1-4 | REVERSE CURVE (L or R) | 2 | $48^{\prime \prime} \times 48^{\prime \prime}$ | 16.0 | 32.0 |
| W9-3 | CENTER LANE CLOSED AHEAD | 1 | $48^{\prime \prime} \times 48^{\prime \prime}$ | 16.0 | 16.0 |
| W13-1P | ADVISORY SPEED (plaque) | 2 | 30" $\times 301$ | 6.3 | 12.6 |
| W20-1 | ROAD WORK AHEAD | 12 | $48^{\prime \prime} \times 48^{\prime \prime}$ | 16.0 | 192.0 |
| G20-2 | END ROAD WORK | 6 | $36^{\prime \prime} \times 18^{\prime \prime}$ | 4.5 | 27.0 |
|  |  | CONVENTIONAL ROAD TRAFFIC CONTROL SIGNS SQFT |  |  | 490.0 |

SD38 OVER SKUNK CREEK

|  |  | CONVENTIONAL ROAD |  |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: |
| SIGN CODE | SIGN DESCRIPTION | NUMBER | SIGN SIZE | $\begin{array}{\|c\|} \hline \text { SQFT } \\ \text { PER SIGN } \\ \hline \end{array}$ | SQFT |
| R10-6 | STOP HERE ON RED | 2 | $24^{\prime \prime} \times 3{ }^{\prime \prime}$ | 6.0 | 12.0 |
| W1-4 | REVERSE CURVE (L or R) | 2 | $48^{\prime \prime} \times 48^{\prime \prime}$ | 16.0 | 32.0 |
| W3-3 | SIGNAL AHEAD (symbol) | 2 | $48^{\prime \prime} \times 48^{\prime \prime}$ | 16.0 | 32.0 |
| W13-1P | ADVISORY SPEED (plaque) | 2 | $301 \times 301$ | 6.3 | 12.6 |
| W20-1 | ROAD WORK AHEAD | 2 | $48^{\prime \prime} \times 48^{\prime \prime}$ | 16.0 | 32.0 |
| W20-4 | ONe LANE RoAD AHEAD | 2 | $48^{\prime \prime} \times 48^{\prime \prime}$ | 16.0 | 32.0 |
| G20-2 | END ROAD WORK | 2 | 36" $\times 18^{\prime \prime}$ | 4.5 | 9.0 |
|  |  | $\underset{\substack{\text { CONVENTIONAL ROAD } \\ \text { TRAFFIC CONTROL SIGNS SQFT }}}{ } 161.6$ |  |  |  |

SD38E AT W 60 ST N IN SIOUX FALLS

|  |  | CONVENTIONAL ROAD |  |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: |
| $\begin{aligned} & \hline \text { SIGN } \\ & \text { CODE } \\ & \hline \end{aligned}$ | SIGN DESCRIPTION | NUMBER | SIGN SIZE | $\begin{array}{\|c\|} \hline \text { SQFT } \\ \text { PER SIGN } \\ \hline \end{array}$ | SQFT |
| R3-1 | RIGHT TURN PROHIBITION (symbol) | 4 | $24^{\prime \prime} \times 24^{\prime \prime}$ | 4.0 | 16.0 |
| R3-2 | LEFT TURN PROHIBITION (symbol) | 4 | $24^{\prime \prime} \times 24^{\prime \prime}$ | 4.0 | 16.0 |
| R3-7R | RIGHT LANE MUST TURN RIGHT | 2 | $30^{\prime \prime} \times 30^{\prime \prime}$ | 6.3 | 12.6 |
| R3-7L | LEFT LANE MUST TURN LEFT | 2 | $30^{\prime \prime} \times 3{ }^{\prime \prime}$ | 6.3 | 12.6 |
| R4-7 | KEEP RIGHT (symbol) | 4 | $24^{\prime \prime} \times 30{ }^{\prime \prime}$ | 5.0 | 20.0 |
| W4-2 | LEFT or RIGHT LANE ENDS (symbol) | 1 | $48^{\prime \prime} \times 48^{\prime \prime}$ | 16.0 | 16.0 |
| W20-1 | Road work ahead | 3 | $48^{\prime \prime} \times 48^{\prime \prime}$ | 16.0 | 48.0 |
| W20-5 | LeFT or RIGHT LANE CLOSED AHEAD | 1 | $48^{\prime \prime} \times 48^{\prime \prime}$ | 16.0 | 16.0 |
| 620-2 | END ROAD WORK | 1 | $36^{\prime \prime} \times 18^{\prime \prime}$ | 4.5 | 4.5 |
|  |  | CONVENTIONAL ROAD TRAFFIC CONTROL SIGNS SQFT |  |  | 161.7 |

