

CIVIL ENGINEERING REPORT: TRAFFIC IMPACT ASSESSMENT

# **Casey Apartments**

Block 9 Section 132, Casey

PREPARED FOR Jega LG08/17 The Causeway Kingston ACT 2604

Ref: CR220895\_EC01 Rev: 1.3 Date: 23.01.2022 G



# Traffic Impact Assessment Report

# **Revision Schedule**

Date	Revision Issue Pre		Prepared By	Approved By
14.09.2022	1.0	Development Application	N.Grinter	M.Pike
07.10.2022	1.1	Development Application V1	N.Grinter	M.Pike
21.12.2022	1.2	Development Application V2	N.Grinter	M.Pike
23.01.2023	1.3	Development Application V2	N.Grinter	M.Pike

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# 1. Introduction

# 1.1 Purpose of Report

Northrop Consulting Engineers Pty Ltd (Northrop) have been engaged by Jega to prepare a Traffic Impact Assessment (TIA) for the proposed development on Block 9 Section 132, Casey (referred to as to the subject site in this report).

This TIA investigates the impact which the additional traffic to the area will have on the current surrounding vicinity.

# 1.2 Study Objectives

This TIA is in line with the intent of the ACT Government Transport Canberra and City Services Directorate (TCCS) Guidelines for Transport Impact Assessment (Version 3.1, April 2020) as well as the Austroads Guide to Traffic Management Part 12: Integrated Transport Assessments for Developments (2020).

This TIA will detail the below:

- An introduction to the report and summary of the proposed development;
- A summary of the development site and nearby conditions;
- An investigation in the existing conditions of the site and key roads including:
  - Traffic Volumes and conditions at key intersections;
  - Public transport within the vicinity of the site;
  - Active travel within the vicinity of the site.
- A summary of the projected traffic and parking conditions from the proposed development and surrounding key roads and intersections including:
  - The trip generation, trip distribution, modal split and trip assignment for the site generated traffic;
  - The increase of traffic at the key intersections;
  - The car park generation on site against the amount of car parking required; and
  - Car park compliance commentary.
- A transportation analysis including:
  - o Commentary on proposed site access locations;
  - Commentary on the SIDRA Intersections models completed by Northrop for the key intersections for the base case, development conditions and future conditions for the site; and
  - Commentary on the current accident data for the key roads near the site supplied from the Transport Canberra and City Services Directorate (TCCS).
- A summary of the findings regarding:
  - Site accessibility;
  - Transportation impacts; and
  - Parking impacts.

The key intersections for this TIA are:

- Kingsland Parade and Bentley Place;
- Kingsland Parade and Clarrie Hermes Drive; and
- Horse Park Drive and Overall Avenue.



The key roads for this TIA are:

- Kingsland Parade between Clarrie Hermes Drive and Dallin Crescent North; and
- Bentley Place.

# 1.3 References

In preparation of this report, reference has been made to the following:

- Inspections of the site and its surroundings;
- ACT Government TCCS Guidelines for Transport Impact Assessment (Version 3.1, April 2020);
- Austroads Guide to Traffic Management Part 12: Integrated Transport Assessments for Developments (2020)
- Traffic surveys as undertaken by Matrix Traffic and Transport Data as referenced in the context of this report;
- AS2890.1:2004 Parking facilities Part 1: Off-street car parking;
- AS2890.2:2018 Parking facilities Part 2: Off-street commercial vehicle facilities;
- ACT Planning and Land Authority Parking and Vehicular Access General Code (June, 2022);
- NSW Transport Roads & Maritime Services Guide to Traffic Generating Developments Updated Traffic Surveys (August, 2013);
- NSW Transport Roads & Maritime Services (RTA) Guide to Traffic Generating Developments – V2.2 (October, 2002); and
- Other documents as referenced by this report.



# 2. Proposed Development

# 2.1 Surrounding Area

The proposed development is located within the suburb of Casey in Canberra. In the vicinity of the subject site, the surrounding land use is generally residential in nature. Nearby points of interests include the following:

- The Casey Market Town Shops at Block 3, Section 131 Casey;
- The car park available to the public located at Block 10, Section 132 Casey; and
- The 7 Eleven Petrol Station located at Block 6, Section 132 Casey.

The nearby points of interest and their location in relation to the site are shown in Figure 1.

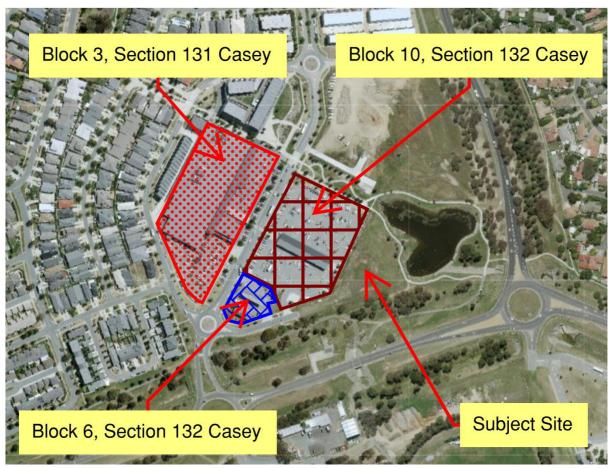
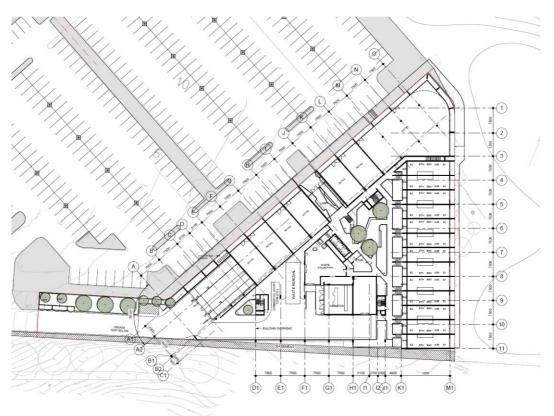


Figure 1 Nearby Points of Interest to the Site

# 2.2 Development Description and Locality

The proposed development will include a new multi-storey mixed-use building comprising of commercial/retail space, 219 residential units, bicycle parking and basement parking. An extract of drawing DA-20-04 Revision 2 by Cox Architecture (dated 21/09/2022) is shown in Figure 2. The proposed development is anticipated to have a gross floor area (GFA) of approximately 27,914m2 in accordance with drawing DA-01-02 Revision 3 by Cox Architecture (dated 21/09/2022).





# Figure 2 – Proposed Development Upper Ground Plan

The subject site is located on Block 9, Section 132 Casey which is off Bentley Place. Figure 3 shows the general site within the Casey area and Figure 4 shows the site-specific location.



Figure 3 Site Locality within Casey (Source: Metromaps, February 2022)





Figure 4 Site Specific Location (Source: ACTMapi, February 2022)

Figure 5 illustrates the locality of the subject site in relation to the key roads and intersections outlined in Section 1.2 of this report. The key intersections have a red circle on them for identification purposes in Figure 5.



Figure 5 Key Roads and Intersections in relation to the Subject Site (Source: ACTMapi, February 2022)



# 3. Existing Conditions

# 3.1 Study Area

# 3.1.1 Area of Influence

For the purpose of this TIA Report, the key roads and intersections of interest are as follows:

# Key Roads:

- Kingsland Parade between Clarrie Hermes Drive and Dallin Crescent; and
- Bentley Place.

# **Key Intersections:**

- Kingsland Parade and Bentley Place;
- Kingsland Parade and Clarrie Hermes Drive; and
- Horse Park Drive and Overall Avenue.

# 3.2 Study Area Land Use

### 3.2.1 Existing Land Use

At the time of this TIA, the subject site is a vacant block of land.

#### 3.2.2 Existing Zoning

The subject site is zoned in the ACT Government Territory Plan as CZ1: Core Zone. Northrop understand that the land is within the Casey Group Centre.

# 3.3 Site Accessibility

### 3.3.1 Area Roadway System

# 3.3.1.1 Existing Roads Hierarchy

The subject site can be accessed via Bentley Place from the Eastern leg of the Kingsland Parade and Bentley Place intersection. The hierarchy of these roads are defined as follows in accordance with Active Travel Infrastructure Planning Map (accessed on the 18/12/2022):

- Kingsland Parade Minor Collector
- Bentley Place Local Access Street (Access Street)

TCCS Municipal Infrastructure Standards (MIS) 01 Street Planning and Design Edition 1 Revision 1 described a Minor Collector and an Access Road as follows:

- **Minor Collector:** Minor collector roads distribute traffic from Access Streets to Major Collector or Arterial Roads.
- Access Streets: Access Streets are used where the residential environment is dominant, traffic is subservient, speed and volumes are low and pedestrian and cycle movements are facilitated.



# 3.3.1.2 Existing Traffic Infrastructure and Traffic Controls for the Key Roads

# 3.3.1.2.1 Kingsland Parade

Between Clarrie Hermes Drive and Dallin Crescent (North), Kingsland Parade is aligned in a North/South direction. It is a two lane – two way road with a carriageway of an approximate width of 10.0m. Kingsland Parade between Clarrie Hermes Drive and Dallin Crescent (North) has a posted speed limit of 40 km/h.

There are indented parking bays (90 degree and parallel bays) as well as two bus laybys along this section of Kingsland Parade.

#### 3.3.1.2.2 Bentley Place

Bentley place is aligned in an East/West direction. It is a two lane - two way road with a carriageway of an approximate width of 7.0m. Bentley Place between Kingsland Parade and the subject site has a posted speed of 40km/h.



# 3.4 Traffic Volumes and Conditions

#### 3.4.1 Current Traffic Volumes

Matrix Traffic and Transport Data (Matrix) were engaged by Northrop to undertake a traffic survey for the key intersections over a Thursday between 6am to 10am and 2:30pm to 6:30pm and a Saturday between 7am to 3pm.

# 3.4.1.1 Current Traffic Volumes at the Intersection of Kingsland Parade, Dallin Crescent and Bentley Place

The traffic survey outlining the traffic volumes at the roundabout intersection of Kingsland Parade, Dallin Crescent and Bentley Place for the 27/10/2022 (Thursday) and the 29/10/2022 (Saturday) are summarised in Table 1 and Table 2 respectively.

Road	Location	Direction	AM Volume (as surveyed)	PM Volume (as surveyed)	AM Peak Volume (veh/hr)	PM Peak Volume (veh/hr)
Kingsland	Northern	Northbound	737	1,529	226	438
Parade	Leg	Southbound	559	877	221	240
Bentley	Eastern Leg	Eastbound	41	139	12	46
Place		Westbound	425	702	122	217
Kingsland	Southern	Northbound	703	1,566	217	447
Parade	Leg	Southbound	1,003	1,416	354	388
Dallin	Western	Eastbound	214	192	83	57
Crescent	Leg	Westbound	120	253	51	89

#### Table 1: Traffic Volumes on Thursday 27/10/2022

#### Note:

- 1. The AM and PM Peak Periods have been identified to be between 8:15am to 9:15am and 5:30pm to 6:30pm respectively.
- 2. The entry to Bentley Place has a low vehicular usage compared with the remainder of the legs of the intersection.



#### Table 2: Traffic Volumes on Saturday 29/10/2022 2022

Road	Location	Direction	8-hour Volume (as surveyed)	AM Peak Volume (veh/h)	PM Peak Volume (veh/hr)
Kingsland	Northern	Northbound	2,198	308	357
Parade	Leg	Southbound	1,477	227	217
Doubles: Diese	Eastern Leg	Eastbound	215	38	38
Bentley Place		Westbound	1,167	178	219
Kingsland	Southern Leg	Northbound	2,249	331	388
Parade		Southbound	2,517	394	400
Dallin	Western	Eastbound	400	65	40
Crescent	Leg	Westbound	363	61	69

# Note:

1. The AM and PM Peak Periods have been identified to be between 11:00am to 12:00pm and 12:15pm to 1:15pm respectively.

2. The entry to Bentley Place has a low vehicular usage compared with the remainder of the legs of the intersection.



# 3.4.1.2 Current Traffic Volumes at the Intersection of Kingsland Parade and Clarrie Hermes Drive

The traffic survey outlining the traffic volumes at the roundabout intersection of Kingsland Parade and Clarrie Hermes Drive for the 10/11/2022 (Thursday) and the 12/11/2022 (Saturday) are summarised in Table 3 and Table 4 respectively.

Road	Location	Direction	AM Volume (as surveyed)	PM Volume (as surveyed)	AM Peak Volume (veh/hr)	PM Peak Volume (veh/hr)
Kingsland	Northern	Northbound	636	1,571	189	405
Parade	Leg	Southbound	979	1,404	299	353
Clarrie Hermes	Eastern Leg	Eastbound	2,021	3,002	718	782
Drive		Westbound	2,313	3,257	737	950
Playing Field	Southern	Northbound	63	129	33	64
Access	Leg	Southbound	66	184	38	45
Clarrie Hermes	Western	Eastbound	1,823	3,261	630	859
Drive	Leg	Westbound	2,455	3,294	754	994

### Table 3: Traffic Volumes on Thursday 10/11/2022

Note:

1. The AM and PM Peak Periods have been identified to be between 8:00am to 9:00am and 3:15pm to 4:15pm respectively.

2. Insignificant traffic utilised the Southern let of the intersection during the surveyed periods.

3. Clarrie Hermes Drive has the largest number of vehicles travelling along it at this intersection.



# Table 4: Traffic Volumes on Saturday 12/11/2022 2022

Road	Location	Direction	8-hour Volume (as surveyed)	AM Peak Volume (veh/h)	PM Peak Volume (veh/hr)
Kingsland	Northern Leg	Northbound	2,289	368	380
Parade	Norment Leg	Southbound	2,584	397	428
Clarrie Hermes	Eastern Leg	Eastbound	4,695	688	737
Drive		Westbound	5,002	754	824
Playing Field	Southern	Northbound	213	17	75
Access	Leg	Southbound	222	23	28
Clarrie Hermes	Western Leg	Eastbound	4,658	704	708
Drive	western Leg	Westbound	5,251	797	888

# Note:

1. The AM and PM Peak Periods have been identified to be between 11:00am to 12:00pm and 12:00pm to 1:00pm respectively.

2. Insignificant traffic utilised the Southern let of the intersection during the surveyed periods.

3. Clarrie Hermes Drive has the largest number of vehicles travelling along it at this intersection.

# NORTHROP

# 3.4.1.3 Current Traffic Volumes at the Intersection of Horse Park Drive and Overall Avenue

The traffic survey outlining the traffic volumes at the roundabout intersection of Horse Park Drive and Overall Avenue for the 27/10/2022 (Thursday) and the 29/10/2022 (Saturday) are summarised in Table 5 and Table 6 respectively.

Road	Location	Direction	AM Volume (as surveyed)	PM Volume (as surveyed)	AM Peak Volume (veh/hr)	PM Peak Volume (veh/hr)
Horse Park	Northern	Northbound	1,578	2.860	532	900
Drive	Leg	Southbound	2,476	2,306	967	615
Newlop	Eastern Leg	Eastbound	195	517	79	166
Street		Westbound	451	455	178	145
Horse Park	Southern Leg	Northbound	1,398	2,923	498	913
Drive		Southbound	2,560	2,079	951	533
Overall	Western	Eastbound	886	1,141	340	345
Avenue	Leg	Westbound	878	1,369	421	419

### Table 5: Traffic Volumes on Thursday 27/10/2022

Note:

1. The AM and PM Peak Periods have been identified to be between 8:00am to 9:00am and 5:15pm to 6:15pm respectively.

2. The majority of traffic flows along Horse Park Drive.

3. Vehicles appear to be rat running through Overall Avenue to avoid the arterial road network. The project team notes that this is not part of the scope of this TIA and recommends Road ACT consider further investigation of the traffic volumes and speeds along Overall Avenue separate to this study.



#### Table 6: Traffic Volumes on Saturday 29/10/2022 2022

Road	Location	Direction	8-hour Volume (as surveyed)	AM Peak Volume (veh/h)	PM Peak Volume (veh/hr)
Horse Park Drive	Northern	Northbound	3,272	490	523
	Leg	Southbound	4,139	665	563
Newlop	Eastern Leg	Eastbound	695	131	119
Street		Westbound	812	111	119
Horse Park	Southern	Northbound	3,199	503	543
Drive	Leg	Southbound	4,028	628	550
Overall	Western	Eastbound	1,755	280	260
Avenue	Leg	Westbound	1,910	310	293

#### Note:

- 1. The AM and PM Peak Periods have been identified to be between 11:00am to 12:00pm and 12:00pm to 1:00pm respectively.
- 2. The majority of traffic flows along Horse Park Drive.

# 3.4.2 Current Condition of Key Intersections

The key intersections have been modelled using SIDRA Intersection 9.1. The Thursday results have been modelled due to the higher traffic volumes across the three sites being experienced on the Thursday.

For consistency, the peak hours for intersection modelling have been assumed to be 8:00am – 9:00am and 5:15pm – 6:15pm which align to the general travel times for the average working day.

A summary of the SIDRA Intersection 9.1 results are in the following sections. The results listed in this TIA include the level of service (LOS), degree of saturation (DOS), average queue length and average delay. These results provide a quantitative measure of the performance of the intersection for the period modelled.

The LOS is a rating from A (best operating conditions) to F (worst operating conditions) as described by Austroads. In accordance with the RTA Guide to Traffic Generating Developments (Version 2.2, 2022), a LOS A represents good operation of a roundabout while LOS F represents a roundabout which is at capacity. The LOS provided in the summary of results is based on the delay method for New South Wales. The LOS has been provided to allow authority analysis of the intersection in relation to intent of the ACT Government Transport Canberra and City Services Guidelines for Transport Impact Assessment (3.1 Version, April 2020). A description of the LOS for roundabouts is provided in Table 7.



#### Table 7 LOS Summary for Roundabouts

LOS	Average Delay per Vehicle (seconds per vehicle)	Roundabout Services Description
А	< 14	Good operation
В	15 to 28	Good with acceptable delays and space capacity
С	29 to 42	Satisfactory
D	43 to 56	Operating near capacity
E	57 to 70	At capacity
F	> 70	Over capacity

The DOS is the ratio of arrival flow of vehicles to the capacity of the leg of the intersection. The DOS has been provided to assist indicate the available capacity of the intersection/leg of the intersection.

The average queue length represents a line of vehicles waiting to proceed through an intersection including slow moving vehicles at the back of the queue. The average queue length has been provided to indicate potential effects on the surrounding road network.

The average delay, for the purpose of this TIA, is the control delay which accounts for the time lost during the negotiation of an intersection including all stop-start and slow down delays and stopping times. The average delay will be in line with the LOS provided. The average delay has been provided as a metric to show an increase in waiting times anticipated based on the results of the SIDRA Intersection 9.1 model.

Assumptions and adjustments for the SIDRA models in the following sections are as follows:

- Grading for the legs of the intersection have been left at 0% as no survey has been provided.
- Lane geometry accuracy has limitation due to the options available in the program. These have been modelled to reflect the conditions as reasonably practical.
- Initial queue demand has been set as 0.0 veh.
- No pedestrians have been modelled at the intersections to reflect observations made by Northrop on 15/12/2022. Northrop understand that pedestrians cross midblock at locations along Clarrie Hermes Drive and Kingsland Parade which is beyond the parameters of the modelling.
- Cyclists have not been modelled to reflect observations made by Northrop on 15/12/2022.



# 3.4.2.1 Current Traffic Conditions at the Intersection of Kingsland Parade, Dallin Crescent and Bentley Place

Table 8 contains the summary of results from the SIDRA Intersection 9.1 model for the current conditions of the intersection of Kingsland Parade, Dallin Crescent and Bentley Place.

Road	Leg	Period	LOS	DOS	Average Queue Length (m)	Average Delay (s)
Kingsland	North	AM	А	0.17	3	1.8
Parade	North	PM	А	0.18	3	2.1
Bentley	East	AM	А	0.12	1	3.4
Place		PM	А	0.19	3	3.5
Kingsland	South	AM	А	0.15	2	1.7
Parade	3000	PM	А	0.35	6	2.1
Dallin	W/oct	AM	А	0.08	1	5.6
Crescent	West	PM	А	0.08	1	6.2

# Table 8 Summary SIDRA Intersection Results

In line with the summary of the results from the current conditions:

• The intersection has good operation.



# 3.4.2.2 Current Traffic Conditions at the Intersection of Kingsland Parade and Clarrie Hermes Drive

Table 9 contains the summary of results from the SIDRA Intersection 9.1 model for the current conditions of the intersection of Kingsland Parade and Clarrie Hermes Drive.

Road	Leg	Period	LOS	DOS	Average Queue Length (m)	Average Delay (s)
Kingsland	North	AM	А	0.31	6	4.9
Parade	North	PM	А	0.43	9	6.1
Clarrie Hermes	East	AM	А	0.51	12	6.4
Drive		PM	А	0.58	14	7.5
Playing Field	South	AM	А	0.04	1	9.2
Access	30001	PM	А	0.03	1	8.0
Clarrie	West	AM	А	0.43	8	5.4
Hermes Drive	West	PM	А	0.63	16	6.0

# **Table 9 Summary SIDRA Intersection Results**

In line with the summary of the results from the current conditions:

• The intersection has good operation.

Northrop acknowledge that observations on the 10/11/2022 showed that there were delays for the vehicles travelling East to West across the intersection of Clarrie Hermes Drive and Kingsland Parade. It was observed that the delays were experienced between 8:00am - 8:55am. Before and after the noted times, the delays were no longer observed.

It is noted that the delays observed coincided with Gold Creek School's Senior Site. There is also a signalised pedestrian crossing to the West of the intersection of Clarrie Hermes Drive and Kingsland Parade which may have caused delays to vehicles.

Modelling shows that the intersection has sufficient capacity for the current traffic volumes. It should be noted that intersection performance may be impacted by other parts of the network which may influence intersection operation. This TIA addresses the scope of a TIA as outlined in the ACT Government TCCS Guidelines for Transport Impact Assessment (Version 3.1, April 2020) as it provides commentary on the impact of the development on the surrounding intersections including the intersection of Kingsland Parade and Clarrie Hermes Drive.

Table 9 contains the results for the intersection which shows the "current conditions" as the base case, to which the impact of the development can be compared.



Northrop recommends that Roads ACT complete a study on the intersection to review the wider network issues in relation to the intersection of Kingsland Parade and Clarrie Hermes Drive separate to this TIA and implement appropriate outcomes as identified by their study.

# 3.4.2.3 Current Traffic Conditions at the Intersection of Horse Park Drive and Overall Avenue

Table 10 contains the summary of results from the SIDRA Intersection 9.1 model for the current conditions of the intersection of Horse Park Drive and Overall Avenue.

Road	Leg	Period	LOS	DOS	Average Queue Length (m)	Average Delay (s)
Horse Park	North	AM	А	0.87	48	14.4
Drive	NOTUT	PM	А	0.55	13	9.2
Newlop	East	AM	В	0.28	7	15.1
Street		PM	А	0.11	2	6.5
Horse Park	South	AM	А	0.56	14	9.8
Drive	South	PM	В	0.88	49	15.5
Overall	West	AM	А	0.35	6	7.2
Avenue	West	PM	А	0.57	15	11.1

#### **Table 10 Summary SIDRA Intersection Results**

In line with the summary of the results from the current conditions:

• The intersection has good operation.



# 3.5 Carparking

There are approximately 412 publicly available car parking spaces along Kingsland Parade and within the on-grade parking on Block 10 Section 132, Casey. The breakdown of these car parking spaces is summarised in Sections 3.5.1 to 3.5.2 of this report.

#### 3.5.1 Kingsland Parade

The number of carparking spaces on Kingsland Parade are as follows:

- 2 x Disabled Parking Spaces;
- 31 x 90 Degree Indented Parking Bays (1P); and
- 3 x 3 Parallel Indented Parking Bays (Unrestricted).

#### 3.5.2 On-grade Parking on Block 10 Section 132, Casey

The number of carparking spaces on Block 10 Section 132, Casey are as follows:

- 12 x Disabled Parking Spaces;
- 366 x Car Parking Bays (a combination of all day parking, 3-hour parking, 1 hour parking and 30minute parking); and
- 21 x Motorcycle Bays.

#### 3.5.3 Car Parking Survey

A car and motorcycle parking survey was undertaken during the period from 24/11/2022 through to 26/11/2022. Various times were captured over the 3 days.

General observations from the photos taken from the survey include:

- No motorcycles were observed to be parked in the designated motorcycle parking on Block 10, Section 132 Casey across the 3 days during the survey periods;
- Parking along Kingsland Parade was almost at capacity during the survey during the survey periods and has not been further considered; and
- Parking on Block 3, Section 131 Casey was almost at capacity during the survey during the survey periods and has not been further considered.

It is noted that parking along Kingsland Parade and parking on Block 3, Section 131 Casey are within close proximity to Casey Market Town shops.

Table 11 provides a summary of the car park capacity on Block 10, Section 132 Casey during the survey period.