SD38W AT W 60 ST N IN SIOUX FALLS

|  |  | CONVENTIONAL ROAD |  |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: |
| $\begin{aligned} & \hline \text { SIGN } \\ & \text { CODE } \end{aligned}$ | SIGN DESCRIPTION | NUMBER | SIGN SIZE | $\begin{array}{\|c\|} \hline \text { SQFT } \\ \text { PER SIGN } \end{array}$ | SQFT |
| W4-2 | LEFT or RIGHT LANE ENDS (symbol) | 1 | 48" $\times 48^{\prime \prime}$ | 16.0 | 16.0 |
| W20-1 | ROAD WORK AHEAD | 1 | $48^{\prime \prime} \times 48^{\prime \prime}$ | 16.0 | 16.0 |
| W20-5 | LeFT or RIGHT LANE CLOSED AHEAD | 1 | 48" $\times 48^{\prime \prime}$ | 16.0 | 16.0 |
| G20-2 | END ROAD WORK | 1 | $36^{\prime \prime} \times 18^{\prime \prime}$ | 4.5 | 4.5 |
|  |  | CONVENTIONAL ROADTRAFFIC CONTROL SIGNS SQFT |  |  | 52.5 |

190E NEAR BUFFALO RIDGE

|  |  |  | Press WAY | / INTERSTA |  |
| :---: | :---: | :---: | :---: | :---: | :---: |
| $\overline{\text { SIGN }}$ CODE | SIGN DESCRIPTION | NUMBER | SIGN SIZE | $\begin{array}{\|c\|} \hline \text { SQFT } \\ \text { PER SIGN } \end{array}$ | SQFT |
| R2-1 | SPEED LIMIT 65 | 4 | $36^{\prime \prime} \times 48{ }^{\prime \prime}$ | 12.0 | 48.0 |
| R2-1 | SPEED LIMIT 45 | 2 | $36^{\prime \prime} \times 48^{\prime \prime}$ | 12.0 | 24.0 |
| R2-1 | SPEED LIMIT 80 | 2 | $36^{\prime \prime} \times 48^{\prime \prime}$ | 12.0 | 24.0 |
| R2-6aP | FINES DOUBLE (plaque) | 2 | 36" $\times 24$ " | 6.0 | 12.0 |
| W3-5 | SPEED REDUCTION AHEAD (65 MPH) | 2 | $48^{\prime \prime} \times 48^{\prime \prime}$ | 16.0 | 32.0 |
| W4-2 | LEFT or RIGHT LANE ENDS (symbol) | 2 | $48^{\prime \prime} \times 48^{\prime \prime}$ | 16.0 | 32.0 |
| W20-1 | Road work ahead | 2 | $48^{\prime \prime} \times 48^{\prime \prime}$ | 16.0 | 32.0 |
| W3-5 | SPEED REDUCTION AHEAD (45 MPH) | 2 | $48^{\prime \prime} \times 48^{\prime \prime}$ | 16.0 | 32.0 |
| W20-5 | LeFT or RIGHT LANE CLOSED AHEAD | 2 | $48^{\prime \prime} \times 48^{\prime \prime}$ | 16.0 | 32.0 |
| G20-2 | END ROAD WORK | 2 | $48^{\prime \prime} \times 24$ " | 8.0 | 16.0 |
|  |  | EXPRESSWAY / INTERSTATE TRAFFIC CONTROL SIGNS SQFT |  |  | 284.0 |