#### Table 11 Car Parking Survey Summary

Day	Time	No. Cars Parked	No. Available Car Parking Spaces	Percent Available Car Parking Spaces
	10:45am	137	241	63%
Thursday	11:45am	151	227	60%
24/11/2022	12:35pm	176	202	53%
	1:30pm	169	209	55%
	2:00pm	171	207	54%
	3:00pm	127	251	66%
Friday 25/11/2022	4:00pm	161	217	57%
	5:00pm	185	193	51%
	6:00pm	187	191	50%
	10:00am	180	198	47%
	11:00am	186	192	50%
	12:00 Midday	202	176	46%
Saturday 26/11/2022	1:00pm	215	163	43%
	2:00pm	189	189	50%
	3:00pm	182	196	51%
	4:00pm	149	229	60%

In line with the car and motorcycle parking survey undertaken:

- The Saturday was the busiest day at the car park;
- The least amount of car parking available during the surveyed times was 163 car parking spaces;
- The most amount of car parking available during the surveyed times was 251 on a Friday.

Figure 6 and Figure 7 showing the car park at its busiest time and quietest time surveyed respectively.





Figure 6 Car Parking Available Saturday 26/11/2022 1:00pm



Figure 7 Car Parking Available Friday 25/11/2022 3:00pm



# 3.6 Public Transport

At the time of this report, there are two bus stops situated on Kingsland Parade (Stop ID 6109 and Stop ID 6110) as shown location E in Figure 8 and Location A in Figure 9. These bus stops serve Bus Routes 25, 26, 27 and 28.

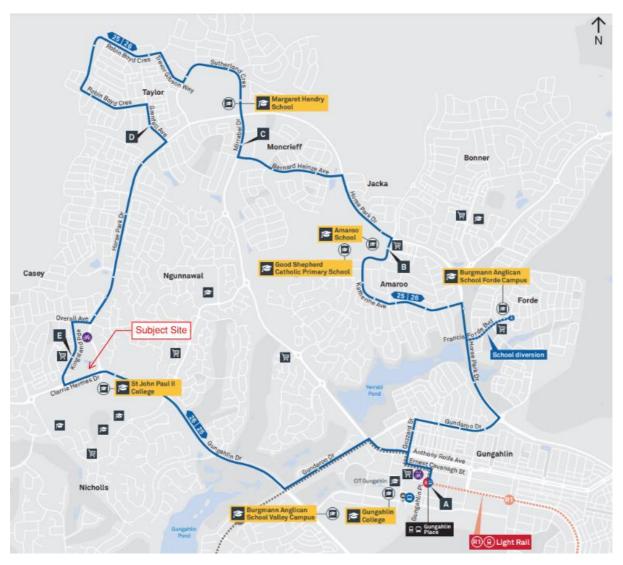


Figure 8 Route 25 and 26



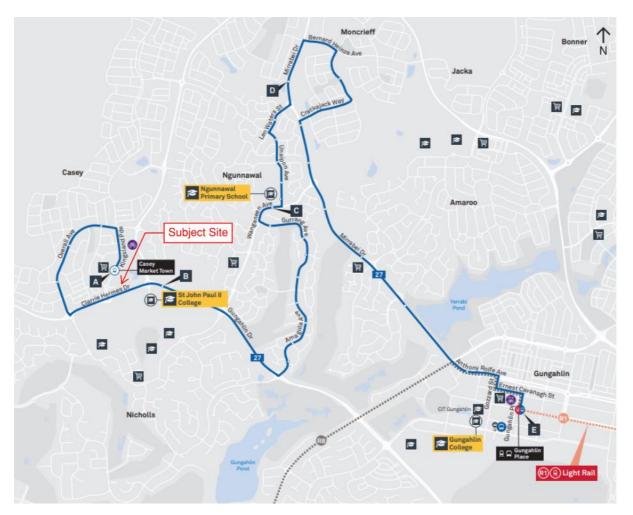


Figure 9 Route 27 and 28

Routes 25 and 26 stop at Kingsland Parade in line with the current weekday and weekend timetable. The buses depart approximately every 30 minutes during weekdays and approximately every 1 hour in the morning on Saturday's until 10:45, then every 2 hours for the rest of the weekend's timetable. It is noted that Routes 25 and 26 link with the Gungahlin Interchange.

Routes 27 and 28 stops at Kingsland Parade in line with the current weekday and weekend timetable. The buses depart approximately every 30 minutes during weekdays and approximately every hour on weekends in the morning and every 2 hours on for the rest of the weekend's timetable. It is noted that Routes 27 and 28 link with the Gungahlin Interchange.

Gungahlin Interchange provides connectivity to City Interchange and Belconnen through Transport Canberra's Rapid Routes as well as providing connection to other bus routes.



# 3.7 Active Travel

Public footpaths follow the verges of the streets surrounding the subject site providing access for both pedestrians and cyclists. Figure 10 is a marked-up extract from the Active Travel Practitioner Tool which indicates the existing pedestrian facilities in the vicinity of the subject site.

The public footpaths shown in Figure 10 link to the greater pedestrian footpath and active travel facilities for the greater Canberra region.

Underpasses linking Casey to Ngunnawal and Nichols are identified in Figure 10 by the red circles. The underpasses provide an alternate route for pedestrians, cyclists and users of other active travel types to miss negotiating Horse Park Drive and Clarrie Hermes Drive.

A signalised pedestrian crossing linking Casey to Nichols has been identified in Figure 10 by the orange circle. The signalised crossing enable pedestrian to cross Clarrie Hermes Drive in a controlled manner.



Figure 10 Pedestrian Footpath Infrastructure in the Vicinity of the Subject Site



# 3.8 Accident Data

Accident data has been obtained from TCCS for the period of 1/01/2016 to 31/12/2020 for the following locations:

- Bentley Place between Kingsland Parade and the Subject Site
- Kingsland Parade between Clarrie Hermes Drive and Dallin Crescent (North)

The data collected provides a typical 5-year behaviour for accidents within the abovementioned locations and is summarised in Table 12.

#### Table 12 Accident Data Along Clarrie Hermes Drive, Bentley Place, Kingsland Parade & Dallin Crescent

Location	Type of Accident	Number of Accidents
Intersection of Clarrie Hermes Drive and Kingsland Parade	Injury	0
	Property Damage	24
Midblock of Kingsland Parade between Bentley Place and	Injury	0
Dallin Crescent	Property Damage	12

#### Note:

- 1. No accidents have been recorded in the mid-block of Bentley Place.
- 2. No accidents have been recorded at the midblock of Kingsland Parade between Clarrie Hermes Drive and Bentley Place.
- 3. No accidents have been recorded at the intersection of Bentley Place, Kingsland Parade and Dallin Crescent.
- 4. No accidents have been recorded at the intersection of Dallin Crescent and Kingsland Parade.

It is noted that of the accidents recorded at the intersection of Clarrie Hermes Drive and Kingsland Parade, 58% were recorded to be rear end related (the most common crash type). Information as provided by the ACT Government does not allow a root cause for these accidents to be identified nor potential improvements to the area to be recommended. This would need to be completed as a separate study to this TIA.

It is noted that of the accidents recorded at the midblock of Kingsland Parade between Bentley Place and Dallin Crescent, 42% were recorded to be related to leaving parking spaces (the most common crash type). Information as provided by the ACT Government does not allow a root cause for these accidents to be identified nor potential improvements to the area to be recommended. This would need to be completed as a separate study to this TIA.

Under the Federal Government's Black Spot Program, for an area to be defined as a Black Spot Road (midblock or intersection) requiring modification, the road in question is required to meet the following condition:

"For individual sites such as intersections, mid-block or short road sections, there should be a history of at least three casualty crashes over a five-year period. For lengths of road, there should be an average of 0.2 casualty crashes per kilometre per annum over the length in question over five years."

In line with the data presented in Table 12, there are no black spots along Clarrie Hermes Drive, Bentley Place, Kingsland Parade and Dallin Crescent.



# 4. Proposed Development

# 4.1 Development Description

In line with Cox Architecture drawing DA-01-02 Revision 3 dated 21/09/2022, the development is a mixed-use building which contains the following:

- 6 x studio apartments;
- 18 x 1 bedroom apartments;
- 133 x 2 bedroom apartments;
- 19 x 3 bedroom apartments;
- 20 x 3 bedroom apartments with a study;
- 10 x 4 bedroom apartments with a study;
- 13 adaptive commercial apartments (for the purpose of this study, they have been modelled as 2 bedrooms); and
- 1,087m2 of retail space.

Northrop understand that the retail will be a split of approximately 478m2 shop style tenancies, 400m2 office style tenancies and 200m2 café/restaurant style tenancies.

# 4.2 Access

### 4.2.1 Driveway

Driveway access to the proposed development is off Bentley Place.

With the arrangement of the site in line with Cox Architecture drawings DA-20-04 Rev 2 (dated 21/09/2022) and DA-20-03 Rev 2 (dated 12/09/2022), there is approximate 70m between the block boundary and the basement parking control point. This exceeds the requirements of AS2890.1 for minimum queue lengths at a car park with a control point at the entrance.

# 4.3 Compliance to Relevant Standards

Northrop note that a full compliance review of the car park is required prior to construction of the proposed development, however can note from a preliminary review:

- Car parking spaces in blind aisles must be assigned to users;
- The basement car park mainly meets the intent of AS2890.1:2004 for residential parking and employee parking;
- A full review of the car park must be undertaken in the detailed design phase of the project to ensure full compliance to relevant standards, codes and guidelines.

# 4.4 Traffic Generation

The peak traffic generation has been based on the:

- ACT Government Environment and Sustainable Development Estate Development Code (28 August, 2020);
- RTA Guide to Traffic Generating Developments Version 2.2 (October, 2022); and
- NSW Government Transport Roads and Maritime Services Guide to Traffic Generating Developments Updated Traffic Surveys TDT 2013/04a (August, 2013).

The ACT Government Environment and Sustainable Development Estate Development Code (26 August, 2020) advises multiunit dwellings generate 6 vehicle movements per day per dwelling. As the



code is silent on peak hour traffic generation and other type of peak vehicle generation, we have referred to the RTA Guide to Traffic Generating Developments Version 2.2 (October, 2022) and the NSW Government Transport Roads and Maritime Services Guide to Traffic Generating Developments Updated Traffic Surveys TDT 2013/04a (August, 2013).

The RTA Guide to Traffic Generating Developments Version 2.2 (October, 2022) provides traffic generation rates for restaurants (which also reflect a similar behaviour to cafes).

The NSW Government Transport Roads and Maritime Services Guide to Traffic Generating Developments Updated Traffic Surveys TDT 2013/04a (August, 2013) provides traffic generation rates for high density residential flat dwellings, office blocks and retail (assumed shopping centre in nature due to shops in vicinity of the development).

It is noted that for the purpose of this TIA and the traffic generation, the commercial adaptive units will be classified as 2 bedroom units.

The traffic generation from the development is summarized in Table 13.

Usage	No. of	Peak Generation Rate₃	Peak Traffic	Daily Traffic Generation <sub>3</sub>	Daily Traffic
Residential	219	0.67 vehicle trips/unit <sub>1</sub>	147	6 vehicle trips/unit <sub>3</sub>	1,314
Restaurant/ Café	200m2	5 vehicle trips /100m2 GFA <sub>2</sub>	10	60 vehicle trips/100m2 GFA <sub>2</sub>	120
Office	400m2	1.57 vehicle trips/100m2 GFA <sub>1</sub>	7	18.17 vehicle trips/100m2 GFA <sub>1</sub>	73
Shop	478m2	6.99 vehicle trips/100m2 GFA	34	60.67 vehicle trips/100m2 GFA <sub>1</sub>	290

#### **Table 13 Development Traffic Generation**

 As per NSW Government Transport Roads and Maritime Services Guide to Traffic Generating Developments Updated Traffic Surveys TDT 2013/04a (August, 2013) using the most conservative regional value.

2. As per RTA Guide to Traffic Generating Developments Version 2.2 (October, 2022).

3. As per the ACT Government Environment and Sustainable Development Estate Development Code (26 August, 2020).

# 4.5 Traffic Distribution

As per section 3.4.1 of this report, the weekday peak hour periods for the intersection of Kingsland Parade and Bentley Place are:

- 8:00am 9:00am; and
- 5:15pm 6:15pm.



During the am peak period, it would be assumed the development would have 20% incoming traffic and 80% outgoing traffic.

During the pm peak period, it would be assumed the development would have 80% incoming traffic and 20% outgoing traffic.

With the acknowledgement of likely origins and destinations for vehicles in close proximity to the proposed development site, the following traffic distribution has been modelled as per Figure 11, Figure 12 and Figure 13. The traffic distribution shown in Figure 11 has taken into consideration the following sites which represent a local shopping hub, major public transport hub, education and the typical work locations in Canberra. It has been assumed that most drivers will travel East along Clarrie Hermes Drive to travel towards City/Belconnen/Woden/Fyshwick/Tuggernong:

- Gungahlin to the East;
- Gold Creek Highschool to the South;
- St John Paul II College to the South;
- Ngunnawal Primary School to the North East;
- City/Belconnen/Woden/Fyshwick/Tuggernong to the South.

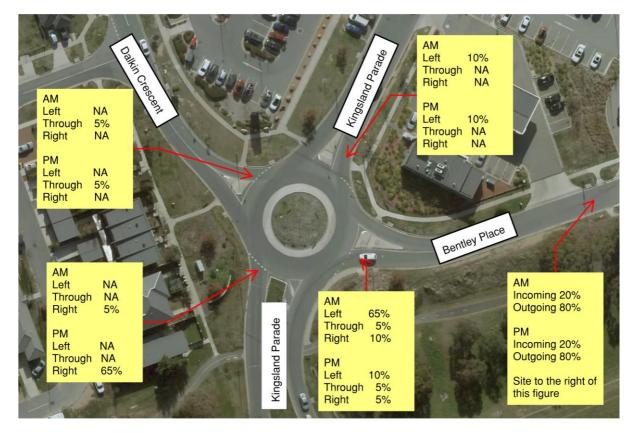


Figure 11 Proposed Development Traffic Distribution – Intersection of Kingsland Parade and Bentley Place



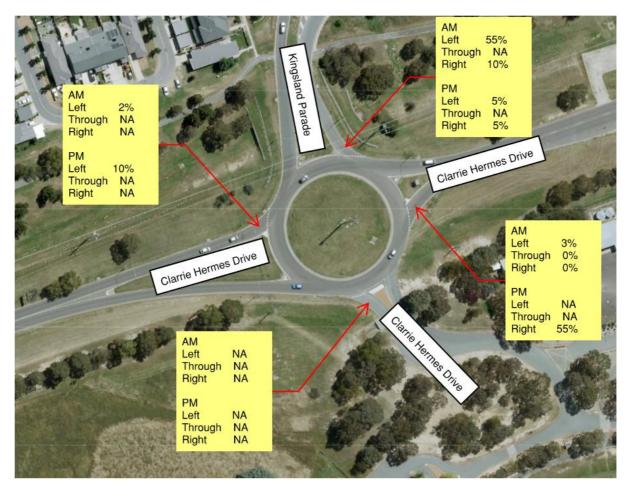


Figure 12 Proposed Development Traffic Distribution - Intersection of Clarrie Hermes Drive and Kingsland Parade



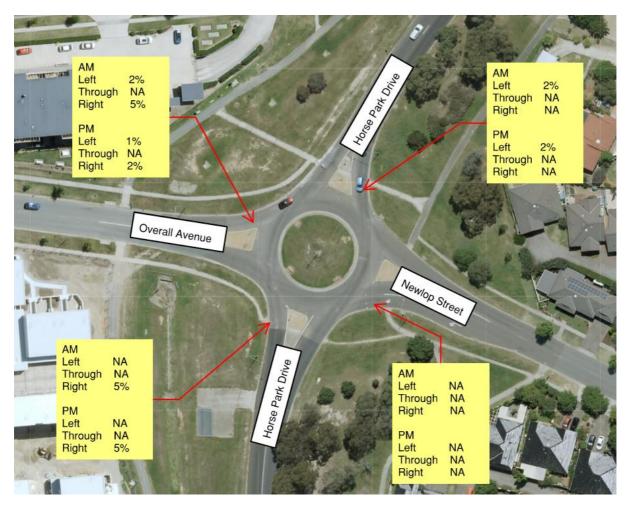


Figure 13 Proposed Development Traffic Distribution - Intersection of Horse Park Drive and Overall Avenue

# 4.6 Traffic Modal Split

The proposed development generally will generate passenger vehicle trips based on its residential component.

It is acknowledged there will be heavy vehicle trips which service both the residential and commercial aspects of the development, however due to the anticipated small number of trips for heavy vehicles, these have not further been considered for modelling purposes.

# 4.7 Traffic Impact

The performance of the key intersections have been reviewed for the development conditions and the future conditions.

For the purpose of this TIA, it has been assumed that the development year will be 2023 and the future conditions will be modelled in 2033.

The increase in traffic on the roads will increase by 2% per year as per the ACT Government TCCS Guidelines for Transport Impact Assessment (Version 3.1, April 2020).



# 4.7.1 Development Conditions

# 4.7.1.1 Development Traffic Conditions at the Intersection of Kingsland Parade, Dallin Crescent and Bentley Place

The intersection of Kingsland Parade and Bentley Place has been modelled using SIDRA Intersection 9.1 for the development conditions (2023).

A summary of the results is available in Table 14.

#### Table 14 SIDRA Intersection Results for the Development Case

Road	Leg	Period	LOS	DOS	Average Queue Length (m)	Average Delay (s)
Kingsland	North	AM	А	0.20	3	2.2
Parade	North	PM	А	0.25	4	3.0
Bentley Place	East	AM	А	0.27	4	4.6
		PM	А	0.24	4	3.7
Kingsland Parade	South	AM	А	0.20	3	2.5
	ooun	PM	А	0.48	10	3.2
Dallin Crescent	West	AM	А	0.10	2	6.3
	vvesi	PM	А	0.11	2	7.3

From the results in Table 14, it can be seen that the intersection has good operation. The results in Table 14 indicate the development has an impact on the operation of the roundabout (refer to the results in Table 8), however the roundabout is in line with the ACT Government TCCS Guidelines for Transport Impact Assessment (Version 3.1, April 2020).



# 4.7.1.2 Development Traffic Conditions at the Intersection of Kingsland Parade and Clarrie Hermes Drive

The intersection of Kingsland Parade and Clarrie Hermes Drive has been modelled using SIDRA Intersection 9.1 for the development conditions (2023).

In line with Section 3.4.2.2 of this TIA, due to the limitations of SIDRA Intersection 9.1 single intersection analysis and Northrop's scope, these results are to demonstrate the impact of the development on the intersection in line with the results within this TIA.

A summary of the results is available in Table 15.

#### Table 15 SIDRA Intersection Results for the Development Case

Road	Leg	Period	LOS	DOS	Average Queue Length (m)	Average Delay (s)
Kingsland	North	AM	А	0.45	9	5.1
Parade	North	PM	А	0.48	11	6.6
Clarrie Hermes Drive	East	AM	A	0.51	12	6.1
		PM	A	0.68	19	8.5
Clarrie Hermes	South	AM	A	0.04	1	9.0
Drive	South	PM	А	0.04	1	10.1
Clarrie Hermes Drive	West	AM	А	0.43	8	5.3
	west	PM	А	0.72	23	7.8

From the results in Table 15, it can be seen that the intersection has good operation under the model conditions (refer to Section 2.4.2.2 of this TIA). The results in Table 15 indicate the development has an impact on the operation of the roundabout, however the impact does not cause inconvenience in line with the SIDRA Intersection 9.1 results.



# 4.7.1.3 Development Traffic Conditions at the Intersection of Horse Park Drive and Overall Avenue

The intersection of Horse Park Drive and Overall Avenue has been modelled using SIDRA Intersection 9.1 for the development conditions (2023).

A summary of the results is available in Table 16.

#### Table 16 SIDRA Intersection Results for the Development Case

Road	Leg	Period	LOS	DOS	Average Queue Length (m)	Average Delay (s)
Horse Park	North	AM	В	0.91	62	17.8
Drive		PM	А	0.58	14	9.4
Newlop Street	East	AM	В	0.34	9	16.9
		PM	А	0.12	2	6.8
Horse Park Drive	South	AM	А	0.60	16	10.5
		PM	А	0.92	65	19.7
Overall Avenue	West	AM	А	0.38	7	7.4
		PM	В	0.62	18	12.7

From the results in Table 16, it can be seen that the intersection has good operation. The results in Table 16 indicate the development has an impact on the operation of the roundabout (refer to Table 10), however the roundabout is in line with the ACT Government TCCS Guidelines for Transport Impact Assessment (Version 3.1, April 2020).



# 4.7.2 Future Conditions

# 4.7.2.1 Future Traffic Conditions at the Intersection of Kingsland Parade, Dallin Crescent and Bentley Place

The intersection of Kingsland Parade and Bentley Place has been modelled using SIDRA Intersection 9.1 for the future conditions (2033).

For the purpose of the future conditions, the development traffic generation has remained the same as the 2023 model with the external traffic being increased by the 2% growth factor only.

A summary of the results is available in Table 17.

Road	Leg	Period	LOS	DOS	Average Queue Length (m)	Average Delay (s)
Kingsland	North	AM	А	0.24	4	2.2
Parade	North	PM	А	0.31	5	3.4
Bentley Place	East	AM	А	0.31	5	5.1
		PM	А	0.30	5	4.0
Kingsland Parade	South	AM	А	0.20	3	2.7
		PM	А	0.65	19	3.8
Dallin Crescent	West	AM	А	0.12	2	6.5
		PM	А	0.16	3	9.0

#### Table 17 SIDRA Intersection Results for the Future Case

From the results in Table 17, it can be seen that the intersection has good operation in the future case modelled inclusive of the development traffic.



### 4.7.2.2 Future Traffic Conditions at the Intersection of Kingsland Parade and Clarrie Hermes Drive

The intersection of Kingsland Parade and Clarrie Hermes Drive has been modelled using SIDRA Intersection 9.1 for the future conditions (2033).

For the purpose of the future conditions, the development traffic generation has remained the same as the 2023 model with the external traffic being increased by the 2% growth factor only.

In line with Section 3.4.2.2 of this TIA, due to the limitations of SIDRA Intersection 9.1 single intersection analysis and Northrop's scope, these results are to demonstrate the impact of the future case on the intersection in line with the results in this TIA.

A summary of the results is available in Table 18.

#### Table 18 SIDRA Intersection Results for the Future Case

Road	Leg	Period	LOS	DOS	Average Queue Length (m)	Average Delay (s)
Kingsland	North	AM	А	0.58	16	8.5
Parade	North	PM	В	0.79	34	19.0
Clarrie Hermes Ea	East	AM	А	0.64	18	6.4
Drive	Last	PM	А	0.85	42	11.8
Clarrie Hermes	South	AM	А	0.07	1	11.6
Drive	South	PM	В	0.08	2	17.1
Clarrie Hermes West	West	AM	А	0.53	12	5.4
Drive	00031	PM	В	0.93	69	16.3

From the results in Table 18, it can be seen that the intersection modelled has good operation in the am peak and operates good in the pm peak with acceptable delays and spare capacity in the future case modelled inclusive of the development traffic.

It is noted however that Clarrie Hermes Drive approaching the West of the intersection has a DOS of 0.93 and is approaching capacity.



#### 4.7.2.3 Development Traffic Conditions at the Intersection of Horse Park Drive and Overall Avenue

The intersection of Horse Park Drive and Overall Avenue has been modelled using SIDRA Intersection 9.1 for the future conditions (2033).

For the purpose of the future conditions, the development traffic generation has remained the same as the 2023 model with the external traffic being increased by the 2% growth factor only.

A summary of the results is available in Table 19.

#### Table 19 SIDRA Intersection Results for the Future Case

Road	Leg	Period	LOS	DOS	Average Queue Length (m)	Average Delay (s)
Horse Park	North	AM	F	1.18	405	175.3
Drive	North	PM	А	0.72	25	11.4
Newlop Street	East	AM	В	0.47	14	25.5 8.5
	Last	PM	А	0.18	4	
Horse Park	South	AM	А	0.75	27	13.7
Drive	South	PM	F	1.20	408	193.0
Overall Avenue	West	AM	А	0.52	13	9.1
	vvost	PM	В	0.77	28	18.9

From the results in Table 19, it can be seen that the intersection is over capacity due to the increased delays along Horse Park Drive. Due to the limited and consistent traffic generated from the proposed development, the saturation of the intersections would be from traffic generation from other origins and destinations.



#### 4.8 Parking

#### 4.8.1 Car Parking

Car parking generation has been reviewed for both the residential and commercial aspects for the building.

#### 4.8.1.1 Residential Car Parking Required

The ACT Planning & Land Authority Parking and Vehicular Access General Code (17 June, 2022) was reviewed to determine the number of car parking spaces required for the residential parking. As the block is located with a CZ1: Core Zone, the following car parking generation rates apply:

- One (1) parking space per single bedroom dwelling; and
- A minimum average provision of 1.5 spaces per two bedroom dwelling, provided that each two bedroom dwelling is allocated a minimum of one (1) parking space and each two (2) bedroom dwelling is allocated no more than two (2) parking spaces; or
- Two (2) parking spaces per two bedroom dwelling; and
- Two (2) parking spaces for each dwelling with three or more bedrooms; plus
- One (1) visitor space per four (4) dwellings or part thereof where a complex comprises four (4) or more dwellings.

This results in 341 car parking spaces being required for the residents of the proposed development.