I90W NEAR BUFFALO RIDGE

|  |  |  | Pressway | INTERSTA |  |
| :---: | :---: | :---: | :---: | :---: | :---: |
| SIGN CODE | SIGN DESCRIPTION | NUMBER | SIGN SIZE | $\begin{array}{\|c\|} \hline \text { SQFT } \\ \text { PER SIGN } \end{array}$ | SQFT |
| R2-1 | SPEED LIMIT 65 | 4 | $36^{\prime \prime} \times 48^{\prime \prime}$ | 12.0 | 48.0 |
| R2-1 | SPEED LIMIT 45 | 2 | $36^{\prime \prime} \times 48^{\prime \prime}$ | 12.0 | 24.0 |
| R2-1 | SPEED LIMIT 80 | 2 | $36^{\prime \prime} \times 48^{\prime \prime}$ | 12.0 | 24.0 |
| R2-6aP | FINES DOUBLE (plaque) | 2 | 36" $\times 24$ " | 6.0 | 12.0 |
| W3-5 | SPEED REDUCTION AHEAD (65 MPH) | 2 | $48^{\prime \prime} \times 48^{\prime \prime}$ | 16.0 | 32.0 |
| W4-2 | LEFT or RIGHT LANE ENDS (symbol) | 2 | $48^{\prime \prime} \times 48^{\prime \prime}$ | 16.0 | 32.0 |
| W20-1 | Road work ahead | 2 | $48^{\prime \prime} \times 48^{\prime \prime}$ | 16.0 | 32.0 |
| W3-5 | SPEED REDUCTION AHEAD (45 MPH) | 2 | $48^{\prime \prime} \times 48^{\prime \prime}$ | 16.0 | 32.0 |
| W20-5 | LeFt or RIGHt LANE CLOSED AHEAD | 2 | $48^{\prime \prime} \times 48^{\prime \prime}$ | 16.0 | 32.0 |
| G20-2 | END ROAD WORK | 2 | $48^{\prime \prime} \times 24$ " | 8.0 | 16.0 |
|  |  | EXPRESSWAY / INTERSTATE TRAFFIC CONTROL SIGNS SQFT |  |  | 284.0 |

I29N NEAR 190 INTERCHANGE

|  |  | EXPRESSWAY / INTERSTATE |  |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: |
| $\overline{\text { SIGN }}$ CODE | SIGN DESCRIPTION | NUMBER | SIGN SIZE | $\begin{array}{\|c\|} \hline \text { SQFT } \\ \text { PER SIGN } \end{array}$ | SQFT |
| R2-1 | SPEED LIMIT 65 | 4 | 36" $\times 48^{\prime \prime}$ | 12.0 | 48.0 |
| R2-1 | SPEED LIMIT 45 | 2 | $36^{\prime \prime} \times 48^{\prime \prime}$ | 12.0 | 24.0 |
| R2-1 | SPEED LIMIT 80 | 2 | $36^{\prime \prime} \times 48^{\prime \prime}$ | 12.0 | 24.0 |
| R2-6aP | FINES DOUBLE (plaque) | 2 | $36^{\prime \prime} \times 24{ }^{\text {2 }}$ | 6.0 | 12.0 |
| W3-5 | SPEED REDUCTION AHEAD (65 MPH) | 2 | $48^{\prime \prime} \times 48^{\prime \prime}$ | 16.0 | 32.0 |
| W4-2 | LEFT or RIGHT LANE ENDS (symbol) | 2 | $48^{\prime \prime} \times 48^{\prime \prime}$ | 16.0 | 32.0 |
| W20-1 | ROAD WORK AHEAD | 2 | $48^{\prime \prime} \times 48^{\prime \prime}$ | 16.0 | 32.0 |
| W3-5 | SPEED REDUCTION AHEAD (45 MPH) | 2 | $48^{\prime \prime} \times 48^{\prime \prime}$ | 16.0 | 32.0 |
| W20-5 | LEFT or RIGHT LANE CLOSED AHEAD | 2 | $48^{\prime \prime} \times 48^{\prime \prime}$ | 16.0 | 32.0 |
| G20-2 | END ROAD WORK | 2 | $48^{\prime \prime} \times 24{ }^{\prime \prime}$ | 8.0 | 16.0 |
|  |  | EXPRESSWAY / INTERSTATE TRAFFIC CONTROL SIGNS SQFT |  |  | 284.0 |