A total of 55 visitor car parking spaces are required for the development.

#### 4.8.1.2 Commercial Car Parking Required

Northrop understand that the retail will be a split of approximately 478m2 shop style tenancies, 400m2 office style tenancies and 200m2 café/restaurant style tenancies. The ACT Planning & Land Authority Parking and Vehicular Access General Code (17 June, 2022) describes the car parking generation rates for these uses are as follows:

- Restaurant: 10 car parking spaces per 100m2 GFA;
- Office: 2.5 car parking spaces per 100m2 GFA; and
- Shop: 5 car parking spaces per 100m2 GFA.

Based on the areas for each use, 54 car parking spaces would be required for the commercial area of the development.

#### 4.8.2 Car Parking Provided for the Development and Alternate Parking Locations

The proposed development contains 364 car parking spaces in line with drawing DA-01-02 dated 21/09/2022 by Cox Architecture.

The basement will be able to contain the 341 car parking spaces required for the residents.

23 of the 109 commercial and residential visitor parking will be able to be located within the basement car park.

The project team has advised that the car park on Block 10, Section 132 Casey can be used to accommodate the remaining 86 car parking spaces. It is noted that Block 10, Section 132 Casey is located next to the proposed site.

In line with Section 3.5.3 of this TIA and based on the car parking survey, Block 10, Section 132 Casey has the car park capacity to accommodate the additional parking required.



#### 4.8.3 Motorcycle Parking

The ACT Planning & Land Authority Parking and Vehicular Access General Code (17 June, 2022) nominates that 3 dedicated motorcycle/motor scooter parking spaces are required per 100 car parking spaces.

Based on 364 car parking spaces within the development, it would be anticipated that 11 dedicated motorcycle/motor scooter parking spaces are provided.

The proposed development contains 11 motorcycle parking spaces in line with drawing DA-01-02 dated 21/09/2022 by Cox Architecture.

#### 4.8.4 Bicycle Parking

The ACT Planning & Land Authority Bicycle Parking General Code (October, 2013) was reviewed for the purpose of the proposed development. Based on this code, the following bicycle parking facilities are required:

- Residential 1 per apartment (assumed in the storage cage of sufficient size or apartment);
- Residential visitor 18 x class 3 spaces required;
- Office employees 1 x class 1 or 2 space
- Office visitor nil required;
- Restaurant employees nil required;
- Restaurant visitors 2 x class 3 spaces required;
- Shop employees nil required;
- Shop visitors 2 x class 3 spaces required.

It is noted that the latest plans detail 8 bicycle parking spaces for visitors to the development, storage cages for the residents and 18 bicycle parking spaces within the basement which has a similar nature to class 2 bicycle parking (assuming these would be unavailable to the general public but will be available to residential visitors and employees).

In line with the code, no showers or lockers are required as less than 4 employee bicycle parking spaces are required.



### 5. Conclusion

Northrop Consulting Engineers Pty Ltd (Northrop) have been engaged by Jega to prepare a Traffic Impact Assessment (TIA) for the proposed development on Block 9 Section 132, Casey (referred to as to the subject site in this report).

This Traffic Impact Assessment Report has detailed the below:

- An introduction to the report and summary of the proposed development;
- A summary of the development site and nearby conditions;
- An investigation in the existing conditions of the site and key roads including:
  - Traffic Volumes and conditions at key intersections;
  - Public transport within the vicinity of the site;
  - Active travel within the vicinity of the site.
- A summary of the projected traffic and parking conditions from the proposed development and surrounding key roads and intersections including:
  - The trip generation, trip distribution, modal split and trip assignment for the site generated traffic;
  - The increase of traffic at the key intersections;
  - The car park generation on site against the amount of car parking required; and
  - Car park compliance commentary.
- A transportation analysis including:
  - Commentary on proposed site access locations;
  - Commentary on the SIDRA Intersections models completed by Northrop for the key intersections for the base case, development conditions and future conditions for the site; and
  - Commentary on the current accident data for the key roads near the site supplied from the Transport Canberra and City Services Directorate (TCCS).
- A summary of the findings regarding:
  - Site accessibility;
  - Transportation impacts; and
  - Parking impacts.

This conclusion details:

- A summary of the findings regarding:
  - Site accessibility;
  - Transportation impacts; and
  - o Parking impacts.

The report has identified:

- The site access is off Bentley Place for both passenger vehicles and service vehicles;
- The site connects with active travel infrastructure which links the development with the greater Canberra region;
- There are 2 bus stops which are within close vicinity of the proposed development which provide connectivity to the area and Gungahlin Interchange. Gungahlin Interchange provides connectivity to City Interchange and Belconnen through Transport Canberra's Rapid Routes as well as providing connection to other bus routes;



- There are no black spots in close vicinity to the development;
- There is existing available car parking located on Block 10, Section 132 Casey and along Kingsland Parade which provides 418 car parking spaces and 21 motorcycle parking spaces to the area;
- The car park capacity survey undertaken for this report indicated there was at least 160 car parking spaces available in Block 10, Section 132 Casey during the busiest time surveyed;
- The intersection of Kingsland Parade and Bentley Place is at good operation at the base conditions, development conditions (2023) and future conditions (2033);
- The intersection of Clarrie Hermes Drive and Kingsland Parade has been modelled based on model conditions due to limitations of SIDRA Intersections 9.1. The model indicated that the intersection has good operation at the base conditions, development conditions (2023) and future conditions (2033) however was also approaching capacity on the Western leg in the future conditions (2033). Northrop recommends that Roads ACT complete a study on the conditions of the intersection of Kingsland Parade and Clarrie Hermes Drive separate to this TIA and implement appropriate outcomes as identified by their study;
- The intersection of Horse Park Drive and Overall Avenue is at good operation at the base conditions and development conditions (2023), however is at capacity for the future conditions (2033) due to the growth in traffic generation from origins and destinations other than the proposed development;
- The basement car park has capacity for 364 car parking spaces (which accounts for all resident parking required);
- Visitor car parking not provided within the basement for the proposed development is to be provided on Block 10, Section 132 Casey which has capacity for it;
- Dedicated motorcycle/motor scooter parking spaces have been nominated on the architectural plans;
- Bicycle parking has been nominated for the proposed development for both the residential and commercial components;
- Active travel facilities such as showers and lockers are not required for the development due to the employee bicycle parking required.



#### Attachments

# **DEVELOPMENT STATISTICS**

#### YIELD CALCULATION

UNIT TYPE	NO. UNITS	% OF UNITS
1BED	18	8.2%
2BED	133	60.7%
3BED	19	8.7%
3BED ST	20	9.1%
4BED ST	10	4.6%
ADAPTIVE COMMERCIAL	13	5.9%
STUDIO	6	2.7%
TOTAL NO. OF UNITS: 219	219	100.0%

	R PARKI			
		DIMEN		NUMBER C
TYPE		LENGTH	WIDTH	SPACES
BASEMENT 2				
Residential		5400	2400	142
Residential - Tandem		5400	2400	6
BASEMENT 1				148
Residential		5400	2400	106
Retail		5400	2400	16
Residential - Tandem		5400	2400	6
Residential Adaptable		5400	2400	11
LOWER GROUND				139
Residential		5400	2400	65
Residential Adaptable		5400	2400	11
				76
UPPER GROUND EVO Dedicated Carpark		5400	3000	1
		5400	3000	1
TOTAL NO. OF CARS				364
M	отосус	LE SCHE	DULE	
		DIMEN	SIONS	NUMBER C
TYPE		LENGTH	WIDTH	SPACES
LOWER GROUND MOTORCYCLE PARKIN	6			A A
TOTAL NO. OF CARS	0			<u> </u>
STORAGE		2'	32	]
				_
ADAPTABLE UI				
TYPE	ι	INITS		
1 BED		3		
2 BED		13		
3 BED		6		
TOTAL		22		
COMMUNAL C				
	437 m <sup>2</sup>			
LEVEL 1 LEVEL 1	251 m <sup>2</sup> 187 m <sup>2</sup>			
	291 m <sup>2</sup>			
LEVEL 2	1166 m <sup>2</sup>			
LEVEL 2 Grand total	1100 111			
		]		
Grand total	PE ARE			
Grand total LANDSCA LEVEL	PE ARE	A REA		
Grand total LANDSCA LEVEL LOWER GROUND	PE ARE	4		
Grand total LANDSCA LEVEL LOWER GROUND UPPER GROUND	PE ARE	A REA 14 m <sup>2</sup> 960 m <sup>2</sup>		
Grand total LANDSCA LEVEL LOWER GROUND UPPER GROUND LEVEL 1	PE ARE	A REA 14 m <sup>2</sup> 960 m <sup>2</sup> 164 m <sup>2</sup>		
Grand total LANDSCA LEVEL LOWER GROUND UPPER GROUND LEVEL 1 LEVEL 2	PE ARE	A REA 960 m <sup>2</sup> 164 m <sup>2</sup> 302 m <sup>2</sup>		
Grand total LANDSCA LEVEL LOWER GROUND UPPER GROUND LEVEL 1	PE ARE	A REA 14 m <sup>2</sup> 960 m <sup>2</sup> 164 m <sup>2</sup>		

NAME	AREA
	AKEA
LOWER GROUND	
ADAPTIVE COMMERCIAL	641 m²
	641 m²
	740 2
ADAPTIVE COMMERCIAL RETAIL	718 m <sup>2</sup> 1087 m <sup>2</sup>
RETAIL	1806 m <sup>2</sup>
	1000 111
LEVEL 1	
2BED	610 m <sup>2</sup>
3BED	112 m²
	722 m²
LEVEL 2 1BED	181 m²
2BED	181 m²
3BED	347 m <sup>2</sup>
STUDIO	45 m <sup>2</sup>
	2398 m <sup>2</sup>
LEVEL 3	
1BED	181 m <sup>2</sup>
2BED	1825 m <sup>2</sup>
3BED	348 m <sup>2</sup>
STUDIO	45 m <sup>2</sup> 2399 m <sup>2</sup>
	2399 112
LEVEL 4	
1BED	181 m²
2BED	1825 m²
3BED	348 m²
STUDIO	45 m²
	2399 m²
LEVEL 5 1BED	181 m²
2BED	181 m² 1825 m²
3BED	348 m <sup>2</sup>
STUDIO	45 m <sup>2</sup>
010010	2399 m <sup>2</sup>
LEVEL 6	
1BED	181 m²
2BED	1825 m <sup>2</sup>
3BED	348 m <sup>2</sup>
STUDIO	45 m <sup>2</sup>
	2399 m²
LEVEL 7	
1BED	181 m²
2BED	1825 m <sup>2</sup>
3BED	347 m²
STUDIO	45 m²
	2398 m²
LEVEL 8 3BED ST	1162 m <sup>2</sup>
4BED ST	702 m <sup>2</sup>
#!	1864 m <sup>2</sup>
	-
LEVEL 9	
3BED ST	1245 m²
4BED ST	746 m <sup>2</sup>
	1991 m²
TOTAL NLA	21415 m <sup>2</sup>



TOTAL GFA



1466 m<sup>2</sup>

#### GFA EXCLUDES: CARPARK, LANDSCAPE, AND BALCONIES

AREA SCHEDUL	.E - GFA
NAME	AREA
Not Placed	,
WASTE	0 m²
L	0 m²
BASEMENT 2	
SERVICES	22 m²
	22 m²
BASEMENT 1	
SERVICES	22 m²
	22 m²
LOWER GROUND	
ADAPTIVE COMMERCIAL	641 m <sup>2</sup>
POS	180 m²
SERVICES	322 m²
	1143 m²
UPPER GROUND	
ADAPTIVE COMMERCIAL	718 m²
AMENITY	49 m²
LIFT / STAIR	100 m <sup>2</sup>

AMENITY	49 m²
LIFT / STAIR	100 m²
POS	122 m²
RETAIL	1087 m²
SERVICES	163 m²
WASTE	218 m²
	2458 m²

NAME	AREA
1BED	181 m
2BED	1825 m
3BED	347 m
CORRIDOR	517 m
POS	110 m
SERVICES	43 m
STUDIO	45 m
	3068 m
LEVEL 8	

AREA SCHEDULE - GFA

3BED ST	1162 m
4BED ST	702 m
CORRIDOR	456 m
SERVICES	43 m
	2363 m

LEVEL 9	
3BED ST	1245 m²
4BED ST	746 m <sup>2</sup>
SERVICES	82 m²
	2073 m <sup>2</sup>
TOTAL GFA	27914 m²

LEVEL 1	
2BED	610 m <sup>2</sup>
3BED	112 m <sup>2</sup>
AMENITY	273 m <sup>2</sup>
CORRIDOR	264 m²
POS	33 m²
SERVICES	151 m²
	1443 m²

LEVEL 2	
1BED	181 m²
2BED	1825 m²
3BED	347 m²
CORRIDOR	513 m²
POS	110 m <sup>2</sup>
SERVICES	44 m²
STUDIO	45 m²

LEVEL 3	
1BED	181 m²
2BED	1825 m²
3BED	348 m²
CORRIDOR	505 m²
POS	110 m <sup>2</sup>
SERVICES	43 m <sup>2</sup>
STUDIO	45 m²
-	3057 m <sup>2</sup>

3065 m<sup>2</sup>

LEVEL 4	
1BED	181 m
2BED	1825 m
3BED	348 m
CORRIDOR	510 m
POS	110 m
SERVICES	43 m
STUDIO	45 m
	3062 m

LEVEL 5	
1BED	181 m²
2BED	1825 m²
3BED	348 m²
CORRIDOR	517 m²
POS	110 m²
SERVICES	43 m²
STUDIO	45 m²
	3069 m²

181 m <sup>2</sup>
1825 m <sup>2</sup>
348 m <sup>2</sup>
516 m <sup>2</sup>
110 m <sup>2</sup>
43 m <sup>2</sup>
45 m <sup>2</sup>
3069 m <sup>2</sup>

LEVEL 7

DA-01	-02

DA SUBMISSION

@ A1

3

21.09.2022

Worth Street - Mixed Use

Project

BLOCK 09 SECTION 132 CASEY ACT 2913

Drawing Title

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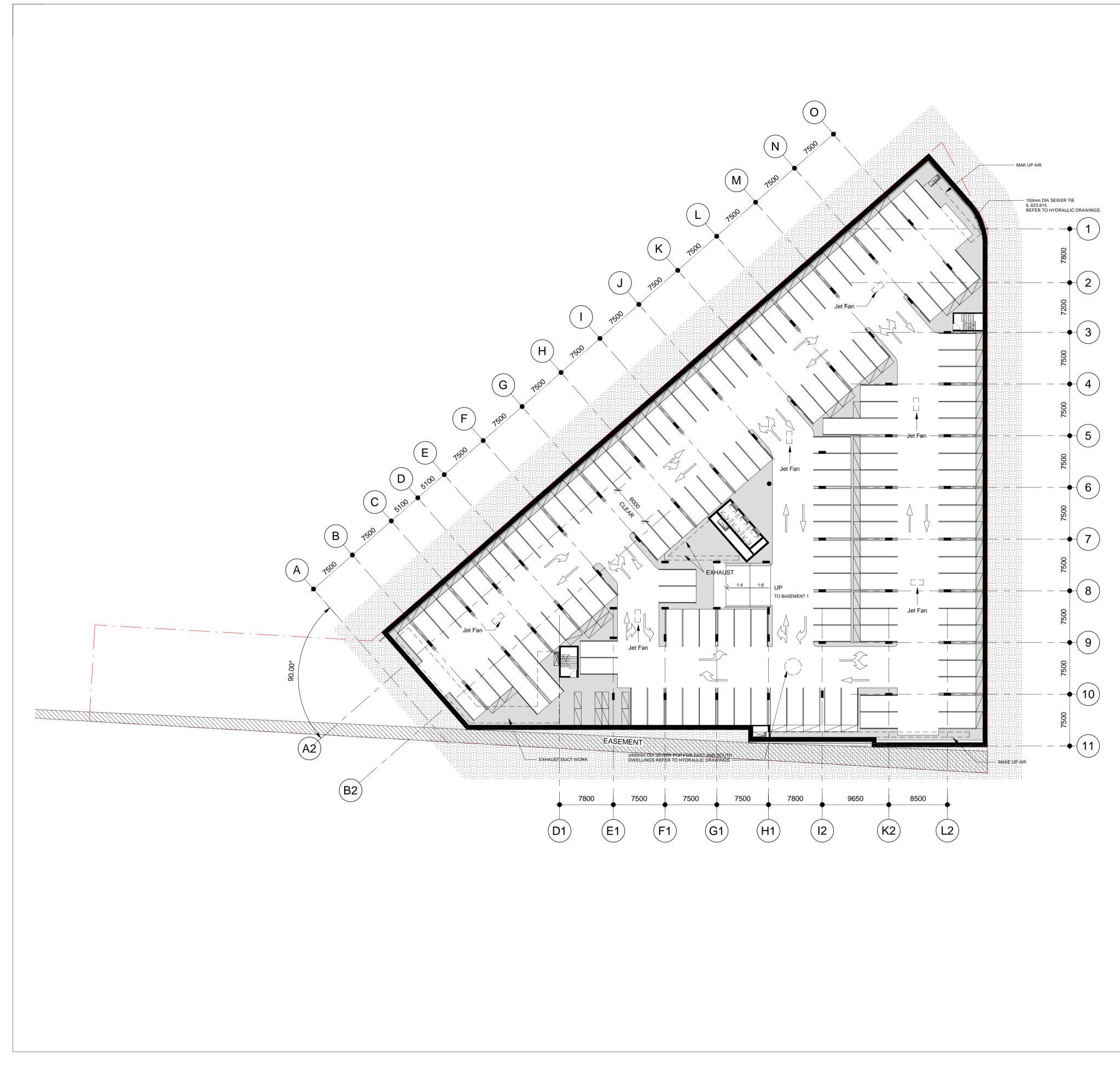
Drawing Number:

Scale:

Date:

DEVELOPMENT STATISTICS

DA-01-02









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LEGEND:	PLANS
FFL X.XXX	PROPOSED FINISHED FLOOR LEVEL (METRES) ABOVE DATUM.
FSL X.XXX	PROPOSED FINISHED SLAB LEVEL (METRES) ABOVE DATUM.
+ RL X.XXX	PROPOSED REDUCED SPOT LEVEL (METRES) ABOVE DATUM.
ROOM NAME	ROOM TAG WITH NAME & NUMBER
5000	GENERAL DIMENSIONS (DIAGONAL)
$\begin{array}{c} \hline GR \\ \bot \end{array} \begin{array}{c} \hline GR \\ 5000 \end{array} \begin{array}{c} \hline GR \\ \bot \end{array}$	DIMENSIONS TO GRID (DOT)
• • •	SITE BOUNDARY
<u>ل</u>	ACCESSIBLE PARKING
А	ADAPTABLE UNIT PARKING
	STORAGE CAGES
[]]]	DRYING AREA
	AIR CONDITIONING
	COMMUNAL OPEN SPACE
SERVICES	S LEGEND:
WM	WATER METER
FBBV	FIRE BRIGADE BOOSTER VALVE
SVR	SPRINKLER VALVE ROOM
FIP EXH	FIRE INDICATIVE PANEL
LB	LETTERBOX
GENERAL	NOTES:
	IS TO BE READ IN CONJUCTION
	N DRAWINGS ARE INDICATIVE AND ETAIL DESIGN AND FINAL CIVIL
HEAD HEIGHT	OVER ACCESSIBLE CAR PARKING
	FTS TO <b>AS 1735.12</b> AND <b>BCA E3.6</b> .
	RKING WILL HAVE ACCESS OPERABLE GATES.
ACCESSIBLE P	ATH OF TRAVEL PROVIDED TO ALL
ENTRANCES TO	NCY UNITS SERVED BY A LIFT. O THE BUILDING AND ALL
	ALONG THE ACCESSIBLE PATH OF MPLY WITH <b>AS 1428.1</b> .
	IDICATORS TO <b>AS 1428.4</b> . RFACES TO BE SLIP RESISTANT
BROOM FINISH	ED.
<ul> <li>HANDRAILS TC</li> <li>STORAGE CAG</li> </ul>	es to be secured with Min.
AREA OF 1.5m <sup>2</sup>	
UNIT LEG	END:
P	REDROOM
BL	BEDROOM
D	DINING
ĸ	KITCHEN
BTH ENS	BATHROOM
L'DRY	LAUNDRY
ST	STUDY
PR	POWDER ROOM
WIR COMM	WALK-IN ROBE COMMERCIAL / OFFICE SPACE
MR	MEDIA ROOM

Project

Drawing Title

### Worth Street - Mixed Use

BLOCK 09 SECTION 132 CASEY ACT 2913

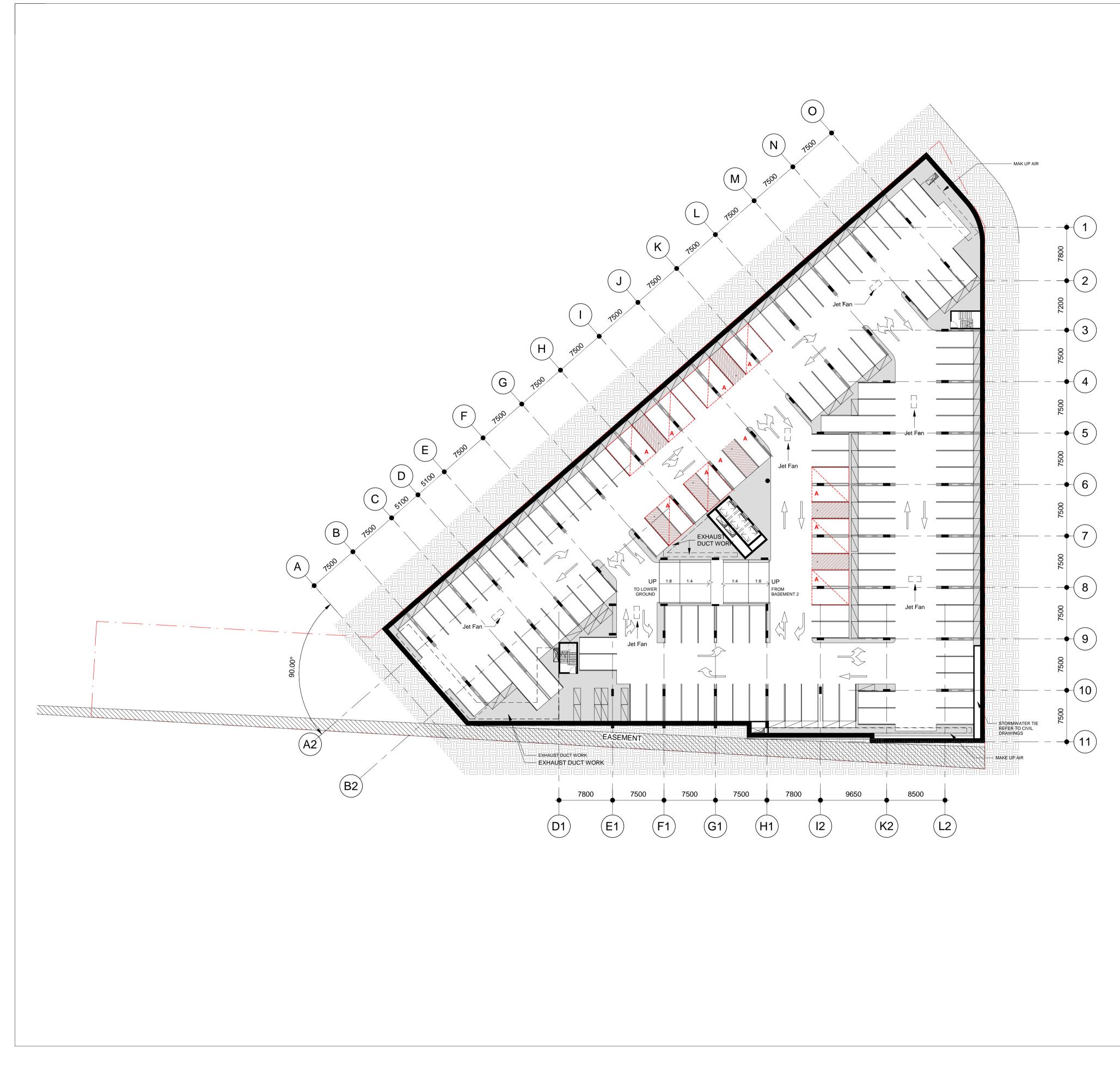
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Drawing Number:



PN - BASEMENT 2

DA-20-01



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LEGEND:	PLANS
FFL X.XXX	PROPOSED FINISHED FLOOR LEVEL (METRES) ABOVE DATUM.
FSL X.XXX	PROPOSED FINISHED SLAB LEVEL (METRES) ABOVE DATUM.
+ RL X.XXX	PROPOSED REDUCED SPOT LEVEL (METRES) ABOVE DATUM.
ROOM NAME	ROOM TAG WITH NAME & NUMBER
5000	GENERAL DIMENSIONS (DIAGONAL)
$\begin{array}{c} \hline GR \\ \bot \end{array} \begin{array}{c} \hline GR \\ 5000 \end{array} \begin{array}{c} \hline GR \\ \bot \end{array}$	DIMENSIONS TO GRID (DOT)
• • •	SITE BOUNDARY
<u>ل</u>	ACCESSIBLE PARKING
А	ADAPTABLE UNIT PARKING
	STORAGE CAGES
[]]]	DRYING AREA
	AIR CONDITIONING
	COMMUNAL OPEN SPACE
SERVICES	S LEGEND:
WM	WATER METER
FBBV	FIRE BRIGADE BOOSTER VALVE
SVR	SPRINKLER VALVE ROOM
FIP EXH	FIRE INDICATIVE PANEL
LB	LETTERBOX
GENERAL	NOTES:
	IS TO BE READ IN CONJUCTION
	N DRAWINGS ARE INDICATIVE AND ETAIL DESIGN AND FINAL CIVIL
HEAD HEIGHT	OVER ACCESSIBLE CAR PARKING
	FTS TO <b>AS 1735.12</b> AND <b>BCA E3.6</b> .
	RKING WILL HAVE ACCESS OPERABLE GATES.
ACCESSIBLE P	ATH OF TRAVEL PROVIDED TO ALL
ENTRANCES TO	NCY UNITS SERVED BY A LIFT. O THE BUILDING AND ALL
	ALONG THE ACCESSIBLE PATH OF MPLY WITH <b>AS 1428.1</b> .
	IDICATORS TO <b>AS 1428.4</b> . RFACES TO BE SLIP RESISTANT
BROOM FINISH	ED.
<ul> <li>HANDRAILS TC</li> <li>STORAGE CAG</li> </ul>	es to be secured with Min.
AREA OF 1.5m <sup>2</sup>	
UNIT LEG	END:
P	REDROOM
BL	BEDROOM
D	DINING
ĸ	KITCHEN
BTH ENS	BATHROOM
L'DRY	LAUNDRY
ST	STUDY
PR	POWDER ROOM
WIR COMM	WALK-IN ROBE COMMERCIAL / OFFICE SPACE
MR	MEDIA ROOM

Project

Drawing Title

### Worth Street - Mixed Use

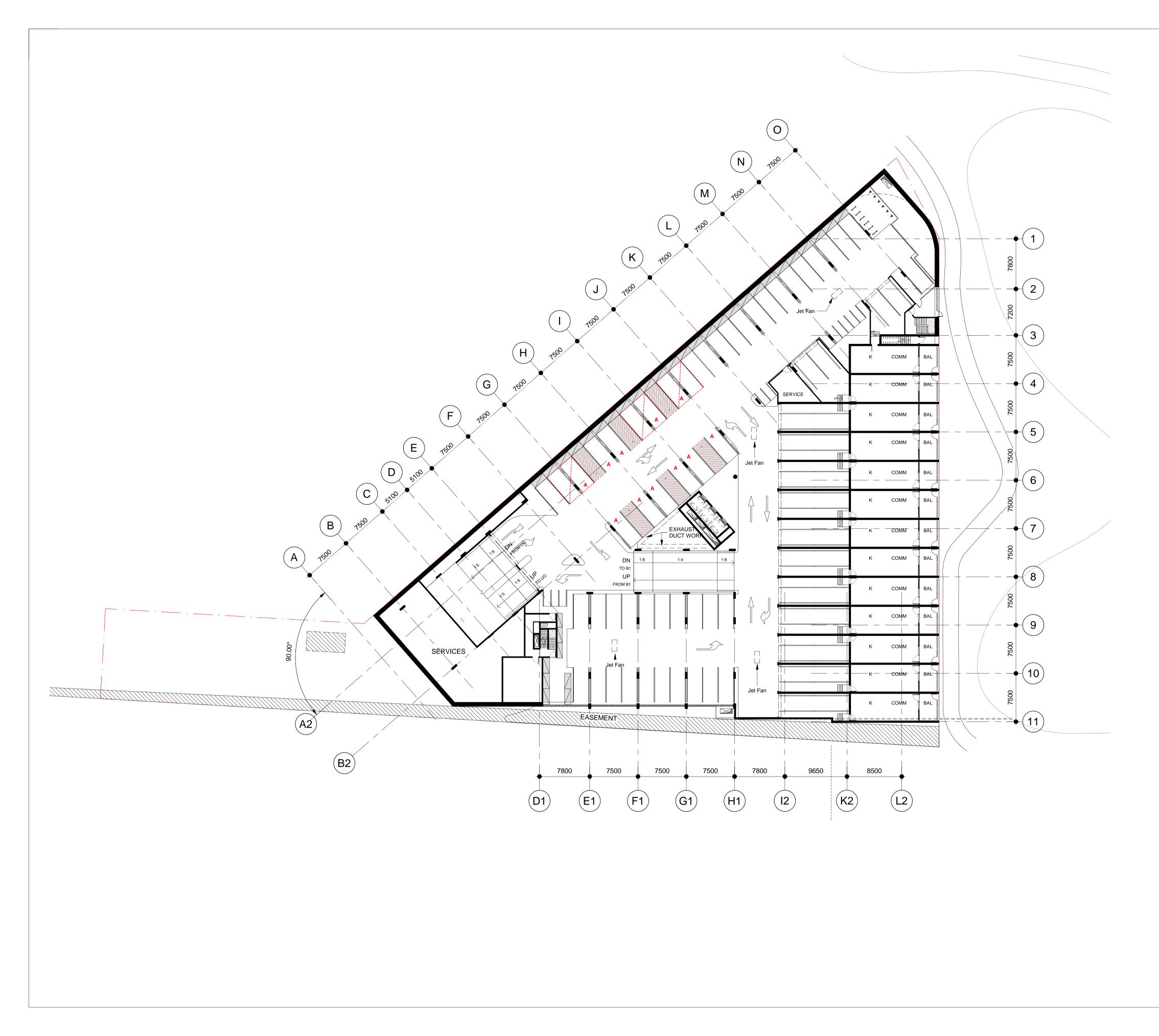
BLOCK 09 SECTION 132 CASEY ACT 2913

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Drawing Number:

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PN - BASEMENT 1



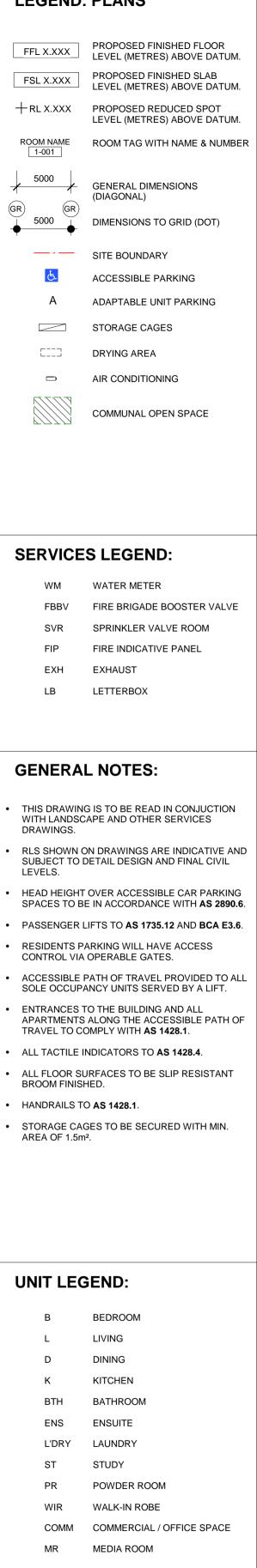
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#### AREA TYPE ADAPTIVE COMMERCIAL

#### LEGEND: PLANS



Project

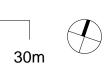
#### Worth Street - Mixed Use

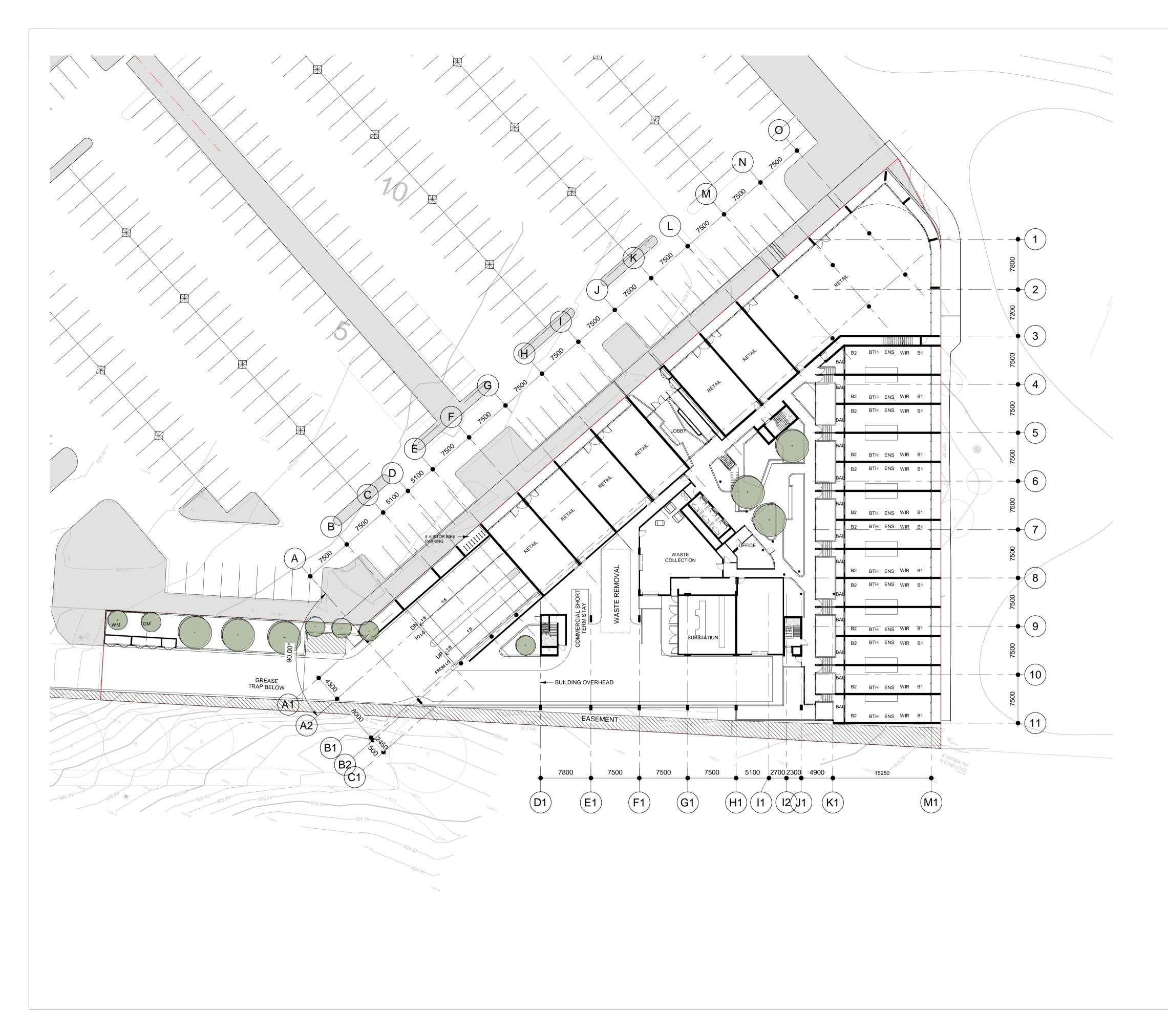
BLOCK 09 SECTION 132 CASEY ACT 2913

Scale: 1 : 300 @ A1 Date: 21.09.2022 **Revision:** 2 DA SUBMISSION

Drawing Title PN - LOWER GROUND LEVEL

Drawing Number:



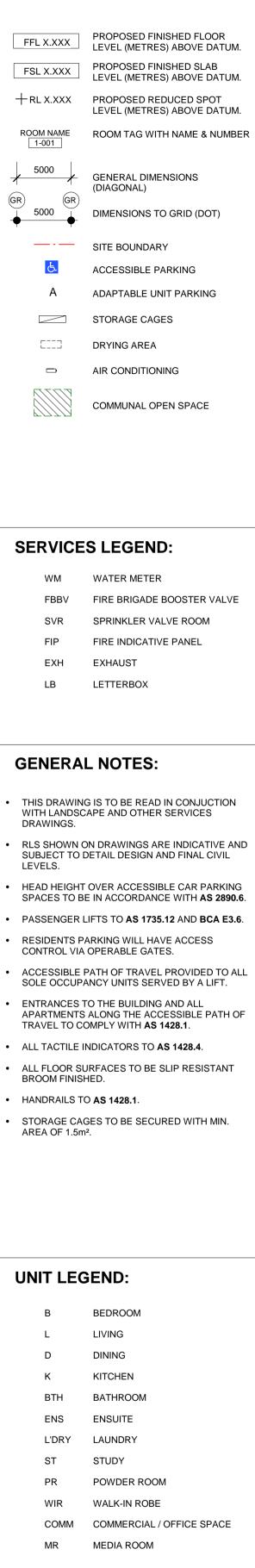


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#### AREA TYPE ADAPTIVE COMMERCIAL RETAIL

#### LEGEND: PLANS



Project	

30m

### Worth Street - Mixed Use

BLOCK 09 SECTION 132 CASEY ACT 2913

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evision:	2 DA SUBMISSION

Drawing Title PN - UPPER GROUND LEVEL

Drawing Number:

Scale:

## V Site: 101A [AM Base 2022 Weekday Peak 8:00am - 9:00am (Site Folder: Kingsland Parade and Bentley Place)]

Output produced by SIDRA INTERSECTION Version: 9.1.1.200

New Site Site Category: (None) Roundabout

Intersection Performance - Hourly Values			
Performance Measure	Vehicles:	All MCs	Persons
Travel Speed (Average) Travel Distance (Total) Travel Time (Total) Desired Speed Speed Efficiency Travel Time Index Congestion Coefficient	km/h veh-km/h veh-h/h km/h	36.4 262.9 7.2 40.0 0.91 9.01 1.10	36.4 km/h 315.5 pers-km/h 8.7 pers-h/h
Demand Flows (Total) Arrival Flows (Total) Percent Heavy Vehicles (Demand) Percent Heavy Vehicles (Arrivals) Degree of Saturation Practical Spare Capacity Effective Intersection Capacity	veh/h veh/h % % veh/h	651 651 4.5 4.5 0.172 395.4 3791	781 pers/h
Control Delay (Total) Control Delay (Average) Control Delay (Worst Lane by MC) Control Delay (Worst Movement by MC) Geometric Delay (Average) Stop-Line Delay (Average) Idling Time (Average) Intersection Level of Service (LOS)	veh-h/h sec sec sec sec sec sec	0.47 2.6 5.6 8.1 2.0 0.6 0.0 LOS A	0.57 pers-h/h 2.6 sec 8.1 sec
95% Back of Queue - Veh (Worst Lane) 95% Back of Queue - Dist (Worst Lane) Ave. Que Storage Ratio (Worst Lane) Effective Stops (Total) Effective Stop Rate Proportion Queued Performance Index	veh m veh/h	0.9 6.4 0.03 209 0.32 0.26 12.6	250 pers/h 0.32 0.26 12.6
Cost (Total) Fuel Consumption (Total) Carbon Dioxide (Total) Hydrocarbons (Total) Carbon Monoxide (Total) NOx (Total)	\$/h L/h kg/h kg/h kg/h kg/h	298.19 26.3 62.5 0.005 0.04 0.113	298.19 \$/h

Site Level of Service (LOS) Method: Delay (RTA NSW). Site LOS Method is specified in the Parameter Settings dialog (Options tab).

Intersection LOS value for Vehicles is based on average delay for all vehicle movements.

Roundabout Capacity Model: SIDRA Standard.

Delay Model: SIDRA Standard (Control Delay: Geometric Delay is included).

Queue Model: SIDRA queue estimation methods are used for Back of Queue and Queue at Start of Gap.

Arrival Flows used in performance calculations are adjusted to include any Initial Queued Demand effects.

In Network analysis, Arrival Flows will be reduced if Upstream Capacity Constraint exists.

Gap-Acceptance Capacity Formula: SIDRA Standard (Akçelik M3D).

Site Model Variability Index (Average value of largest changes in Lane Degrees of Saturation from the third to the last Main (Timing-Capacity) Iterations): 0.6 %

Number of Iterations: 3 (Maximum: 10)

Largest change in Lane Degrees of Saturation for the last three Flow-Capacity Iterations: 100.0% 0.0% 0.6%

Intersection Performance - Annual Values			
Performance Measure	Vehicles:	All MCs	Persons
Demand Flows (Total)	veh/y	312,253	374,703 pers/y
Delay (Total)	veh-h/y	227	273 pers-h/y

Effective Stops (Total)	veh/y	100,165	120,198 pers/y	
Travel Distance (Total)	veh-km/y	126,204	151,445 pers-km/y	
Travel Time (Total)	veh-h/y	3,465	4,158 pers-h/y	
Cost (Total) Fuel Consumption (Total) Carbon Dioxide (Total) Hydrocarbons (Total) Carbon Monoxide (Total) NOx (Total)	\$/y L/y kg/y kg/y kg/y kg/y	143,132 12,613 30,024 2 19 54	143,132 \$/y	

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### V Site: 101A [AM Development 2023 Weekday Peak 8:00am - 9:00am (Site Folder: Kingsland Parade and Bentley Place)]

Output produced by SIDRA INTERSECTION Version: 9.1.1.200

New Site Site Category: (None) Roundabout

Intersection Performance - Hourly Va	lues		
Performance Measure	Vehicles:	All MCs	Persons
Travel Speed (Average) Travel Distance (Total) Travel Time (Total) Desired Speed Speed Efficiency Travel Time Index Congestion Coefficient	km/h veh-km/h veh-h/h km/h	35.8 348.8 9.7 40.6 0.88 8.69 1.13	35.8 km/h 418.6 pers-km/h 11.7 pers-h/h
Demand Flows (Total) Arrival Flows (Total) Percent Heavy Vehicles (Demand) Percent Heavy Vehicles (Arrivals) Degree of Saturation Practical Spare Capacity Effective Intersection Capacity	veh/h veh/h % % veh/h	883 883 4.1 4.1 0.275 209.4 3214	1060 pers/h
Control Delay (Total) Control Delay (Average) Control Delay (Worst Lane by MC) Control Delay (Worst Movement by MC) Geometric Delay (Average) Stop-Line Delay (Average) Idling Time (Average) Intersection Level of Service (LOS)	veh-h/h sec sec sec sec sec sec sec	0.88 3.6 6.3 8.4 2.5 1.1 0.0 LOS A	1.05 pers-h/h 3.6 sec 8.4 sec
95% Back of Queue - Veh (Worst Lane) 95% Back of Queue - Dist (Worst Lane) Ave. Que Storage Ratio (Worst Lane) Effective Stops (Total) Effective Stop Rate Proportion Queued Performance Index	veh m veh/h	1.4 10.0 0.04 362 0.41 0.38 19.2	435 pers/h 0.41 0.38 19.2
Cost (Total) Fuel Consumption (Total) Carbon Dioxide (Total) Hydrocarbons (Total) Carbon Monoxide (Total) NOx (Total)	\$/h L/h kg/h kg/h kg/h kg/h	405.10 36.6 87.1 0.007 0.06 0.149	405.10 \$/h

Site Level of Service (LOS) Method: Delay (RTA NSW). Site LOS Method is specified in the Parameter Settings dialog (Options tab).

Intersection LOS value for Vehicles is based on average delay for all vehicle movements.

Roundabout Capacity Model: SIDRA Standard.

Delay Model: SIDRA Standard (Control Delay: Geometric Delay is included).

Queue Model: SIDRA queue estimation methods are used for Back of Queue and Queue at Start of Gap.

Arrival Flows used in performance calculations are adjusted to include any Initial Queued Demand effects.

In Network analysis, Arrival Flows will be reduced if Upstream Capacity Constraint exists.

Gap-Acceptance Capacity Formula: SIDRA Standard (Akçelik M3D).

Site Model Variability Index (Average value of largest changes in Lane Degrees of Saturation from the third to the last Main (Timing-Capacity) Iterations): 0.8 %

Number of Iterations: 3 (Maximum: 10)

Largest change in Lane Degrees of Saturation for the last three Flow-Capacity Iterations: 100.0% 0.0% 0.8%

Intersection Performance - Annual Values				
Performance Measure	Vehicles:	All MCs	Persons	
Demand Flows (Total)	veh/y	423,916	508,699 pers/y	
Delay (Total)	veh-h/y	422	506 pers-h/y	

Effective Stops (Total)	veh/y	173,834	208,600 pers/y	
Travel Distance (Total)	veh-km/y	167,430	200,916 pers-km/y	
Travel Time (Total)	veh-h/y	4,677	5,612 pers-h/y	
Cost (Total) Fuel Consumption (Total) Carbon Dioxide (Total) Hydrocarbons (Total) Carbon Monoxide (Total) NOx (Total)	\$/y L/y kg/y kg/y kg/y kg/y	194,448 17,575 41,793 3 27 71	194,448 \$/y	

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### V Site: 101A [AM Future 2033 Weekday Peak 8:00am - 9:00am (Site Folder: Kingsland Parade and Bentley Place)]

Output produced by SIDRA INTERSECTION Version: 9.1.1.200

New Site Site Category: (None) Roundabout

Intersection Performance - Hourly Values			
Performance Measure	Vehicles:	All MCs	Persons
Travel Speed (Average) Travel Distance (Total) Travel Time (Total) Desired Speed Speed Efficiency Travel Time Index Congestion Coefficient	km/h veh-km/h veh-h/h km/h	35.6 391.5 11.0 40.5 0.88 8.65 1.14	35.6 km/h 469.8 pers-km/h 13.2 pers-h/h
Demand Flows (Total) Arrival Flows (Total) Percent Heavy Vehicles (Demand) Percent Heavy Vehicles (Arrivals) Degree of Saturation Practical Spare Capacity Effective Intersection Capacity	veh/h veh/h % % veh/h	979 979 3.9 0.314 171.0 3121	1175 pers/h
Control Delay (Total) Control Delay (Average) Control Delay (Worst Lane by MC) Control Delay (Worst Movement by MC) Geometric Delay (Average) Stop-Line Delay (Average) Idling Time (Average) Intersection Level of Service (LOS)	veh-h/h sec sec sec sec sec sec sec	1.04 3.8 6.5 9.1 2.6 1.3 0.0 LOS A	1.25 pers-h/h 3.8 sec 9.1 sec
95% Back of Queue - Veh (Worst Lane) 95% Back of Queue - Dist (Worst Lane) Ave. Que Storage Ratio (Worst Lane) Effective Stops (Total) Effective Stop Rate Proportion Queued Performance Index	veh m veh/h	1.7 12.0 0.05 421 0.43 0.42 22.2	506 pers/h 0.43 0.42 22.2
Cost (Total) Fuel Consumption (Total) Carbon Dioxide (Total) Hydrocarbons (Total) Carbon Monoxide (Total) NOx (Total)	\$/h L/h kg/h kg/h kg/h kg/h	456.73 41.1 97.6 0.008 0.06 0.160	456.73 \$/h

Site Level of Service (LOS) Method: Delay (RTA NSW). Site LOS Method is specified in the Parameter Settings dialog (Options tab).

Intersection LOS value for Vehicles is based on average delay for all vehicle movements.

Roundabout Capacity Model: SIDRA Standard.

Delay Model: SIDRA Standard (Control Delay: Geometric Delay is included).

Queue Model: SIDRA queue estimation methods are used for Back of Queue and Queue at Start of Gap.

Arrival Flows used in performance calculations are adjusted to include any Initial Queued Demand effects.

In Network analysis, Arrival Flows will be reduced if Upstream Capacity Constraint exists.

Gap-Acceptance Capacity Formula: SIDRA Standard (Akçelik M3D).