SD34 IN HOWARD


I90W NEAR 1229 INTERCHANGE

|  |  |  | PRESSWAY | INTERSTA |  |
| :---: | :---: | :---: | :---: | :---: | :---: |
| $\begin{aligned} & \text { SIGN } \\ & \text { CODE } \end{aligned}$ | SIGN DESCRIPTION | NUMBER | SIGN SIZE | $\begin{array}{\|c\|} \hline \text { SQFT } \\ \text { PER SIGN } \\ \hline \end{array}$ | SQFT |
| R2-1 | SPEED LIMIT 65 | 4 | $36^{\prime \prime} \times 48^{\prime \prime}$ | 12.0 | 48.0 |
| R2-1 | SPEED LIMIT 45 | 2 | $36^{\prime \prime} \times 48^{\prime \prime}$ | 12.0 | 24.0 |
| R2-1 | SPEED LIMIT 80 | 2 | $36^{\prime \prime} \times 48^{\prime \prime}$ | 12.0 | 24.0 |
| R2-6aP | FINES DOUBLE (plaque) | 2 | 36" $\times 24$ " | 6.0 | 12.0 |
| W1-4 | REVERSE CURVE (L or R) | 2 | $48^{\prime \prime} \times 48^{\prime \prime}$ | 16.0 | 32.0 |
| W3-5 | SPEED REDUCTION AHEAD (65 MPH) | 2 | $48^{\prime \prime} \times 48^{\prime \prime}$ | 16.0 | 32.0 |
| W4-2 | LEFT or RIGHT LANE ENDS (symbol) | 2 | $48^{\prime \prime} \times 48^{\prime \prime}$ | 16.0 | 32.0 |
| W20-1 | ROAD WORK AHEAD | 2 | $48^{\prime \prime} \times 48^{\prime \prime}$ | 16.0 | 32.0 |
| W3-5 | SPEED REDUCTION AHEAD (45 MPH) | 2 | $48^{\prime \prime} \times 48^{\prime \prime}$ | 16.0 | 32.0 |
| W20-5 | LeFT or RIGHt LANE CLOSED AHEAD | 2 | 48" $\times 48^{\prime \prime}$ | 16.0 | 32.0 |
| G20-2 | END ROAD WORK | 2 | $48^{\prime \prime} \times 24{ }^{\prime \prime}$ | 8.0 | 16.0 |
|  EXPRESSWAY /INTERSTATE <br> TRAFFIC CONTROL SIGNS SQFT 316.0 |  |  |  |  |  |

1229S NEAR 190 INTERCHANGE

|  |  | EXPRESSWAY / INTERSTATE |  |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: |
| $\begin{aligned} & \text { SIGN } \\ & \text { CODE } \end{aligned}$ | SIGN DESCRIPTION | NUMBER | SIGN SIZE | $\begin{array}{\|c\|} \hline \text { SQFT } \\ \text { PER SIGN } \\ \hline \end{array}$ | SQFT |
| R2-1 | SPEED LIMIT 65 | 4 | $36^{\prime \prime} \times 48^{\prime \prime}$ | 12.0 | 48.0 |
| R2-1 | SPEED LIMIT 45 | 2 | $36^{\prime \prime} \times 48^{\prime \prime}$ | 12.0 | 24.0 |
| R2-6aP | FINES DOUBLE (plaque) | 2 | $36^{\prime \prime} \times 24^{\prime \prime}$ | 6.0 | 12.0 |
| W3-5 | SPEED REDUCTION AHEAD (65 MPH) | 2 | $48^{\prime \prime} \times 48^{\prime \prime}$ | 16.0 | 32.0 |
| W4-2 | LEFT or RIGHT LANE ENDS (symbol) | 3 | $48^{\prime \prime} \times 48^{\prime \prime}$ | 16.0 | 48.0 |
| W4-3 | ADDED LANE (symbol) | 1 | $48^{\prime \prime} \times 48^{\prime \prime}$ | 16.0 | 16.0 |
| W20-1 | ROAD WORK AHEAD | 4 | $48^{\prime \prime} \times 48^{\prime \prime}$ | 16.0 | 64.0 |
| W3-5 | SPEED REDUCTION AHEAD (45 MPH) | 2 | $48^{\prime \prime} \times 48^{\prime \prime}$ | 16.0 | 32.0 |
| W20-5 | LeFT or RIGHT LANE CLOSED AHEAD | 3 | $48^{\prime \prime} \times 48^{\prime \prime}$ | 16.0 | 48.0 |
| G20-2 | END ROAD WORK | 2 | $48^{\prime \prime} \times 24^{\prime \prime}$ | 8.0 | 16.0 |
|  |  | EXPRESSWAY /INTERSTATETRAFFIC CONTROL SIGNS SQFT |  |  |  |

















GENERAL NOTES:
The top of anchor posts and slip bases WILL NOT extend above a $60^{\prime \prime}$ chord line within a 120" diameter circle around the post with ends 4 " above the ground

At locations where there is curb and gutter adjacent to the breakaway sign support, the stub height will be maximum of 4 "above the ground line at the localized area adjacent to the breakaway support stub.
The 4" stub height clearance is not necessary for U-channel lap splices where the support is designed to
yield (bend) at the base.




[^0]:    Where no Longitudinal or Transverse joints are