Site Model Variability Index (Average value of largest changes in Lane Degrees of Saturation from the third to the last Main (Timing-Capacity) Iterations): 0.8 %

Number of Iterations: 4 (Maximum: 10)

Largest change in Lane Degrees of Saturation for the last three Flow-Capacity Iterations: 0.0% 1.1% 0.6%

Intersection Performance - Annual Values				
Performance Measure	Vehicles:	All MCs	Persons	
Demand Flows (Total)	veh/y	469,895	563,874 pers/y	
Delay (Total)	veh-h/y	500	600 pers-h/y	

Effective Stops (Total)	veh/y	202,265	242,718 pers/y	
Travel Distance (Total)	veh-km/y	187,937	225,525 pers-km/y	
Travel Time (Total)	veh-h/y	5,280	6,336 pers-h/y	
Cost (Total) Fuel Consumption (Total) Carbon Dioxide (Total) Hydrocarbons (Total) Carbon Monoxide (Total) NOx (Total)	\$/y L/y kg/y kg/y kg/y kg/y	219,229 19,718 46,844 4 31 77	219,229 \$/y	

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## V Site: 101B [PM Base 2022 Weekday Peak 5:15pm - 6:15pm (Site Folder: Kingsland Parade and Bentley Place)]

Output produced by SIDRA INTERSECTION Version: 9.1.1.200

New Site Site Category: (None) Roundabout

Intersection Performance - Hourly Values			
Performance Measure	Vehicles:	All MCs	Persons
Travel Speed (Average) Travel Distance (Total) Travel Time (Total) Desired Speed Speed Efficiency Travel Time Index Congestion Coefficient	km/h veh-km/h veh-h/h km/h	36.2 394.5 10.9 40.0 0.90 8.94 1.11	36.2 km/h 473.4 pers-km/h 13.1 pers-h/h
Demand Flows (Total) Arrival Flows (Total) Percent Heavy Vehicles (Demand) Percent Heavy Vehicles (Arrivals) Degree of Saturation Practical Spare Capacity Effective Intersection Capacity	veh/h veh/h % % veh/h	1000 1000 2.0 2.0 0.353 140.6 2831	1200 pers/h
Control Delay (Total) Control Delay (Average) Control Delay (Worst Lane by MC) Control Delay (Worst Movement by MC) Geometric Delay (Average) Stop-Line Delay (Average) Idling Time (Average) Intersection Level of Service (LOS)	veh-h/h sec sec sec sec sec sec sec	0.74 2.7 6.2 8.4 1.9 0.8 0.0 LOS A	0.89 pers-h/h 2.7 sec 8.4 sec
95% Back of Queue - Veh (Worst Lane) 95% Back of Queue - Dist (Worst Lane) Ave. Que Storage Ratio (Worst Lane) Effective Stops (Total) Effective Stop Rate Proportion Queued Performance Index	veh m veh/h	2.2 15.5 0.09 338 0.34 0.31 21.4	405 pers/h 0.34 0.31 21.4
Cost (Total) Fuel Consumption (Total) Carbon Dioxide (Total) Hydrocarbons (Total) Carbon Monoxide (Total) NOx (Total)	\$/h L/h kg/h kg/h kg/h kg/h	441.24 35.7 84.3 0.006 0.05 0.089	441.24 \$/h

Site Level of Service (LOS) Method: Delay (RTA NSW). Site LOS Method is specified in the Parameter Settings dialog (Options tab).

Intersection LOS value for Vehicles is based on average delay for all vehicle movements.

Roundabout Capacity Model: SIDRA Standard.

Delay Model: SIDRA Standard (Control Delay: Geometric Delay is included).

Queue Model: SIDRA queue estimation methods are used for Back of Queue and Queue at Start of Gap.

Arrival Flows used in performance calculations are adjusted to include any Initial Queued Demand effects.

In Network analysis, Arrival Flows will be reduced if Upstream Capacity Constraint exists.

Gap-Acceptance Capacity Formula: SIDRA Standard (Akçelik M3D).

Site Model Variability Index (Average value of largest changes in Lane Degrees of Saturation from the third to the last Main (Timing-Capacity) Iterations): 1.2 %

Number of Iterations: 4 (Maximum: 10)

Largest change in Lane Degrees of Saturation for the last three Flow-Capacity Iterations: 0.0% 1.5% 0.9%

Intersection Performance - Annual Values				
Performance Measure	Vehicles:	All MCs	Persons	
Demand Flows (Total)	veh/y	480,000	576,000 pers/y	
Delay (Total)	veh-h/y	356	428 pers-h/y	

Effective Stops (Total)	veh/y	162,085	194,502 pers/y
Travel Distance (Total)	veh-km/y	189,350	227,220 pers-km/y
Travel Time (Total)	veh-h/y	5,232	6,278 pers-h/y
Cost (Total)	\$/y	211,797	211,797 \$/y
Fuel Consumption (Total)	L/y	17,120	
Carbon Dioxide (Total)	kg/y	40,472	
Hydrocarbons (Total)	kg/y	3	
Carbon Monoxide (Total)	kg/y	23	
NOx (Total)	kg/y	43	

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### V Site: 101B [PM Development 2023 Weekday Peak 5:15pm - 6:15pm (Site Folder: Kingsland Parade and Bentley Place)]

Output produced by SIDRA INTERSECTION Version: 9.1.1.200

New Site Site Category: (None) Roundabout

Intersection Performance - Hourly Values			
Performance Measure	Vehicles:	All MCs	Persons
Travel Speed (Average) Travel Distance (Total) Travel Time (Total) Desired Speed Speed Efficiency Travel Time Index Congestion Coefficient	km/h veh-km/h veh-h/h km/h	35.1 465.8 13.3 40.0 0.88 8.63 1.14	35.1 km/h 558.9 pers-km/h 15.9 pers-h/h
Demand Flows (Total) Arrival Flows (Total) Percent Heavy Vehicles (Demand) Percent Heavy Vehicles (Arrivals) Degree of Saturation Practical Spare Capacity Effective Intersection Capacity	veh/h veh/h % % veh/h	1242 1242 2.1 2.1 0.475 78.8 2613	1491 pers/h
Control Delay (Total) Control Delay (Average) Control Delay (Worst Lane by MC) Control Delay (Worst Movement by MC) Geometric Delay (Average) Stop-Line Delay (Average) Idling Time (Average) Intersection Level of Service (LOS)	veh-h/h sec sec sec sec sec sec	1.22 3.5 7.3 9.8 2.3 1.2 0.1 LOS A	1.46 pers-h/h 3.5 sec 9.8 sec
95% Back of Queue - Veh (Worst Lane) 95% Back of Queue - Dist (Worst Lane) Ave. Que Storage Ratio (Worst Lane) Effective Stops (Total) Effective Stop Rate Proportion Queued Performance Index	veh m veh/h	3.4 24.3 0.14 518 0.42 0.43 29.1	621 pers/h 0.42 0.43 29.1
Cost (Total) Fuel Consumption (Total) Carbon Dioxide (Total) Hydrocarbons (Total) Carbon Monoxide (Total) NOx (Total)	\$/h L/h kg/h kg/h kg/h kg/h	542.10 45.4 107.3 0.008 0.06 0.123	542.10 \$/h

Site Level of Service (LOS) Method: Delay (RTA NSW). Site LOS Method is specified in the Parameter Settings dialog (Options tab).

Intersection LOS value for Vehicles is based on average delay for all vehicle movements.

Roundabout Capacity Model: SIDRA Standard.

Delay Model: SIDRA Standard (Control Delay: Geometric Delay is included).

Queue Model: SIDRA queue estimation methods are used for Back of Queue and Queue at Start of Gap.

Arrival Flows used in performance calculations are adjusted to include any Initial Queued Demand effects.

In Network analysis, Arrival Flows will be reduced if Upstream Capacity Constraint exists.

Gap-Acceptance Capacity Formula: SIDRA Standard (Akçelik M3D).

Site Model Variability Index (Average value of largest changes in Lane Degrees of Saturation from the third to the last Main (Timing-Capacity) Iterations): 2.0 %

Number of Iterations: 5 (Maximum: 10)

Largest change in Lane Degrees of Saturation for the last three Flow-Capacity Iterations: 3.1% 1.9% 1.0%

Intersection Performance - Annual Values				
Performance Measure	Vehicles:	All MCs	Persons	
Demand Flows (Total)	veh/y	596,211	715,453 pers/y	
Delay (Total)	veh-h/y	584	701 pers-h/y	

Effective Stops (Total)	veh/y	248,540	298,248 pers/y
Travel Distance (Total)	veh-km/y	223,577	268,292 pers-km/y
Travel Time (Total)	veh-h/y	6,378	7,653 pers-h/y
Cost (Total)	\$/y	260,210	260,210 \$/y
Fuel Consumption (Total)	L/y	21,772	
Carbon Dioxide (Total)	kg/y	51,483	
Hydrocarbons (Total)	kg/y	4	
Carbon Monoxide (Total)	kg/y	31	
NOx (Total)	kg/y	59	

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### V Site: 101B [PM Future 2033 Weekday Peak 5:15pm - 6:15pm (Site Folder: Kingsland Parade and Bentley Place)]

Output produced by SIDRA INTERSECTION Version: 9.1.1.200

New Site Site Category: (None) Roundabout

Intersection Performance - Hourly Va	alues		
Performance Measure	Vehicles:	All MCs	Persons
Travel Speed (Average) Travel Distance (Total) Travel Time (Total) Desired Speed Speed Efficiency Travel Time Index Congestion Coefficient	km/h veh-km/h veh-h/h km/h	34.4 580.1 16.9 40.0 0.86 8.45 1.16	34.4 km/h 696.1 pers-km/h 20.2 pers-h/h
Demand Flows (Total) Arrival Flows (Total) Percent Heavy Vehicles (Demand) Percent Heavy Vehicles (Arrivals) Degree of Saturation Practical Spare Capacity Effective Intersection Capacity	veh/h veh/h % % veh/h	1536 1536 6.6 6.6 0.649 31.0 2366	1843 pers/h
Control Delay (Total) Control Delay (Average) Control Delay (Worst Lane by MC) Control Delay (Worst Movement by MC) Geometric Delay (Average) Stop-Line Delay (Average) Idling Time (Average) Intersection Level of Service (LOS)	veh-h/h sec sec sec sec sec sec sec	1.74 4.1 9.0 11.5 2.3 1.7 0.2 LOS A	2.09 pers-h/h 4.1 sec 11.5 sec
95% Back of Queue - Veh (Worst Lane) 95% Back of Queue - Dist (Worst Lane) Ave. Que Storage Ratio (Worst Lane) Effective Stops (Total) Effective Stop Rate Proportion Queued Performance Index	veh m veh/h	6.1 46.2 0.27 717 0.47 0.55 43.8	861 pers/h 0.47 0.55 43.8
Cost (Total) Fuel Consumption (Total) Carbon Dioxide (Total) Hydrocarbons (Total) Carbon Monoxide (Total) NOx (Total)	\$/h L/h kg/h kg/h kg/h kg/h	713.84 69.1 165.5 0.013 0.11 0.421	713.84 \$/h

Site Level of Service (LOS) Method: Delay (RTA NSW). Site LOS Method is specified in the Parameter Settings dialog (Options tab).

Intersection LOS value for Vehicles is based on average delay for all vehicle movements.

Roundabout Capacity Model: SIDRA Standard.

Delay Model: SIDRA Standard (Control Delay: Geometric Delay is included).

Queue Model: SIDRA queue estimation methods are used for Back of Queue and Queue at Start of Gap.

Arrival Flows used in performance calculations are adjusted to include any Initial Queued Demand effects.

In Network analysis, Arrival Flows will be reduced if Upstream Capacity Constraint exists.

Gap-Acceptance Capacity Formula: SIDRA Standard (Akçelik M3D).

Site Model Variability Index (Average value of largest changes in Lane Degrees of Saturation from the third to the last Main (Timing-Capacity) Iterations): 2.7 %

Number of Iterations: 7 (Maximum: 10)

Largest change in Lane Degrees of Saturation for the last three Flow-Capacity Iterations: 2.1% 1.1% 0.6%

Intersection Performance - An	nual Values		
Performance Measure	Vehicles:	All MCs	Persons
Demand Flows (Total)	veh/y	737,179	884,615 pers/y
Delay (Total)	veh-h/y	834	1,001 pers-h/y

Effective Stops (Total)	veh/y	344,251	413,101 pers/y
Travel Distance (Total)	veh-km/y	278,432	334,119 pers-km/y
Travel Time (Total)	veh-h/y	8,092	9,711 pers-h/y
Cost (Total)	\$/y	342,642	342,642 \$/y
Fuel Consumption (Total)	L/y	33,166	
Carbon Dioxide (Total)	kg/y	79,449	
Hydrocarbons (Total)	kg/y	6	
Carbon Monoxide (Total)	kg/y	54	
NOx (Total)	kg/y	202	

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#### V Site: 101A [AM Base 2022 Weekday Peak 8:00am - 9:00am (Site Folder: Kingsland Parade and Bentley Place)]

Output produced by SIDRA INTERSECTION Version: 9.1.1.200

New Site Site Category: (None) Roundabout

Vehio	cle Mo	ovemen	t Performa	nce									
Mov ID	Turn	Mov Class	Demand Flows [ Total HV ] veh/h %	Arriva Flows [ Total HV ] veh/h %	s Satn	Aver. Delay sec	Level of Service	95% B Que [ Veh. veh		Prop. Que	Eff. Stop Rate	Aver. No. of Cycles	Aver. Speed km/h
South	: King	sland Pa	rade										
1	L2	All MCs	32 13.3	32 13.3	0.153	2.0	LOS A	0.8	5.6	0.17	0.24	0.17	37.7
2	T1	All MCs	165 5.1	165 5.1	0.153	1.4	LOS A	0.8	5.6	0.17	0.24	0.17	37.4
3	R2	All MCs	12 0.0	12 0.0	0.153	5.7	LOS A	0.8	5.6	0.17	0.24	0.17	33.6
Appro	ach		208 6.1	208 6.1	0.153	1.7	LOS A	0.8	5.6	0.17	0.24	0.17	37.3
East:	Bentle	ey Place											
4	L2	All MCs	97 2.2	97 2.2	0.117	2.9	LOS A	0.5	3.7	0.38	0.45	0.38	31.7
5	T1	All MCs	13 0.0	13 0.0	0.117	2.3	LOS A	0.5	3.7	0.38	0.45	0.38	37.0
6	R2	All MCs	20 5.3	20 5.3	0.117	6.7	LOS A	0.5	3.7	0.38	0.45	0.38	35.5
Appro	ach		129 2.4	129 2.4	0.117	3.4	LOS A	0.5	3.7	0.38	0.45	0.38	33.6
North	Kings	sland Par	ade										
8	T1	All MCs	209 4.5	209 4.5	0.172	1.5	LOS A	0.9	6.4	0.22	0.24	0.22	37.1
9	R2	All MCs	11 0.0	11 0.0	0.172	5.8	LOS A	0.9	6.4	0.22	0.24	0.22	38.0
9u	U	All MCs	4 100. 0	4 100 (	0.172	8.1	LOS A	0.9	6.4	0.22	0.24	0.22	37.2
Appro	ach		224 6.1	224 6.1	0.172	1.8	LOS A	0.9	6.4	0.22	0.24	0.22	37.1
West:	Dalki	n Crescei	nt										
10	L2	All MCs	25 0.0	25 0.0	0.080	2.8	LOS A	0.4	3.1	0.40	0.52	0.40	36.6
12	R2	All MCs	63 0.0	63 0.0	0.080	6.7	LOS A	0.4	3.1	0.40	0.52	0.40	35.1
Appro	ach		88 0.0	88 0.0	0.080	5.6	LOS A	0.4	3.1	0.40	0.52	0.40	35.7
All Ve	hicles		651 4.5	651 4.5	6 0.172	2.6	LOS A	0.9	6.4	0.26	0.32	0.26	36.4

Site Level of Service (LOS) Method: Delay (RTA NSW). Site LOS Method is specified in the Parameter Settings dialog (Options tab).

Vehicle movement LOS values are based on average delay per movement.

Intersection and Approach LOS values are based on average delay for all vehicle movements.

Roundabout Capacity Model: SIDRA Standard.

Delay Model: SIDRA Standard (Control Delay: Geometric Delay is included).

Queue Model: SIDRA queue estimation methods are used for Back of Queue and Queue at Start of Gap.

Gap-Acceptance Capacity Formula: SIDRA Standard (Akcelik M3D).

HV (%) values are calculated for All Movement Classes of All Heavy Vehicle Model Designation.

Arrival Flows used in performance calculations are adjusted to include any Initial Queued Demand and Upstream Capacity Constraint effects.

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### V Site: 101A [AM Development 2023 Weekday Peak 8:00am - 9:00am (Site Folder: Kingsland Parade and Bentley Place)]

Output produced by SIDRA INTERSECTION Version: 9.1.1.200

New Site Site Category: (None) Roundabout

Vehi	cle Mo	ovemen	t Performa	nce									
Mov ID	Turn	Mov Class			Satn	Aver. Delay sec	Level of Service		Back Of ieue Dist ] m	Prop. Que	Eff. Stop Rate	Aver. No. of Cycles	Aver. Speed km/h
South	: King	sland Pa				360		Ven		_	_	_	K11/11
1	L2	All MCs	34 15.6	34 15.6	0.196	2.7	LOS A	1.0	7.7	0.35	0.34	0.35	36.9
2	T1	All MCs	169 5.6	169 5.6	0.196	2.0	LOS A	1.0	7.7	0.35	0.34	0.35	36.3
3	R2	All MCs	23 0.0	23 0.0	0.196	6.3	LOS A	1.0	7.7	0.35	0.34	0.35	31.9
Appro	ach		226 6.5	226 6.5	0.196	2.5	LOS A	1.0	7.7	0.35	0.34	0.35	36.2
East:	Bentle	ey Place											
4	L2	All MCs	158 1.3	158 1.3	0.275	3.1	LOS A	1.4	10.0	0.44	0.52	0.44	30.4
5	T1	All MCs	24 0.0	24 0.0	0.275	2.6	LOS A	1.4	10.0	0.44	0.52	0.44	36.3
6	R2	All MCs	121 2.6	121 2.6	0.275	6.9	LOS A	1.4	10.0	0.44	0.52	0.44	34.7
Appro	ach		303 1.7	303 1.7	0.275	4.6	LOS A	1.4	10.0	0.44	0.52	0.44	33.4
North	: Kings	sland Par	ade										
7	L2	All MCs	21 0.0	21 0.0	0.198	4.3	LOS A	1.0	7.7	0.26	0.28	0.26	38.7
8	T1	All MCs	215 4.9	215 4.9	0.198	1.6	LOS A	1.0	7.7	0.26	0.28	0.26	37.4
9	R2	All MCs				5.9	LOS A	1.0	7.7	0.26	0.28	0.26	38.2
9u	U	All MCs	5 100. 0	-	0	8.4	LOS A	1.0	7.7	0.26	0.28	0.26	37.4
Appro	ach		252 6.3	252 6.3	0.198	2.2	LOS A	1.0	7.7	0.26	0.28	0.26	37.6
West:	Dalki	n Crescei	nt										
10	L2	All MCs	26 0.0	26 0.0	0.102	3.6	LOS A	0.6	4.1	0.51	0.57	0.51	37.1
11	T1	All MCs	11 0.0	11 0.0	0.102	6.0	LOS A	0.6	4.1	0.51	0.57	0.51	37.4
12	R2	All MCs	65 0.0	65 0.0	0.102	7.5	LOS A	0.6	4.1	0.51	0.57	0.51	35.7
Appro	ach		102 0.0	102 0.0	0.102	6.3	LOS A	0.6	4.1	0.51	0.57	0.51	36.3
All Ve	hicles		883 4.1	883 4.1	0.275	3.6	LOS A	1.4	10.0	0.38	0.41	0.38	35.8

Site Level of Service (LOS) Method: Delay (RTA NSW). Site LOS Method is specified in the Parameter Settings dialog (Options tab).

Vehicle movement LOS values are based on average delay per movement.

Intersection and Approach LOS values are based on average delay for all vehicle movements.

Roundabout Capacity Model: SIDRA Standard.

Delay Model: SIDRA Standard (Control Delay: Geometric Delay is included).

Queue Model: SIDRA queue estimation methods are used for Back of Queue and Queue at Start of Gap.

Gap-Acceptance Capacity Formula: SIDRA Standard (Akçelik M3D).

HV (%) values are calculated for All Movement Classes of All Heavy Vehicle Model Designation.

Arrival Flows used in performance calculations are adjusted to include any Initial Queued Demand and Upstream Capacity Constraint effects.

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### V Site: 101A [AM Future 2033 Weekday Peak 8:00am - 9:00am (Site Folder: Kingsland Parade and Bentley Place)]

Output produced by SIDRA INTERSECTION Version: 9.1.1.200

New Site Site Category: (None) Roundabout

Vehi	cle Mo	ovement	t Perforn	nanc	е										
Mov ID	Turn	Mov Class	Dema Flo [ Total H	ws V][T	Fl otal F		Deg. Satn	Aver. Delay	Level of Service	Qu [ Veh.	ack Of eue Dist ]	Prop. Que	Eff. Stop Rate	Aver. No. of Cycles	Aver. Speed
Couth	. Kin a	aland Day		% ve	eh/h	%	v/c	sec	-	veh	m			-	km/h
	•	sland Pa		- 0			0.000		100.4		0.4	0.00	0.00	0.00	00.7
1		All MCs	34 15			15.6	0.203	2.8	LOS A	1.1	8.1	0.39	0.36	0.39	36.7
2	T1	All MCs	169 5	5.6	169	5.6	0.203	2.2	LOS A	1.1	8.1	0.39	0.36	0.39	36.1
3	R2	All MCs		0.0	23	0.0	0.203	6.4	LOS A	1.1	8.1	0.39	0.36	0.39	31.7
Appro	bach		226 6	5.5	226	6.5	0.203	2.7	LOS A	1.1	8.1	0.39	0.36	0.39	36.0
East:	Bentle	ey Place													
4	L2	All MCs	162 ´	1.3	162	1.3	0.314	3.5	LOS A	1.7	12.0	0.51	0.55	0.51	30.0
5	T1	All MCs	26 (	0.0	26	0.0	0.314	3.0	LOS A	1.7	12.0	0.51	0.55	0.51	36.0
6	R2	All MCs	142 2	2.2	142	2.2	0.314	7.3	LOS A	1.7	12.0	0.51	0.55	0.51	34.4
Appro	bach		331 1	1.6	331	1.6	0.314	5.1	LOS A	1.7	12.0	0.51	0.55	0.51	33.2
North	: Kings	sland Par	ade												
7	L2	All MCs	21 (	0.0	21	0.0	0.240	4.4	LOS A	1.3	9.8	0.30	0.29	0.30	38.5
8	T1	All MCs	262 4	4.8	262	4.8	0.240	1.7	LOS A	1.3	9.8	0.30	0.29	0.30	37.1
9	R2	All MCs	13 (	0.0	13	0.0	0.240	6.0	LOS A	1.3	9.8	0.30	0.29	0.30	38.0
9u	U	All MCs	5 <sup>10</sup>	00. 0	5 ์	100. 0	0.240	8.6	LOS A	1.3	9.8	0.30	0.29	0.30	37.3
Appro	bach		301 క	5.9	301	5.9	0.240	2.2	LOS A	1.3	9.8	0.30	0.29	0.30	37.3
West:	Dalki	n Crescei	nt												
10	L2	All MCs	32 (	0.0	32	0.0	0.124	3.8	LOS A	0.7	5.0	0.54	0.58	0.54	36.9
11	T1	All MCs	11 (	0.0	11	0.0	0.124	6.2	LOS A	0.7	5.0	0.54	0.58	0.54	37.2
12	R2	All MCs	79 (	0.0	79	0.0	0.124	7.6	LOS A	0.7	5.0	0.54	0.58	0.54	35.5
Appro	bach		121 (	0.0	121	0.0	0.124	6.5	LOS A	0.7	5.0	0.54	0.58	0.54	36.1
All Ve	hicles		979 3	3.9	979	3.9	0.314	3.8	LOS A	1.7	12.0	0.42	0.43	0.42	35.6

Site Level of Service (LOS) Method: Delay (RTA NSW). Site LOS Method is specified in the Parameter Settings dialog (Options tab).

Vehicle movement LOS values are based on average delay per movement.

Intersection and Approach LOS values are based on average delay for all vehicle movements.

Roundabout Capacity Model: SIDRA Standard.

Delay Model: SIDRA Standard (Control Delay: Geometric Delay is included).

Queue Model: SIDRA queue estimation methods are used for Back of Queue and Queue at Start of Gap.

Gap-Acceptance Capacity Formula: SIDRA Standard (Akçelik M3D).

HV (%) values are calculated for All Movement Classes of All Heavy Vehicle Model Designation.

Arrival Flows used in performance calculations are adjusted to include any Initial Queued Demand and Upstream Capacity Constraint effects.

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#### V Site: 101B [PM Base 2022 Weekday Peak 5:15pm - 6:15pm (Site Folder: Kingsland Parade and Bentley Place)]

**Output produced by SIDRA INTERSECTION Version: 9.1.1.200** 

New Site Site Category: (None) Roundabout

Vehi	cle Mo	ovement	t Perfo	rma	nce										
Mov ID	Turn	Mov Class	Fi [ Total		Fi [ Total		Deg. Satn	Aver. Delay	Level of Service	Qu [ Veh.	Back Of ieue Dist ]	Prop. Que	Eff. Stop Rate	Aver. No. of Cycles	Aver. Speed
South	n: King	sland Pai	veh/h rade	%	veh/h	%	v/c	sec	_	veh	m	_	_	_	km/h
1	L2	All MCs	55	0.0	55	0.0	0.353	2.2	LOS A	2.2	15.5	0.29	0.29	0.29	37.2
2	T1	All MCs	382	1.7	382	1.7	0.353	1.7	LOS A	2.2	15.5	0.29	0.29	0.29	36.7
3	R2	All MCs	40	2.6	40	2.6	0.353	6.0	LOS A	2.2	15.5	0.29	0.29	0.29	32.3
Appro	bach		477	1.5	477	1.5	0.353	2.1	LOS A	2.2	15.5	0.29	0.29	0.29	36.5
East:	Bentle	ey Place													
4	L2	All MCs	152	0.7	152	0.7	0.190	2.9	LOS A	0.9	6.3	0.39	0.45	0.39	31.6
5	T1	All MCs	23	0.0	23	0.0	0.190	2.3	LOS A	0.9	6.3	0.39	0.45	0.39	36.9
6	R2	All MCs	39	2.7	39	2.7	0.190	6.7	LOS A	0.9	6.3	0.39	0.45	0.39	35.4
Appro	bach		214	1.0	214	1.0	0.190	3.5	LOS A	0.9	6.3	0.39	0.45	0.39	33.7
North	: Kings	sland Par	ade												
7	L2	All MCs	2	0.0	2	0.0	0.183	2.0	LOS A	0.9	6.7	0.21	0.27	0.21	36.9
8	T1	All MCs	209	3.0	209	3.0	0.183	1.5	LOS A	0.9	6.7	0.21	0.27	0.21	37.0
9	R2	All MCs	23	0.0	23	0.0	0.183	5.8	LOS A	0.9	6.7	0.21	0.27	0.21	37.9
9u	U	All MCs	8	50.0	8	50.0	0.183	7.7	LOS A	0.9	6.7	0.21	0.27	0.21	37.3
Appro	bach		243	4.3	243	4.3	0.183	2.1	LOS A	0.9	6.7	0.21	0.27	0.21	37.1
West	: Dalkiı	n Crescei	nt												
10	L2	All MCs	36	0.0	36	0.0	0.076	4.5	LOS A	0.4	3.1	0.60	0.59	0.60	36.4
11	T1	All MCs	2	0.0	2	0.0	0.076	4.1	LOS A	0.4	3.1	0.60	0.59	0.60	35.4
12	R2	All MCs	28	0.0	28	0.0	0.076	8.4	LOS A	0.4	3.1	0.60	0.59	0.60	34.9
Appro	bach		66	0.0	66	0.0	0.076	6.2	LOS A	0.4	3.1	0.60	0.59	0.60	35.9
All Ve	hicles		1000	2.0	1000	2.0	0.353	2.7	LOS A	2.2	15.5	0.31	0.34	0.31	36.2

Site Level of Service (LOS) Method: Delay (RTA NSW). Site LOS Method is specified in the Parameter Settings dialog (Options tab).

Vehicle movement LOS values are based on average delay per movement.

Intersection and Approach LOS values are based on average delay for all vehicle movements.

Roundabout Capacity Model: SIDRA Standard.

Delay Model: SIDRA Standard (Control Delay: Geometric Delay is included).

Queue Model: SIDRA queue estimation methods are used for Back of Queue and Queue at Start of Gap.

Gap-Acceptance Capacity Formula: SIDRA Standard (Akçelik M3D).

HV (%) values are calculated for All Movement Classes of All Heavy Vehicle Model Designation.

Arrival Flows used in performance calculations are adjusted to include any Initial Queued Demand and Upstream Capacity Constraint effects.

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#### V Site: 101B [PM Development 2023 Weekday Peak 5:15pm -6:15pm (Site Folder: Kingsland Parade and Bentley Place)]

**Output produced by SIDRA INTERSECTION Version: 9.1.1.200** 

New Site Site Category: (None) Roundabout

Vehi	cle Mo	ovement	t Perfo	rma	nce										
Mov ID	Turn	Mov Class	F			rival lows HV ] %	Deg. Satn v/c	Aver. Delay sec	Level of Service		Back Of Jeue Dist ] m	Prop. Que	Eff. Stop Rate	Aver. No. of Cycles	Aver. Speed km/h
South	n: King	sland Par	ade												
1	L2	All MCs	57	0.0	57	0.0	0.475	2.5	LOS A	3.4	24.3	0.39	0.38	0.39	36.4
2	T1	All MCs	392	1.9	392	1.9	0.475	2.0	LOS A	3.4	24.3	0.39	0.38	0.39	35.7
3	R2	All MCs	178	1.2	178	1.2	0.475	6.3	LOS A	3.4	24.3	0.39	0.38	0.39	30.9
Appro	bach		626	1.5	626	1.5	0.475	3.2	LOS A	3.4	24.3	0.39	0.38	0.39	34.9
East:	Bentle	ey Place													
4	L2	All MCs	176	1.2	176	1.2	0.239	3.0	LOS A	1.2	8.7	0.44	0.47	0.44	31.3
5	T1	All MCs	35	0.0	35	0.0	0.239	2.4	LOS A	1.2	8.7	0.44	0.47	0.44	36.7
6	R2	All MCs	52	4.1	52	4.1	0.239	6.8	LOS A	1.2	8.7	0.44	0.47	0.44	35.2
Appro	bach		262	1.6	262	1.6	0.239	3.7	LOS A	1.2	8.7	0.44	0.47	0.44	33.7
North	: Kings	sland Par	ade												
7	L2	All MCs	24	0.0	24	0.0	0.246	2.8	LOS A	1.3	9.6	0.41	0.38	0.41	36.0
8	T1	All MCs	215	3.4	215	3.4	0.246	2.3	LOS A	1.3	9.6	0.41	0.38	0.41	36.0
9	R2	All MCs	24	0.0	24	0.0	0.246	6.6	LOS A	1.3	9.6	0.41	0.38	0.41	37.4
9u	U	All MCs	11	50.0	11	50.0	0.246	8.9	LOS A	1.3	9.6	0.41	0.38	0.41	36.7
Appro	bach		274	4.6	274	4.6	0.246	3.0	LOS A	1.3	9.6	0.41	0.38	0.41	36.2
West	Dalki	n Crescer	nt												
10	L2	All MCs	37	0.0	37	0.0	0.108	6.0	LOS A	0.7	4.7	0.71	0.65	0.71	35.9
11	T1	All MCs	14	0.0	14	0.0	0.108	5.5	LOS A	0.7	4.7	0.71	0.65	0.71	34.7
12	R2	All MCs	29	0.0	29	0.0	0.108	9.8	LOS A	0.7	4.7	0.71	0.65	0.71	34.2
Appro	bach		80	0.0	80	0.0	0.108	7.3	LOS A	0.7	4.7	0.71	0.65	0.71	35.2
All Ve	hicles		1242	2.1	1242	2.1	0.475	3.5	LOS A	3.4	24.3	0.43	0.42	0.43	35.1

Site Level of Service (LOS) Method: Delay (RTA NSW). Site LOS Method is specified in the Parameter Settings dialog (Options tab).

Vehicle movement LOS values are based on average delay per movement.

Intersection and Approach LOS values are based on average delay for all vehicle movements.

Roundabout Capacity Model: SIDRA Standard.

Delay Model: SIDRA Standard (Control Delay: Geometric Delay is included).

Queue Model: SIDRA queue estimation methods are used for Back of Queue and Queue at Start of Gap.

Gap-Acceptance Capacity Formula: SIDRA Standard (Akçelik M3D).

HV (%) values are calculated for All Movement Classes of All Heavy Vehicle Model Designation.

Arrival Flows used in performance calculations are adjusted to include any Initial Queued Demand and Upstream Capacity Constraint effects.

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## V Site: 101B [PM Future 2033 Weekday Peak 5:15pm - 6:15pm (Site Folder: Kingsland Parade and Bentley Place)]

Output produced by SIDRA INTERSECTION Version: 9.1.1.200

New Site Site Category: (None) Roundabout

Vehi	cle Mo	ovement	l Perfo	rma	nce										
Mov ID	Turn	Mov Class		ows		rival lows HV 1	Deg. Satn	Aver. Delay	Level of Service		Back Of Jeue Dist ]	Prop. Que	Eff. Stop Rate	Aver. No. of Cycles	Aver. Speed
			veh/h		veh/h	%	v/c	sec		veh	m				km/h
South	n: King	sland Pa	ade												
1	L2	All MCs	114	39.8	114	39.8	0.649	3.5	LOS A	6.1	46.2	0.55	0.43	0.55	35.8
2	T1	All MCs	471	0.7	471	0.7	0.649	2.4	LOS A	6.1	46.2	0.55	0.43	0.55	35.1
3	R2	All MCs	221	16.2	221	16.2	0.649	7.0	LOS A	6.1	46.2	0.55	0.43	0.55	28.9
Appro	bach		805	10.5	805	10.5	0.649	3.8	LOS A	6.1	46.2	0.55	0.43	0.55	34.1
East:	Bentle	ey Place													
4	L2	All MCs	211	1.0	211	1.0	0.297	3.4	LOS A	1.6	11.6	0.51	0.51	0.51	30.8
5	T1	All MCs	40	0.0	40	0.0	0.297	2.8	LOS A	1.6	11.6	0.51	0.51	0.51	36.5
6	R2	All MCs	60	3.5	60	3.5	0.297	7.2	LOS A	1.6	11.6	0.51	0.51	0.51	35.0
Appro	bach		311	1.4	311	1.4	0.297	4.0	LOS A	1.6	11.6	0.51	0.51	0.51	33.3
North	: King	sland Par	ade												
7	L2	All MCs	24	0.0	24	0.0	0.312	3.2	LOS A	1.8	13.2	0.51	0.44	0.51	35.5
8	T1	All MCs	261	3.2	261	3.2	0.312	2.8	LOS A	1.8	13.2	0.51	0.44	0.51	35.5
9	R2	All MCs	29	0.0	29	0.0	0.312	7.0	LOS A	1.8	13.2	0.51	0.44	0.51	37.1
9u	U	All MCs	11 :	50.0	11	50.0	0.312	9.6	LOS A	1.8	13.2	0.51	0.44	0.51	36.4
Appro	bach		325	4.2	325	4.2	0.312	3.4	LOS A	1.8	13.2	0.51	0.44	0.51	35.8
West	: Dalki	n Crescei	nt												
10	L2	All MCs	45	0.0	45	0.0	0.156	7.6	LOS A	1.0	7.2	0.82	0.72	0.82	35.1
11	T1	All MCs	14	0.0	14	0.0	0.156	7.2	LOS A	1.0	7.2	0.82	0.72	0.82	33.7
12	R2	All MCs	36	0.0	36	0.0	0.156	11.5	LOS A	1.0	7.2	0.82	0.72	0.82	33.1
Appro	bach		95	0.0	95	0.0	0.156	9.0	LOS A	1.0	7.2	0.82	0.72	0.82	34.3
All Ve	hicles		1536	6.6	1536	6.6	0.649	4.1	LOS A	6.1	46.2	0.55	0.47	0.55	34.4

Site Level of Service (LOS) Method: Delay (RTA NSW). Site LOS Method is specified in the Parameter Settings dialog (Options tab).

Vehicle movement LOS values are based on average delay per movement.

Intersection and Approach LOS values are based on average delay for all vehicle movements.

Roundabout Capacity Model: SIDRA Standard.

Delay Model: SIDRA Standard (Control Delay: Geometric Delay is included).

Queue Model: SIDRA queue estimation methods are used for Back of Queue and Queue at Start of Gap.

Gap-Acceptance Capacity Formula: SIDRA Standard (Akçelik M3D).

HV (%) values are calculated for All Movement Classes of All Heavy Vehicle Model Designation.

Arrival Flows used in performance calculations are adjusted to include any Initial Queued Demand and Upstream Capacity Constraint effects.

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Project: J:\YEAR 2022 JOBS\220895-00 Block 9 Section 132 Casey Apartment + Sky Terrace\G - Design Calculations\CIVIL\Casey Apartments.sip9

### V Site: 101C [AM Base 2022 Weekday Peak 8:00am - 9:00am (Site Folder: Kingsland Parade and Clarrie Hermes Drive)]

Output produced by SIDRA INTERSECTION Version: 9.1.1.200

NA Site Category: (None) Roundabout

Intersection Performance - Hourly Va	lues		
Performance Measure	Vehicles:	All MCs	Persons
Travel Speed (Average) Travel Distance (Total) Travel Time (Total) Desired Speed Speed Efficiency Travel Time Index Congestion Coefficient	km/h veh-km/h veh-h/h km/h	53.3 1098.7 20.6 70.4 0.76 7.31 1.32	53.3 km/h 1318.4 pers-km/h 24.7 pers-h/h
Demand Flows (Total) Arrival Flows (Total) Percent Heavy Vehicles (Demand) Percent Heavy Vehicles (Arrivals) Degree of Saturation Practical Spare Capacity Effective Intersection Capacity	veh/h veh/h % % veh/h	1788 1788 5.3 5.3 0.513 65.6 3484	2146 pers/h
Control Delay (Total) Control Delay (Average) Control Delay (Worst Lane by MC) Control Delay (Worst Movement by MC) Geometric Delay (Average) Stop-Line Delay (Average) Idling Time (Average) Intersection Level of Service (LOS)	veh-h/h sec sec sec sec sec sec sec	2.91 5.8 9.2 16.4 4.8 1.0 0.0 LOS A	3.49 pers-h/h 5.8 sec 16.4 sec
95% Back of Queue - Veh (Worst Lane) 95% Back of Queue - Dist (Worst Lane) Ave. Que Storage Ratio (Worst Lane) Effective Stops (Total) Effective Stop Rate Proportion Queued Performance Index	veh m veh/h	3.9 28.8 0.08 869 0.49 0.43 42.7	1042 pers/h 0.49 0.43 42.7
Cost (Total) Fuel Consumption (Total) Carbon Dioxide (Total) Hydrocarbons (Total) Carbon Monoxide (Total) NOx (Total)	\$/h L/h kg/h kg/h kg/h kg/h	1099.27 143.5 341.1 0.033 0.46 0.738	1099.27 \$/h

Site Level of Service (LOS) Method: Delay (RTA NSW). Site LOS Method is specified in the Parameter Settings dialog (Options tab).

Intersection LOS value for Vehicles is based on average delay for all vehicle movements.

Roundabout Capacity Model: SIDRA Standard.

Delay Model: SIDRA Standard (Control Delay: Geometric Delay is included).

Queue Model: SIDRA queue estimation methods are used for Back of Queue and Queue at Start of Gap.

Arrival Flows used in performance calculations are adjusted to include any Initial Queued Demand effects.

In Network analysis, Arrival Flows will be reduced if Upstream Capacity Constraint exists.

Gap-Acceptance Capacity Formula: SIDRA Standard (Akçelik M3D).

Site Model Variability Index (Average value of largest changes in Lane Degrees of Saturation from the third to the last Main (Timing-Capacity) Iterations): 2.3 %

Number of Iterations: 7 (Maximum: 10)

Largest change in Lane Degrees of Saturation for the last three Flow-Capacity Iterations: 1.9% 1.0% 0.5%

Intersection Performance - Ar	Intersection Performance - Annual Values										
Performance Measure	Vehicles:	All MCs	Persons								
Demand Flows (Total)	veh/y	858,442	1,030,131 pers/y								
Delay (Total)	veh-h/y	1,395	1,673 pers-h/y								

Effective Stops (Total)	veh/y	416,902	500,283 pers/y	
Travel Distance (Total)	veh-km/y	527,355	632,826 pers-km/y	
Travel Time (Total)	veh-h/y	9,887	11,864 pers-h/y	
Cost (Total) Fuel Consumption (Total) Carbon Dioxide (Total) Hydrocarbons (Total) Carbon Monoxide (Total) NOx (Total)	\$/y L/y kg/y kg/y kg/y kg/y	527,649 68,877 163,739 16 220 354	527,649 \$/y	

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# V Site: 101C [AM Development 2023 Weekday Peak 8:00am - 9:00am (Site Folder: Kingsland Parade and Clarrie Hermes Drive)]

Output produced by SIDRA INTERSECTION Version: 9.1.1.200

NA

Site Category: (None) Roundabout

Performance Measure	Vehicles:	All MCs	Persons
ravel Speed (Average)	km/h	52.3	52.3 km/h
ravel Distance (Total)	veh-km/h	1163.2	1395.9 pers-km/h
ravel Time (Total)	veh-h/h	22.2	26.7 pers-h/h
Desired Speed	km/h	67.6	2011 0010 1.011
peed Efficiency		0.77	
ravel Time Index		7.49	
Congestion Coefficient		1.29	
		1.20	
Demand Flows (Total)	veh/h	1931	2317 pers/h
Arrival Flows (Total)	veh/h	1931	
Percent Heavy Vehicles (Demand)	%	5.2	
Percent Heavy Vehicles (Arrivals)	%	5.2	
Degree of Saturation		0.510	
Practical Spare Capacity	%	66.7	
Effective Intersection Capacity	veh/h	3786	
Control Delay (Total)	veh-h/h	3.03	3.63 pers-h/h
Control Delay (Average)	sec	5.6	5.6 sec
Control Delay (Worst Lane by MC)	sec	9.0	
Control Delay (Worst Movement by MC)	sec	16.5	16.5 sec
Geometric Delay (Average)	sec	4.3	
Stop-Line Delay (Average)	sec	1.3	
dling Time (Average)	sec	0.1	
ntersection Level of Service (LOS)		LOS A	
		0.0	
95% Back of Queue - Veh (Worst Lane)	veh	3.9	
95% Back of Queue - Dist (Worst Lane)	m	28.8	
Ave. Que Storage Ratio (Worst Lane)		0.13	1110 //
Effective Stops (Total)	veh/h	955	1146 pers/h
Effective Stop Rate		0.49	0.49
Proportion Queued		0.46	0.46
Performance Index		47.1	47.1
Cost (Total)	\$/h	1175.09	1175.09 \$/h
Fuel Consumption (Total)	L/h	150.8	1170.00 ψ/Π
Carbon Dioxide (Total)	kg/h	358.4	
Tydrocarbons (Total)	kg/h	0.034	
Carbon Monoxide (Total)	kg/h	0.034	
NOx (Total)	kg/h	0.776	

Site Level of Service (LOS) Method: Delay (RTA NSW). Site LOS Method is specified in the Parameter Settings dialog (Options tab).

Intersection LOS value for Vehicles is based on average delay for all vehicle movements.

Roundabout Capacity Model: SIDRA Standard.

Delay Model: SIDRA Standard (Control Delay: Geometric Delay is included).

Queue Model: SIDRA queue estimation methods are used for Back of Queue and Queue at Start of Gap.

Arrival Flows used in performance calculations are adjusted to include any Initial Queued Demand effects.

In Network analysis, Arrival Flows will be reduced if Upstream Capacity Constraint exists.

Gap-Acceptance Capacity Formula: SIDRA Standard (Akçelik M3D).

Site Model Variability Index (Average value of largest changes in Lane Degrees of Saturation from the third to the last Main (Timing-Capacity) Iterations): 2.5 %

Number of Iterations: 7 (Maximum: 10)

Largest change in Lane Degrees of Saturation for the last three Flow-Capacity Iterations: 2.1% 1.1% 0.6%

Intersection Performance - Annual Values				
Performance Measure	Vehicles:	All MCs	Persons	
Demand Flows (Total)	veh/y	926,653	1,111,983 pers/y	

Delay (Total)	veh-h/y	1,453	1,743 pers-h/y
Effective Stops (Total)	veh/y	458,587	550,304 pers/y
Travel Distance (Total)	veh-km/y	558,341	670,009 pers-km/y
Travel Time (Total)	veh-h/y	10,667	12,800 pers-h/y
Cost (Total)	\$/y	564,044	564,044 \$/y
Fuel Consumption (Total)	L/y	72,367	
Carbon Dioxide (Total)	kg/y	172,052	
Hydrocarbons (Total)	kg/y	16	
Carbon Monoxide (Total)	kg/y	225	
NOx (Total)	kg/y	373	

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## V Site: 101C [AM Future 2033 Weekday Peak 8:00am - 9:00am (Site Folder: Kingsland Parade and Clarrie Hermes Drive)]

Output produced by SIDRA INTERSECTION Version: 9.1.1.200

NA Site Category: (None) Roundabout

Intersection Performance - Hourly Values			
Performance Measure	Vehicles:	All MCs	Persons
Travel Speed (Average) Travel Distance (Total) Travel Time (Total) Desired Speed Speed Efficiency Travel Time Index Congestion Coefficient	km/h veh-km/h veh-h/h km/h	51.0 1401.9 27.5 68.1 0.75 7.21 1.33	51.0 km/h 1682.2 pers-km/h 33.0 pers-h/h
Demand Flows (Total) Arrival Flows (Total) Percent Heavy Vehicles (Demand) Percent Heavy Vehicles (Arrivals) Degree of Saturation Practical Spare Capacity Effective Intersection Capacity	veh/h veh/h % % veh/h	2315 2315 5.2 5.2 0.638 33.3 3630	2778 pers/h
Control Delay (Total) Control Delay (Average) Control Delay (Worst Lane by MC) Control Delay (Worst Movement by MC) Geometric Delay (Average) Stop-Line Delay (Average) Idling Time (Average) Intersection Level of Service (LOS)	veh-h/h sec sec sec sec sec sec	4.27 6.6 11.6 16.9 4.4 2.3 0.5 LOS A	5.13 pers-h/h 6.6 sec 16.9 sec
95% Back of Queue - Veh (Worst Lane) 95% Back of Queue - Dist (Worst Lane) Ave. Que Storage Ratio (Worst Lane) Effective Stops (Total) Effective Stop Rate Proportion Queued Performance Index	veh m veh/h	5.9 43.7 0.23 1301 0.56 0.57 66.3	1561 pers/h 0.56 0.57 66.3
Cost (Total) Fuel Consumption (Total) Carbon Dioxide (Total) Hydrocarbons (Total) Carbon Monoxide (Total) NOx (Total)	\$/h L/h kg/h kg/h kg/h kg/h	1457.16 187.9 446.7 0.043 0.58 0.978	1457.16 \$/h

Site Level of Service (LOS) Method: Delay (RTA NSW). Site LOS Method is specified in the Parameter Settings dialog (Options tab).

Intersection LOS value for Vehicles is based on average delay for all vehicle movements.

Roundabout Capacity Model: SIDRA Standard.

Delay Model: SIDRA Standard (Control Delay: Geometric Delay is included).

Queue Model: SIDRA queue estimation methods are used for Back of Queue and Queue at Start of Gap.

Arrival Flows used in performance calculations are adjusted to include any Initial Queued Demand effects.

In Network analysis, Arrival Flows will be reduced if Upstream Capacity Constraint exists.

Gap-Acceptance Capacity Formula: SIDRA Standard (Akçelik M3D).

Site Model Variability Index (Average value of largest changes in Lane Degrees of Saturation from the third to the last Main (Timing-Capacity) Iterations): 3.6 %

Number of Iterations: 8 (Maximum: 10)

Largest change in Lane Degrees of Saturation for the last three Flow-Capacity Iterations: 2.0% 1.1% 0.6%

Intersection Performance - Annual Values				
Performance Measure	Vehicles:	All MCs	Persons	
Demand Flows (Total)	veh/y	1,111,074	1,333,289 pers/y	
Delay (Total)	veh-h/y	2,050	2,460 pers-h/y	

Effective Stops (Total)	veh/y	624,524	749,429 pers/y	
Travel Distance (Total)	veh-km/y	672,889	807,467 pers-km/y	
Travel Time (Total)	veh-h/y	13,192	15,830 pers-h/y	
Cost (Total) Fuel Consumption (Total) Carbon Dioxide (Total) Hydrocarbons (Total) Carbon Monoxide (Total) NOx (Total)	\$/y L/y kg/y kg/y kg/y kg/y	699,437 90,195 214,398 20 280 469	699,437 \$/y	

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Project: J:\YEAR 2022 JOBS\220895-00 Block 9 Section 132 Casey Apartment + Sky Terrace\G - Design Calculations\CIVIL\Casey Apartments.sip9

# V Site: 101D [PM Base 2022 Weekday Peak 5:15pm - 6:15pm (Site Folder: Kingsland Parade and Clarrie Hermes Drive)]

Output produced by SIDRA INTERSECTION Version: 9.1.1.200

NA Site Category: (None) Roundabout

Intersection Performance - Hourly Va	alues		
Performance Measure	Vehicles:	All MCs	Persons
Travel Speed (Average) Travel Distance (Total) Travel Time (Total) Desired Speed Speed Efficiency Travel Time Index Congestion Coefficient	km/h veh-km/h veh-h/h km/h	51.3 1286.6 25.1 70.0 0.73 7.02 1.37	51.3 km/h 1543.9 pers-km/h 30.1 pers-h/h
Demand Flows (Total) Arrival Flows (Total) Percent Heavy Vehicles (Demand) Percent Heavy Vehicles (Arrivals) Degree of Saturation Practical Spare Capacity Effective Intersection Capacity	veh/h veh/h % % veh/h	2184 2184 1.5 1.5 0.629 35.1 3472	2621 pers/h
Control Delay (Total) Control Delay (Average) Control Delay (Worst Lane by MC) Control Delay (Worst Movement by MC) Geometric Delay (Average) Stop-Line Delay (Average) Idling Time (Average) Intersection Level of Service (LOS)	veh-h/h sec sec sec sec sec sec sec	4.03 6.6 8.0 14.8 5.0 1.6 0.1 LOS A	4.84 pers-h/h 6.6 sec 14.8 sec
95% Back of Queue - Veh (Worst Lane) 95% Back of Queue - Dist (Worst Lane) Ave. Que Storage Ratio (Worst Lane) Effective Stops (Total) Effective Stop Rate Proportion Queued Performance Index	veh m veh/h	5.6 39.6 0.13 1213 0.56 0.61 55.5	1455 pers/h 0.56 0.61 55.5
Cost (Total) Fuel Consumption (Total) Carbon Dioxide (Total) Hydrocarbons (Total) Carbon Monoxide (Total) NOx (Total)	\$/h L/h kg/h kg/h kg/h kg/h	1281.90 153.5 362.0 0.038 0.52 0.363	1281.90 \$/h

Site Level of Service (LOS) Method: Delay (RTA NSW). Site LOS Method is specified in the Parameter Settings dialog (Options tab).

Intersection LOS value for Vehicles is based on average delay for all vehicle movements.

Roundabout Capacity Model: SIDRA Standard.

Delay Model: SIDRA Standard (Control Delay: Geometric Delay is included).

Queue Model: SIDRA queue estimation methods are used for Back of Queue and Queue at Start of Gap.

Arrival Flows used in performance calculations are adjusted to include any Initial Queued Demand effects.

In Network analysis, Arrival Flows will be reduced if Upstream Capacity Constraint exists.

Gap-Acceptance Capacity Formula: SIDRA Standard (Akçelik M3D).

Site Model Variability Index (Average value of largest changes in Lane Degrees of Saturation from the third to the last Main (Timing-Capacity) Iterations): 3.5 %

Number of Iterations: 7 (Maximum: 10)

Largest change in Lane Degrees of Saturation for the last three Flow-Capacity Iterations: 3.0% 1.6% 0.9%

Intersection Performance - Annual Values										
Performance Measure	Vehicles:	All MCs	Persons							
Demand Flows (Total)	veh/y	1,048,421	1,258,105 pers/y							
Delay (Total)	veh-h/y	1,936	2,323 pers-h/y							

Effective Stops (Total)	veh/y	582,015	698,418 pers/y	
Travel Distance (Total)	veh-km/y	617,549	741,059 pers-km/y	
Travel Time (Total)	veh-h/y	12,048	14,458 pers-h/y	
Cost (Total) Fuel Consumption (Total) Carbon Dioxide (Total) Hydrocarbons (Total) Carbon Monoxide (Total) NOx (Total)	\$/y L/y kg/y kg/y kg/y kg/y	615,311 73,678 173,752 18 251 174	615,311 \$/y	

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# V Site: 101D [PM Development 2023 Weekday Peak 5:15pm - 6:15pm (Site Folder: Kingsland Parade and Clarrie Hermes Drive)]

Output produced by SIDRA INTERSECTION Version: 9.1.1.200

NA

Site Category: (None) Roundabout

Intersection Performance - Hourly Va	alues		
Performance Measure	Vehicles:	All MCs	Persons
Travel Speed (Average) Travel Distance (Total) Travel Time (Total) Desired Speed Speed Efficiency Travel Time Index Congestion Coefficient	km/h veh-km/h veh-h/h km/h	49.8 1380.6 27.7 70.2 0.71 6.78 1.41	49.8 km/h 1656.8 pers-km/h 33.2 pers-h/h
Demand Flows (Total) Arrival Flows (Total) Percent Heavy Vehicles (Demand) Percent Heavy Vehicles (Arrivals) Degree of Saturation Practical Spare Capacity Effective Intersection Capacity	veh/h veh/h % % veh/h	2375 2375 1.2 1.2 0.715 18.8 3319	2850 pers/h
Control Delay (Total) Control Delay (Average) Control Delay (Worst Lane by MC) Control Delay (Worst Movement by MC) Geometric Delay (Average) Stop-Line Delay (Average) Idling Time (Average) Intersection Level of Service (LOS)	veh-h/h sec sec sec sec sec sec sec	5.22 7.9 10.1 15.4 5.4 2.5 0.1 LOS A	6.26 pers-h/h 7.9 sec 15.4 sec
95% Back of Queue - Veh (Worst Lane) 95% Back of Queue - Dist (Worst Lane) Ave. Que Storage Ratio (Worst Lane) Effective Stops (Total) Effective Stop Rate Proportion Queued Performance Index	veh m veh/h	8.2 57.6 0.16 1518 0.64 0.73 69.8	1822 pers/h 0.64 0.73 69.8
Cost (Total) Fuel Consumption (Total) Carbon Dioxide (Total) Hydrocarbons (Total) Carbon Monoxide (Total) NOx (Total)	\$/h L/h kg/h kg/h kg/h kg/h	1409.52 167.5 394.7 0.042 0.57 0.329	1409.52 \$/h

Site Level of Service (LOS) Method: Delay (RTA NSW). Site LOS Method is specified in the Parameter Settings dialog (Options tab).

Intersection LOS value for Vehicles is based on average delay for all vehicle movements.

Roundabout Capacity Model: SIDRA Standard.

Delay Model: SIDRA Standard (Control Delay: Geometric Delay is included).

Queue Model: SIDRA queue estimation methods are used for Back of Queue and Queue at Start of Gap.

Arrival Flows used in performance calculations are adjusted to include any Initial Queued Demand effects.

In Network analysis, Arrival Flows will be reduced if Upstream Capacity Constraint exists.

Gap-Acceptance Capacity Formula: SIDRA Standard (Akçelik M3D).

Site Model Variability Index (Average value of largest changes in Lane Degrees of Saturation from the third to the last Main (Timing-Capacity) Iterations): 4.2 %

Number of Iterations: 8 (Maximum: 10)

Largest change in Lane Degrees of Saturation for the last three Flow-Capacity Iterations: 2.5% 1.4% 0.7%

Intersection Performance - Annual Values										
Performance Measure	Vehicles:	All MCs	Persons							
Demand Flows (Total)	veh/y	1,139,874	1,367,849 pers/y							

Delay (Total)	veh-h/y	2,505	3,006 pers-h/y
Effective Stops (Total)	veh/y	728,606	874,327 pers/y
Travel Distance (Total)	veh-km/y	662,703	795,244 pers-km/y
Travel Time (Total)	veh-h/y	13,294	15,953 pers-h/y
Cost (Total)	\$/y	676,571	676,571 \$/y
Fuel Consumption (Total)	L/y	80,416	
Carbon Dioxide (Total)	kg/y	189,441	
Hydrocarbons (Total)	kg/y	20	
Carbon Monoxide (Total)	kg/y	273	
NOx (Total)	kg/y	158	

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### V Site: 101D [PM Future 2033 Weekday Peak 5:15pm - 6:15pm (Site Folder: Kingsland Parade and Clarrie Hermes Drive)]

Output produced by SIDRA INTERSECTION Version: 9.1.1.200

NA Site Category: (None) Roundabout

Intersection Performance - Hourly Va	alues		
Performance Measure	Vehicles:	All MCs	Persons
Travel Speed (Average) Travel Distance (Total) Travel Time (Total) Desired Speed Speed Efficiency Travel Time Index Congestion Coefficient	km/h veh-km/h veh-h/h km/h	43.7 1677.8 38.4 70.0 0.62 5.83 1.60	43.7 km/h 2013.4 pers-km/h 46.0 pers-h/h
Demand Flows (Total) Arrival Flows (Total) Percent Heavy Vehicles (Demand) Percent Heavy Vehicles (Arrivals) Degree of Saturation Practical Spare Capacity Effective Intersection Capacity	veh/h veh/h % % veh/h	2879 2879 1.5 1.5 0.930 -8.6 3094	3455 pers/h
Control Delay (Total) Control Delay (Average) Control Delay (Worst Lane by MC) Control Delay (Worst Movement by MC) Geometric Delay (Average) Stop-Line Delay (Average) Idling Time (Average) Intersection Level of Service (LOS)	veh-h/h sec sec sec sec sec sec sec	11.99 15.0 19.0 25.0 5.3 9.7 2.2 LOS B	14.38 pers-h/h 15.0 sec 25.0 sec
95% Back of Queue - Veh (Worst Lane) 95% Back of Queue - Dist (Worst Lane) Ave. Que Storage Ratio (Worst Lane) Effective Stops (Total) Effective Stop Rate Proportion Queued Performance Index	veh m veh/h	24.3 172.5 0.48 2869 1.00 0.97 150.2	3443 pers/h 1.00 0.97 150.2
Cost (Total) Fuel Consumption (Total) Carbon Dioxide (Total) Hydrocarbons (Total) Carbon Monoxide (Total) NOx (Total)	\$/h L/h kg/h kg/h kg/h kg/h	1937.65 226.6 534.2 0.056 0.73 0.540	1937.65 \$/h

Site Level of Service (LOS) Method: Delay (RTA NSW). Site LOS Method is specified in the Parameter Settings dialog (Options tab).

Intersection LOS value for Vehicles is based on average delay for all vehicle movements.

Roundabout Capacity Model: SIDRA Standard.

Delay Model: SIDRA Standard (Control Delay: Geometric Delay is included).

Queue Model: SIDRA queue estimation methods are used for Back of Queue and Queue at Start of Gap.

Arrival Flows used in performance calculations are adjusted to include any Initial Queued Demand effects.

In Network analysis, Arrival Flows will be reduced if Upstream Capacity Constraint exists.

Gap-Acceptance Capacity Formula: SIDRA Standard (Akçelik M3D).

Site Model Variability Index (Average value of largest changes in Lane Degrees of Saturation from the third to the last Main (Timing-Capacity) Iterations): 6.2 %

Number of Iterations: 9 (Maximum: 10)

Largest change in Lane Degrees of Saturation for the last three Flow-Capacity Iterations: 2.8% 1.6% 0.9%

Intersection Performance - Annual Values										
Performance Measure	Vehicles:	All MCs	Persons							
Demand Flows (Total)	veh/y	1,381,895	1,658,274 pers/y							
Delay (Total)	veh-h/y	5,754	6,905 pers-h/y							

Effective Stops (Total)	veh/y	1,377,312	1,652,775 pers/y	
Travel Distance (Total)	veh-km/y	805,368	966,441 pers-km/y	
Travel Time (Total)	veh-h/y	18,415	22,098 pers-h/y	
Cost (Total) Fuel Consumption (Total) Carbon Dioxide (Total) Hydrocarbons (Total) Carbon Monoxide (Total) NOx (Total)	\$/y L/y kg/y kg/y kg/y kg/y	930,070 108,763 256,404 27 351 259	930,070 \$/y	

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Project: J:\YEAR 2022 JOBS\220895-00 Block 9 Section 132 Casey Apartment + Sky Terrace\G - Design Calculations\CIVIL\Casey Apartments.sip9

# V Site: 101C [AM Base 2022 Weekday Peak 8:00am - 9:00am (Site Folder: Kingsland Parade and Clarrie Hermes Drive)]

Output produced by SIDRA INTERSECTION Version: 9.1.1.200

NA Site Category: (None) Roundabout

Vehi	cle Mo	ovement	t Perfo	rma	nce										
Mov ID	Turn	Mov Class	[ Total	lows HV ]	Fl [ Total ]		Deg. Satn	Aver. Delay	Level of Service	Qu [ Veh.	Back Of leue Dist ]	Prop. Que	Eff. Stop Rate	Aver. No. of Cycles	Aver. Speed
South	: Clarr	rie Herme	veh/h es Drive	%	veh/h	%	v/c	sec	_	veh	m	-	-	_	km/h
1		All MCs		0.0	6	0.0	0.040	5.3	LOS A	0.2	1.7	0.71	0.67	0.71	39.5
2	T1	All MCs	4	0.0	4	0.0	0.040	4.2	LOS A	0.2	1.7	0.71	0.67	0.71	33.6
3	R2	All MCs	24	0.0	24	0.0	0.040	11.1	LOS A	0.2	1.7	0.71	0.67	0.71	39.2
Appro	bach		35	0.0	35	0.0	0.040	9.2	LOS A	0.2	1.7	0.71	0.67	0.71	38.8
East:	Clarrie	e Hermes	Drive												
4	L2	All MCs	28	0.0	28	0.0	0.513	5.4	LOS A	3.9	28.8	0.41	0.48	0.41	54.7
5	T1	All MCs	655	5.5	655	5.5	0.513	5.4	LOS A	3.9	28.8	0.41	0.48	0.41	60.1
6	R2	All MCs	86	7.3	86	7.3	0.513	13.3	LOS A	3.9	28.8	0.41	0.48	0.41	41.7
6u	U	All MCs	6	0.0	6	0.0	0.513	16.4	LOS B	3.9	28.8	0.41	0.48	0.41	58.9
Appro	bach		776	5.4	776	5.4	0.513	6.4	LOS A	3.9	28.8	0.41	0.48	0.41	57.9
North	: Kings	sland Par	ade												
7	L2	All MCs	179	5.9	179	5.9	0.306	3.1	LOS A	1.9	14.1	0.67	0.57	0.67	35.5
8	T1	All MCs	3	0.0	3	0.0	0.306	2.8	LOS A	1.9	14.1	0.67	0.57	0.67	32.3
9	R2	All MCs	133	3.2	133	3.2	0.306	7.5	LOS A	1.9	14.1	0.67	0.57	0.67	35.0
Appro	bach		315	4.7	315	4.7	0.306	4.9	LOS A	1.9	14.1	0.67	0.57	0.67	35.3
West	Clarri	e Hermes	s Drive												
10	L2	All MCs	108	2.9	108	2.9	0.432	5.4	LOS A	2.8	20.8	0.33	0.44	0.33	41.7
11	T1	All MCs	546	6.4	546	6.4	0.432	5.3	LOS A	2.8	20.8	0.33	0.44	0.33	61.5
12	R2	All MCs	8	0.0	8	0.0	0.432	13.1	LOS A	2.8	20.8	0.33	0.44	0.33	54.4
Appro	bach		663	5.7	663	5.7	0.432	5.4	LOS A	2.8	20.8	0.33	0.44	0.33	58.4
All Ve	hicles		1788	5.3	1788	5.3	0.513	5.8	LOS A	3.9	28.8	0.43	0.49	0.43	53.3

Site Level of Service (LOS) Method: Delay (RTA NSW). Site LOS Method is specified in the Parameter Settings dialog (Options tab).

Vehicle movement LOS values are based on average delay per movement.

Intersection and Approach LOS values are based on average delay for all vehicle movements.

Roundabout Capacity Model: SIDRA Standard.

Delay Model: SIDRA Standard (Control Delay: Geometric Delay is included).

Queue Model: SIDRA queue estimation methods are used for Back of Queue and Queue at Start of Gap.

Gap-Acceptance Capacity Formula: SIDRA Standard (Akçelik M3D).

HV (%) values are calculated for All Movement Classes of All Heavy Vehicle Model Designation.

Arrival Flows used in performance calculations are adjusted to include any Initial Queued Demand and Upstream Capacity Constraint effects.

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#### W Site: 101C [AM Development 2023 Weekday Peak 8:00am -9:00am (Site Folder: Kingsland Parade and Clarrie Hermes Drive)]

Output produced by SIDRA INTERSECTION Version: 9.1.1.200

NA

Site Category: (None) Roundabout

Vehi	cle <u>M</u> o	ovemen	t Perf <u>o</u>	rm <u>a</u>	nce										
Mov ID	Turn	Mov Class		lows HV ]		rival lows HV ] %	Deg. Satn v/c	Aver. Delay sec	Level of Service		ack Of eue Dist ] m	Prop. Que	Eff. Stop Rate	Aver. No. of Cycles	Aver. Speed km/h
South	: Clarr	rie Herme	es Drive												
1	L2	All MCs	7	0.0	7	0.0	0.044	5.3	LOS A	0.3	1.9	0.72	0.67	0.72	39.6
2	T1	All MCs	5	0.0	5	0.0	0.044	4.2	LOS A	0.3	1.9	0.72	0.67	0.72	33.7
3	R2	All MCs	25	0.0	25	0.0	0.044	11.1	LOS A	0.3	1.9	0.72	0.67	0.72	39.3
Appro	bach		38	0.0	38	0.0	0.044	9.0	LOS A	0.3	1.9	0.72	0.67	0.72	38.9
East:	East: Clarrie Hermes Drive														
4	L2	All MCs	29	0.0	29	0.0	0.510	5.6	LOS A	3.9	28.8	0.46	0.49	0.46	54.7
5	T1	All MCs	668	5.5	668	5.5	0.510	5.6	LOS A	3.9	28.8	0.46	0.49	0.46	60.1
6	R2	All MCs	43	17.1	43	17.1	0.510	13.7	LOS A	3.9	28.8	0.46	0.49	0.46	41.5
6u	U	All MCs	7	0.0	7	0.0	0.510	16.5	LOS B	3.9	28.8	0.46	0.49	0.46	58.9
Appro	bach		748	5.9	748	5.9	0.510	6.1	LOS A	3.9	28.8	0.46	0.49	0.46	58.9
North	: Kings	sland Par	ade												
7	L2	All MCs	299	3.9	299	3.9	0.445	3.6	LOS A	3.1	22.3	0.73	0.60	0.73	35.4
8	T1	All MCs	4	0.0	4	0.0	0.445	3.3	LOS A	3.1	22.3	0.73	0.60	0.73	32.1
9	R2	All MCs	158	3.3	158	3.3	0.445	8.0	LOS A	3.1	22.3	0.73	0.60	0.73	35.0
Appro	bach		461	3.7	461	3.7	0.445	5.1	LOS A	3.1	22.3	0.73	0.60	0.73	35.2
West:	Clarri	e Herme	s Drive												
10	L2	All MCs	116	3.6	116	3.6	0.428	5.2	LOS A	2.8	20.6	0.26	0.42	0.26	42.1
11	T1	All MCs	558	6.4	558	6.4	0.428	5.2	LOS A	2.8	20.6	0.26	0.42	0.26	62.2
12	R2	All MCs	9	0.0	9	0.0	0.428	12.9	LOS A	2.8	20.6	0.26	0.42	0.26	55.1
Appro	bach		683	5.9	683	5.9	0.428	5.3	LOS A	2.8	20.6	0.26	0.42	0.26	58.9
All Ve	hicles		1931	5.2	1931	5.2	0.510	5.6	LOS A	3.9	28.8	0.46	0.49	0.46	52.3

Site Level of Service (LOS) Method: Delay (RTA NSW). Site LOS Method is specified in the Parameter Settings dialog (Options tab).

Vehicle movement LOS values are based on average delay per movement.

Intersection and Approach LOS values are based on average delay for all vehicle movements.

Roundabout Capacity Model: SIDRA Standard.

Delay Model: SIDRA Standard (Control Delay: Geometric Delay is included).

Queue Model: SIDRA queue estimation methods are used for Back of Queue and Queue at Start of Gap.

Gap-Acceptance Capacity Formula: SIDRA Standard (Akçelik M3D).

HV (%) values are calculated for All Movement Classes of All Heavy Vehicle Model Designation.

Arrival Flows used in performance calculations are adjusted to include any Initial Queued Demand and Upstream Capacity Constraint effects.

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# V Site: 101C [AM Future 2033 Weekday Peak 8:00am - 9:00am (Site Folder: Kingsland Parade and Clarrie Hermes Drive)]

Output produced by SIDRA INTERSECTION Version: 9.1.1.200

NA Site Category: (None) Roundabout

Vehi	cle Mo	ovement	t Perfo	rma	nce										
Mov ID	Turn	Mov Class	Fi [ Total		Fl [ Total ]		Deg. Satn	Aver. Delay	Level of Service	Qı. [ Veh.	Back Of ieue Dist ]	Prop. Que	Eff. Stop Rate	Aver. No. of Cycles	Aver. Speed
South	n: Clarr	rie Herme	veh/h s Drive	%	veh/h	%	v/c	sec	_	veh	m	_	_	_	km/h
1	L2	All MCs	8	0.0	8	0.0	0.067	7.7	LOS A	0.5	3.3	0.86	0.74	0.86	37.7
2	T1	All MCs	5	0.0	5	0.0	0.067	6.5	LOS A	0.5	3.3	0.86	0.74	0.86	31.2
3	R2	All MCs	31	0.0	31	0.0	0.067	13.5	LOS A	0.5	3.3	0.86	0.74	0.86	37.5
Appro	bach		44	0.0	44	0.0	0.067	11.6	LOS A	0.5	3.3	0.86	0.74	0.86	37.1
East:	Clarrie	e Hermes	Drive												
4	L2	All MCs	36	0.0	36	0.0	0.638	5.9	LOS A	5.9	43.7	0.59	0.51	0.59	53.2
5	T1	All MCs	816	5.5	816	5.5	0.638	5.9	LOS A	5.9	43.7	0.59	0.51	0.59	58.8
6	R2	All MCs	51	16.7	51	16.7	0.638	14.0	LOS A	5.9	43.7	0.59	0.51	0.59	40.5
6u	U	All MCs	8	0.0	8	0.0	0.638	16.9	LOS B	5.9	43.7	0.59	0.51	0.59	57.7
Appro	bach		911	5.9	911	5.9	0.638	6.4	LOS A	5.9	43.7	0.59	0.51	0.59	57.6
North	: Kings	sland Par	ade												
7	L2	All MCs	338	4.0	338	4.0	0.576	7.0	LOS A	5.6	40.1	0.87	0.84	1.08	33.0
8	T1	All MCs	4	0.0	4	0.0	0.576	6.7	LOS A	5.6	40.1	0.87	0.84	1.08	28.6
9	R2	All MCs	186	2.8	186	2.8	0.576	11.4	LOS A	5.6	40.1	0.87	0.84	1.08	32.6
Appro	bach		528	3.6	528	3.6	0.576	8.5	LOS A	5.6	40.1	0.87	0.84	1.08	32.8
West	Clarri	e Hermes	s Drive												
10	L2	All MCs	140	3.0	140	3.0	0.527	5.3	LOS A	4.0	29.7	0.33	0.43	0.33	41.6
11	T1	All MCs	681	6.5	681	6.5	0.527	5.3	LOS A	4.0	29.7	0.33	0.43	0.33	61.4
12	R2	All MCs	11	0.0	11	0.0	0.527	13.0	LOS A	4.0	29.7	0.33	0.43	0.33	54.3
Appro	bach		832	5.8	832	5.8	0.527	5.4	LOS A	4.0	29.7	0.33	0.43	0.33	58.2
All Ve	hicles		2315	5.2	2315	5.2	0.638	6.6	LOS A	5.9	43.7	0.57	0.56	0.62	51.0

Site Level of Service (LOS) Method: Delay (RTA NSW). Site LOS Method is specified in the Parameter Settings dialog (Options tab).

Vehicle movement LOS values are based on average delay per movement.

Intersection and Approach LOS values are based on average delay for all vehicle movements.

Roundabout Capacity Model: SIDRA Standard.

Delay Model: SIDRA Standard (Control Delay: Geometric Delay is included).

Queue Model: SIDRA queue estimation methods are used for Back of Queue and Queue at Start of Gap.

Gap-Acceptance Capacity Formula: SIDRA Standard (Akçelik M3D).

HV (%) values are calculated for All Movement Classes of All Heavy Vehicle Model Designation.

Arrival Flows used in performance calculations are adjusted to include any Initial Queued Demand and Upstream Capacity Constraint effects.

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Project: J:\YEAR 2022 JOBS\220895-00 Block 9 Section 132 Casey Apartment + Sky Terrace\G - Design Calculations\CIVIL\Casey Apartments.sip9

#### **W** Site: 101D [PM Base 2022 Weekday Peak 5:15pm - 6:15pm (Site Folder: Kingsland Parade and Clarrie Hermes Drive)]

Output produced by SIDRA INTERSECTION Version: 9.1.1.200

NA Site Category: (None) Roundabout

Vehi	cle Mo	ovement	t Per <u>fo</u>	rma	nce										
Mov ID	Turn	Mov Class		lows HV ]		rival lows HV ] %	Deg. Satn v/c	Aver. Delay sec	Level of Service		Back Of eue Dist ] m	Prop. Que	Eff. Stop Rate	Aver. No. of Cycles	Aver. Speed km/h
South	n: Clarr	rie Herme	s Drive												
1	L2	All MCs	1	0.0	1	0.0	0.028	6.4	LOS A	0.2	1.3	0.79	0.64	0.79	40.2
2	T1	All MCs	12	0.0	12	0.0	0.028	5.2	LOS A	0.2	1.3	0.79	0.64	0.79	34.4
3	R2	All MCs	8	0.0	8	0.0	0.028	12.2	LOS A	0.2	1.3	0.79	0.64	0.79	39.8
Appro	bach		21	0.0	21	0.0	0.028	8.0	LOS A	0.2	1.3	0.79	0.64	0.79	37.6
East:	Clarrie	e Hermes	Drive												
4	L2	All MCs	41	0.0	41	0.0	0.580	5.9	LOS A	4.9	34.4	0.57	0.55	0.57	52.6
5	T1	All MCs	616	0.7	616	0.7	0.580	5.9	LOS A	4.9	34.4	0.57	0.55	0.57	59.0
6	R2	All MCs	177	1.8	177	1.8	0.580	13.7	LOS A	4.9	34.4	0.57	0.55	0.57	50.1
Appro	bach		834	0.9	834	0.9	0.580	7.5	LOS A	4.9	34.4	0.57	0.55	0.57	57.1
North	: Kings	sland Par	ade												
7	L2	All MCs	187	0.0	187	0.0	0.429	3.8	LOS A	3.2	22.9	0.81	0.66	0.81	34.7
8	T1	All MCs	6	0.0	6	0.0	0.429	3.6	LOS A	3.2	22.9	0.81	0.66	0.81	31.3
9	R2	All MCs	206	4.6	206	4.6	0.429	8.4	LOS A	3.2	22.9	0.81	0.66	0.81	34.3
Appro	bach		400	2.4	400	2.4	0.429	6.1	LOS A	3.2	22.9	0.81	0.66	0.81	34.5
West	: Clarri	e Hermes	s Drive												
10	L2	All MCs	261	0.8	261	0.8	0.629	5.9	LOS A	5.6	39.6	0.55	0.52	0.55	40.1
11	T1	All MCs	651	2.3	651	2.3	0.629	5.9	LOS A	5.6	39.6	0.55	0.52	0.55	60.2
12	R2	All MCs	18	0.0	18	0.0	0.629	13.7	LOS A	5.6	39.6	0.55	0.52	0.55	52.0
Appro	bach		929	1.8	929	1.8	0.629	6.0	LOS A	5.6	39.6	0.55	0.52	0.55	54.6
All Ve	hicles		2184	1.5	2184	1.5	0.629	6.6	LOS A	5.6	39.6	0.61	0.56	0.61	51.3

Site Level of Service (LOS) Method: Delay (RTA NSW). Site LOS Method is specified in the Parameter Settings dialog (Options tab).

Vehicle movement LOS values are based on average delay per movement.

Intersection and Approach LOS values are based on average delay for all vehicle movements.

Roundabout Capacity Model: SIDRA Standard.

Delay Model: SIDRA Standard (Control Delay: Geometric Delay is included).

Queue Model: SIDRA queue estimation methods are used for Back of Queue and Queue at Start of Gap.

Gap-Acceptance Capacity Formula: SIDRA Standard (Akcelik M3D).

HV (%) values are calculated for All Movement Classes of All Heavy Vehicle Model Designation.

Arrival Flows used in performance calculations are adjusted to include any Initial Queued Demand and Upstream Capacity Constraint effects.

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# V Site: 101D [PM Development 2023 Weekday Peak 5:15pm - 6:15pm (Site Folder: Kingsland Parade and Clarrie Hermes Drive)]

Output produced by SIDRA INTERSECTION Version: 9.1.1.200

NA

Site Category: (None) Roundabout

Vehi	cle Mo	ovement	t Perfo	rma	nce										
Mov ID	Turn	Mov Class		lows HV ]		rival lows HV ] %	Deg. Satn v/c	Aver. Delay sec	Level of Service	95% B Que [ Veh. veh		Prop. Que	Eff. Stop Rate	Aver. No. of Cycles	Aver Speed km/r
South	: Clarr	ie Herme	s Drive												
1	L2	All MCs	2	0.0	2	0.0	0.040	8.5	LOS A	0.3	2.1	0.89	0.72	0.89	38.7
2	T1	All MCs	13	0.0	13	0.0	0.040	7.3	LOS A	0.3	2.1	0.89	0.72	0.89	32.3
3	R2	All MCs	9	0.0	9	0.0	0.040	14.3	LOS A	0.3	2.1	0.89	0.72	0.89	38.4
Appro	bach		24	0.0	24	0.0	0.040	10.1	LOS A	0.3	2.1	0.89	0.72	0.89	36.0
East:	Clarrie	e Hermes	Drive												
4	L2	All MCs	42	0.0	42	0.0	0.677	6.2	LOS A	6.6	46.7	0.66	0.58	0.66	51.2
5	T1	All MCs	629	0.8	629	0.8	0.677	6.1	LOS A	6.6	46.7	0.66	0.58	0.66	57.7
6	R2	All MCs	297	1.4	297	1.4	0.677	14.0	LOS A	6.6	46.7	0.66	0.58	0.66	48.8
Appro	bach		968	1.0	968	1.0	0.677	8.5	LOS A	6.6	46.7	0.66	0.58	0.66	55.1
North	: Kings	sland Par	ade												
7	L2	All MCs	202	0.0	202	0.0	0.475	4.3	LOS A	4.0	28.4	0.86	0.72	0.91	34.5
8	T1	All MCs	7	0.0	7	0.0	0.475	4.2	LOS A	4.0	28.4	0.86	0.72	0.91	30.9
9	R2	All MCs	211	4.5	211	4.5	0.475	8.9	LOS A	4.0	28.4	0.86	0.72	0.91	34.0
Appro	bach		420	2.3	420	2.3	0.475	6.6	LOS A	4.0	28.4	0.86	0.72	0.91	34.2
West:	Clarri	e Hermes	s Drive												
10	L2	All MCs	288	1.1	288	1.1	0.715	7.7	LOS A	8.2	57.6	0.75	0.66	0.83	38.7
11	T1	All MCs	655	0.8	655	0.8	0.715	7.6	LOS A	8.2	57.6	0.75	0.66	0.83	58.6
12	R2	All MCs	19	0.0	19	0.0	0.715	15.4	LOS B	8.2	57.6	0.75	0.66	0.83	50.0
Appro	bach		962	0.9	962	0.9	0.715	7.8	LOS A	8.2	57.6	0.75	0.66	0.83	52.7
All Ve	hicles		2375	1.2	2375	1.2	0.715	7.9	LOS A	8.2	57.6	0.73	0.64	0.77	49.8

Site Level of Service (LOS) Method: Delay (RTA NSW). Site LOS Method is specified in the Parameter Settings dialog (Options tab).

Vehicle movement LOS values are based on average delay per movement.

Intersection and Approach LOS values are based on average delay for all vehicle movements.

Roundabout Capacity Model: SIDRA Standard.

Delay Model: SIDRA Standard (Control Delay: Geometric Delay is included).

Queue Model: SIDRA queue estimation methods are used for Back of Queue and Queue at Start of Gap.

Gap-Acceptance Capacity Formula: SIDRA Standard (Akçelik M3D).

HV (%) values are calculated for All Movement Classes of All Heavy Vehicle Model Designation.

Arrival Flows used in performance calculations are adjusted to include any Initial Queued Demand and Upstream Capacity Constraint effects.

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#### V Site: 101D [PM Future 2033 Weekday Peak 5:15pm - 6:15pm (Site Folder: Kingsland Parade and Clarrie Hermes Drive)]

Output produced by SIDRA INTERSECTION Version: 9.1.1.200

NA Site Category: (None) Roundabout

Vehi	cle Mo	ovement	t Perfo	rma	nce										
Mov ID	Turn	Mov Class	F			rival lows HV ] %	Deg. Satn v/c	Aver. Delay sec	Level of Service	95% B Que [ Veh. veh	ack Of eue Dist ] m	Prop. Que	Eff. Stop Rate	Aver. No. of Cycles	Aver. Speed km/h
South	: Cları	rie Herme	s Drive												
1	L2	All MCs	2	0.0	2	0.0	0.080	15.5	LOS B	0.7	4.7	1.00	0.84	1.00	33.7
2	T1	All MCs	15	0.0	15	0.0	0.080	14.3	LOS A	0.7	4.7	1.00	0.84	1.00	26.0
3	R2	All MCs	11	0.0	11	0.0	0.080	21.3	LOS B	0.7	4.7	1.00	0.84	1.00	33.6
Appro	bach		27	0.0	27	0.0	0.080	17.1	LOS B	0.7	4.7	1.00	0.84	1.00	30.3
East:	Clarrie	e Hermes	Drive												
4	L2	All MCs	52	0.0	52	0.0	0.853	9.6	LOS A	14.8	104.6	0.93	0.79	1.11	48.2
5	T1	All MCs	766	0.7	766	0.7	0.853	9.6	LOS A	14.8	104.6	0.93	0.79	1.11	54.9
6	R2	All MCs	336	1.3	336	1.3	0.853	17.4	LOS B	14.8	104.6	0.93	0.79	1.11	45.8
Appro	bach		1154	0.8	1154	0.8	0.853	11.8	LOS A	14.8	104.6	0.93	0.79	1.11	52.4
North	: Kings	sland Par	ade												
7	L2	All MCs	244	0.0	244	0.0	0.791	16.6	LOS B	11.7	83.5	1.00	1.25	1.62	27.4
8	T1	All MCs	8	0.0	8	0.0	0.791	16.4	LOS B	11.7	83.5	1.00	1.25	1.62	21.7
9	R2	All MCs	266	4.0	266	4.0	0.791	21.2	LOS B	11.7	83.5	1.00	1.25	1.62	27.0
Appro	bach		519	2.0	519	2.0	0.791	19.0	LOS B	11.7	83.5	1.00	1.25	1.62	27.1
West	Clarri	e Hermes	s Drive												
10	L2	All MCs	346	0.9	346	0.9	0.930	16.1	LOS B	24.3	172.5	1.00	1.09	1.58	33.3
11	T1	All MCs	809	2.3	809	2.3	0.930	16.1	LOS B	24.3	172.5	1.00	1.09	1.58	50.5
12	R2	All MCs	23	0.0	23	0.0	0.930	23.9	LOS B	24.3	172.5	1.00	1.09	1.58	42.0
Appro	bach		1179	1.9	1179	1.9	0.930	16.3	LOS B	24.3	172.5	1.00	1.09	1.58	45.5
All Ve	hicles		2879	1.5	2879	1.5	0.930	15.0	LOS B	24.3	172.5	0.97	1.00	1.40	43.7

Site Level of Service (LOS) Method: Delay (RTA NSW). Site LOS Method is specified in the Parameter Settings dialog (Options tab).

Vehicle movement LOS values are based on average delay per movement.

Intersection and Approach LOS values are based on average delay for all vehicle movements.

Roundabout Capacity Model: SIDRA Standard.

Delay Model: SIDRA Standard (Control Delay: Geometric Delay is included).

Queue Model: SIDRA queue estimation methods are used for Back of Queue and Queue at Start of Gap.

Gap-Acceptance Capacity Formula: SIDRA Standard (Akcelik M3D).

HV (%) values are calculated for All Movement Classes of All Heavy Vehicle Model Designation.

Arrival Flows used in performance calculations are adjusted to include any Initial Queued Demand and Upstream Capacity Constraint effects.

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Project: J:\YEAR 2022 JOBS\220895-00 Block 9 Section 132 Casey Apartment + Sky Terrace\G - Design Calculations\CIVIL\Casey Apartments.sip9

# V Site: 101E [AM Base 2022 Weekday Peak 8:00am - 9:00am (Site Folder: Overall Avenue and Horse Park Drive)]

Output produced by SIDRA INTERSECTION Version: 9.1.1.200

New Site Site Category: (None) Roundabout

Intersection Performance - Hourly Va	alues		
Performance Measure	Vehicles:	All MCs	Persons
Travel Speed (Average) Travel Distance (Total) Travel Time (Total) Desired Speed Speed Efficiency Travel Time Index Congestion Coefficient	km/h veh-km/h veh-h/h km/h	51.8 1682.4 32.5 72.5 0.71 6.83 1.40	51.8 km/h 2018.9 pers-km/h 39.0 pers-h/h
Demand Flows (Total) Arrival Flows (Total) Percent Heavy Vehicles (Demand) Percent Heavy Vehicles (Arrivals) Degree of Saturation Practical Spare Capacity Effective Intersection Capacity	veh/h veh/h % % veh/h	2088 2088 3.2 3.2 0.869 -2.2 2404	2506 pers/h
Control Delay (Total) Control Delay (Average) Control Delay (Worst Lane by MC) Control Delay (Worst Movement by MC) Geometric Delay (Average) Stop-Line Delay (Average) Idling Time (Average) Intersection Level of Service (LOS)	veh-h/h sec sec sec sec sec sec sec	7.01 12.1 16.7 25.5 6.3 5.8 1.1 LOS A	8.41 pers-h/h 12.1 sec 25.5 sec
95% Back of Queue - Veh (Worst Lane) 95% Back of Queue - Dist (Worst Lane) Ave. Que Storage Ratio (Worst Lane) Effective Stops (Total) Effective Stop Rate Proportion Queued Performance Index	veh m veh/h	16.6 119.1 0.10 1583 0.76 0.86 95.6	1899 pers/h 0.76 0.86 95.6
Cost (Total) Fuel Consumption (Total) Carbon Dioxide (Total) Hydrocarbons (Total) Carbon Monoxide (Total) NOx (Total)	\$/h L/h kg/h kg/h kg/h kg/h	1678.05 205.7 487.0 0.049 0.71 0.801	1678.05 \$/h

Site Level of Service (LOS) Method: Delay (RTA NSW). Site LOS Method is specified in the Parameter Settings dialog (Options tab).

Intersection LOS value for Vehicles is based on average delay for all vehicle movements.

Roundabout Capacity Model: SIDRA Standard.

Delay Model: SIDRA Standard (Control Delay: Geometric Delay is included).

Queue Model: SIDRA queue estimation methods are used for Back of Queue and Queue at Start of Gap.

Arrival Flows used in performance calculations are adjusted to include any Initial Queued Demand effects.

In Network analysis, Arrival Flows will be reduced if Upstream Capacity Constraint exists.

Gap-Acceptance Capacity Formula: SIDRA Standard (Akçelik M3D).

Site Model Variability Index (Average value of largest changes in Lane Degrees of Saturation from the third to the last Main (Timing-Capacity) Iterations): 6.4 %

Number of Iterations: 9 (Maximum: 10)

Largest change in Lane Degrees of Saturation for the last three Flow-Capacity Iterations: 2.5% 1.4% 0.8%

Intersection Performance - Annual Values								
Performance Measure	Vehicles:	All MCs	Persons					
Demand Flows (Total)	veh/y	1,002,442	1,202,931 pers/y					
Delay (Total)	veh-h/y	3,366	4,039 pers-h/y					

Effective Stops (Total)	veh/y	759,626	911,551 pers/y	
Travel Distance (Total)	veh-km/y	807,549	969,059 pers-km/y	
Travel Time (Total)	veh-h/y	15,593	18,712 pers-h/y	
Cost (Total) Fuel Consumption (Total) Carbon Dioxide (Total) Hydrocarbons (Total) Carbon Monoxide (Total) NOx (Total)	\$/y L/y kg/y kg/y kg/y kg/y	805,464 98,724 233,755 23 341 384	805,464 \$/y	

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# V Site: 101E [AM Development 2023 Weekday Peak 8:00am - 9:00am (Site Folder: Overall Avenue and Horse Park Drive)]

Output produced by SIDRA INTERSECTION Version: 9.1.1.200

New Site Site Category: (None) Roundabout

Intersection Performance - Hourly Va	alues		
Performance Measure	Vehicles:	All MCs	Persons
Travel Speed (Average) Travel Distance (Total) Travel Time (Total) Desired Speed Speed Efficiency Travel Time Index Congestion Coefficient	km/h veh-km/h veh-h/h km/h	49.9 1741.4 34.9 72.4 0.69 6.54 1.45	49.9 km/h 2089.7 pers-km/h 41.9 pers-h/h
Demand Flows (Total) Arrival Flows (Total) Percent Heavy Vehicles (Demand) Percent Heavy Vehicles (Arrivals) Degree of Saturation Practical Spare Capacity Effective Intersection Capacity	veh/h veh/h % % veh/h	2174 2174 3.5 3.5 0.912 -6.8 2384	2608 pers/h
Control Delay (Total) Control Delay (Average) Control Delay (Worst Lane by MC) Control Delay (Worst Movement by MC) Geometric Delay (Average) Stop-Line Delay (Average) Idling Time (Average) Intersection Level of Service (LOS)	veh-h/h sec sec sec sec sec sec sec	8.52 14.1 18.5 27.8 6.3 7.8 1.6 LOS A	10.22 pers-h/h 14.1 sec 27.8 sec
95% Back of Queue - Veh (Worst Lane) 95% Back of Queue - Dist (Worst Lane) Ave. Que Storage Ratio (Worst Lane) Effective Stops (Total) Effective Stop Rate Proportion Queued Performance Index	veh m veh/h	21.4 154.2 0.12 1840 0.85 0.89 112.9	2208 pers/h 0.85 0.89 112.9
Cost (Total) Fuel Consumption (Total) Carbon Dioxide (Total) Hydrocarbons (Total) Carbon Monoxide (Total) NOx (Total)	\$/h L/h kg/h kg/h kg/h kg/h	1796.55 218.3 517.0 0.052 0.74 0.887	1796.55 \$/h

Site Level of Service (LOS) Method: Delay (RTA NSW). Site LOS Method is specified in the Parameter Settings dialog (Options tab).

Intersection LOS value for Vehicles is based on average delay for all vehicle movements.

Roundabout Capacity Model: SIDRA Standard.

Delay Model: SIDRA Standard (Control Delay: Geometric Delay is included).

Queue Model: SIDRA queue estimation methods are used for Back of Queue and Queue at Start of Gap.

Arrival Flows used in performance calculations are adjusted to include any Initial Queued Demand effects.

In Network analysis, Arrival Flows will be reduced if Upstream Capacity Constraint exists.

Gap-Acceptance Capacity Formula: SIDRA Standard (Akçelik M3D).

Site Model Variability Index (Average value of largest changes in Lane Degrees of Saturation from the third to the last Main (Timing-Capacity) Iterations): 6.7 %

Number of Iterations: 9 (Maximum: 10)

Largest change in Lane Degrees of Saturation for the last three Flow-Capacity Iterations: 2.1% 1.1% 0.5%

Intersection Performance - Annual Values								
Performance Measure	Vehicles:	All MCs	Persons					
Demand Flows (Total)	veh/y	1,043,368	1,252,042 pers/y					
Delay (Total)	veh-h/y	4,087	4,905 pers-h/y					

Effective Stops (Total)	veh/y	883,054	1,059,665 pers/y	
Travel Distance (Total)	veh-km/y	835,888	1,003,066 pers-km/y	
Travel Time (Total)	veh-h/y	16,767	20,120 pers-h/y	
Cost (Total) Fuel Consumption (Total) Carbon Dioxide (Total) Hydrocarbons (Total) Carbon Monoxide (Total) NOx (Total)	\$/y L/y kg/y kg/y kg/y kg/y	862,345 104,773 248,184 25 356 426	862,345 \$/y	

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### V Site: 101E [AM Future 2033 Weekday Peak 8:00am - 9:00am (Site Folder: Overall Avenue and Horse Park Drive)]

Output produced by SIDRA INTERSECTION Version: 9.1.1.200

New Site Site Category: (None) Roundabout

Intersection Performance - Hourly Va	alues		
Performance Measure	Vehicles:	All MCs	Persons
Travel Speed (Average) Travel Distance (Total) Travel Time (Total) Desired Speed Speed Efficiency Travel Time Index	km/h veh-km/h veh-h/h km/h	21.3 2114.1 99.2 72.4 0.29 2.16	21.3 km/h 2536.9 pers-km/h 119.1 pers-h/h
Congestion Coefficient		3.40	
Demand Flows (Total) Arrival Flows (Total) Percent Heavy Vehicles (Demand) Percent Heavy Vehicles (Arrivals) Degree of Saturation Practical Spare Capacity Effective Intersection Capacity	veh/h veh/h % % veh/h	2636 2636 3.4 3.4 1.176 -27.7 2240	3163 pers/h
Control Delay (Total) Control Delay (Average) Control Delay (Worst Lane by MC)	veh-h/h sec sec	67.33 92.0 175.3	80.79 pers-h/h 92.0 sec
Control Delay (Worst Movement by MC) Geometric Delay (Average) Stop-Line Delay (Average) Idling Time (Average) Intersection Level of Service (LOS)	sec sec sec sec	183.6 6.3 85.6 52.3 LOS F	183.6 sec
95% Back of Queue - Veh (Worst Lane)	veh	139.9	
95% Back of Queue - Dist (Worst Lane) Ave. Que Storage Ratio (Worst Lane)	m	1006.4 0.81	
Effective Stops (Total) Effective Stop Rate Proportion Queued Performance Index	veh/h	6005 2.28 0.95 456.2	7206 pers/h 2.28 0.95 456.2
	<b>•</b> "	4404 54	
Cost (Total) Fuel Consumption (Total) Carbon Dioxide (Total) Hydrocarbons (Total) Carbon Monoxide (Total) NOx (Total)	\$/h L/h kg/h kg/h kg/h kg/h	4421.54 367.3 869.0 0.094 1.02 1.394	4421.54 \$/h

Site Level of Service (LOS) Method: Delay (RTA NSW). Site LOS Method is specified in the Parameter Settings dialog (Options tab).

Intersection LOS value for Vehicles is based on average delay for all vehicle movements.

Roundabout Capacity Model: SIDRA Standard.

Delay Model: SIDRA Standard (Control Delay: Geometric Delay is included).

Queue Model: SIDRA queue estimation methods are used for Back of Queue and Queue at Start of Gap.

Arrival Flows used in performance calculations are adjusted to include any Initial Queued Demand effects.

In Network analysis, Arrival Flows will be reduced if Upstream Capacity Constraint exists.

Gap-Acceptance Capacity Formula: SIDRA Standard (Akçelik M3D).

Site Model Variability Index (Average value of largest changes in Lane Degrees of Saturation from the third to the last Main (Timing-Capacity) Iterations): 3.6 %

Number of Iterations: 9 (Maximum: 10)

Largest change in Lane Degrees of Saturation for the last three Flow-Capacity Iterations: 2.4% 1.6% 0.9%

Intersection Performance - Annual Values								
Performance Measure	Vehicles:	All MCs	Persons					
Demand Flows (Total)	veh/y	1,265,179	1,518,215 pers/y					
Delay (Total)	veh-h/y	32,317	38,780 pers-h/y					

Effective Stops (Total)	veh/y	2,882,204	3,458,646 pers/y	
Travel Distance (Total)	veh-km/y	1,014,766	1,217,720 pers-km/y	
Travel Time (Total)	veh-h/y	47,635	57,162 pers-h/y	
Cost (Total) Fuel Consumption (Total) Carbon Dioxide (Total) Hydrocarbons (Total) Carbon Monoxide (Total) NOx (Total)	\$/y L/y kg/y kg/y kg/y kg/y	2,122,340 176,327 417,118 45 487 669	2,122,340 \$/y	

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Project: J:\YEAR 2022 JOBS\220895-00 Block 9 Section 132 Casey Apartment + Sky Terrace\G - Design Calculations\CIVIL\Casey Apartments.sip9

# V Site: 101E [PM Base 2022 Weekday Peak 5:15pm - 6:15pm (Site Folder: Overall Avenue and Horse Park Drive)]

Output produced by SIDRA INTERSECTION Version: 9.1.1.200

New Site Site Category: (None) Roundabout

Intersection Performance - Hourly Va	alues		
Performance Measure	Vehicles:	All MCs	Persons
Travel Speed (Average) Travel Distance (Total) Travel Time (Total) Desired Speed Speed Efficiency Travel Time Index Congestion Coefficient	km/h veh-km/h veh-h/h km/h	51.8 1694.6 32.7 73.2 0.71 6.74 1.41	51.8 km/h 2033.5 pers-km/h 39.3 pers-h/h
Demand Flows (Total) Arrival Flows (Total) Percent Heavy Vehicles (Demand) Percent Heavy Vehicles (Arrivals) Degree of Saturation Practical Spare Capacity Effective Intersection Capacity	veh/h veh/h % % veh/h	2124 2124 1.2 1.2 0.879 -3.3 2417	2549 pers/h
Control Delay (Total) Control Delay (Average) Control Delay (Worst Lane by MC) Control Delay (Worst Movement by MC) Geometric Delay (Average) Stop-Line Delay (Average) Idling Time (Average) Intersection Level of Service (LOS)	veh-h/h sec sec sec sec sec sec sec	7.18 12.2 15.5 24.2 6.0 6.2 0.6 LOS A	8.62 pers-h/h 12.2 sec 24.2 sec
95% Back of Queue - Veh (Worst Lane) 95% Back of Queue - Dist (Worst Lane) Ave. Que Storage Ratio (Worst Lane) Effective Stops (Total) Effective Stop Rate Proportion Queued Performance Index	veh m veh/h	17.3 122.3 0.15 1668 0.79 0.86 102.0	2002 pers/h 0.79 0.86 102.0
Cost (Total) Fuel Consumption (Total) Carbon Dioxide (Total) Hydrocarbons (Total) Carbon Monoxide (Total) NOx (Total)	\$/h L/h kg/h kg/h kg/h kg/h	1649.38 191.8 452.1 0.048 0.71 0.413	1649.38 \$/h

Site Level of Service (LOS) Method: Delay (RTA NSW). Site LOS Method is specified in the Parameter Settings dialog (Options tab).

Intersection LOS value for Vehicles is based on average delay for all vehicle movements.

Roundabout Capacity Model: SIDRA Standard.

Delay Model: SIDRA Standard (Control Delay: Geometric Delay is included).

Queue Model: SIDRA queue estimation methods are used for Back of Queue and Queue at Start of Gap.

Arrival Flows used in performance calculations are adjusted to include any Initial Queued Demand effects.

In Network analysis, Arrival Flows will be reduced if Upstream Capacity Constraint exists.

Gap-Acceptance Capacity Formula: SIDRA Standard (Akçelik M3D).

Site Model Variability Index (Average value of largest changes in Lane Degrees of Saturation from the third to the last Main (Timing-Capacity) Iterations): 5.4 %

Number of Iterations: 8 (Maximum: 10)

Largest change in Lane Degrees of Saturation for the last three Flow-Capacity Iterations: 2.9% 1.6% 0.8%

Intersection Performance - Annual Values										
Performance Measure	Vehicles:	All MCs	Persons							
Demand Flows (Total)	veh/y	1,019,621	1,223,545 pers/y							
Delay (Total)	veh-h/y	3,447	4,136 pers-h/y							

Effective Stops (Total)	veh/y	800,831	960,998 pers/y	
Travel Distance (Total)	veh-km/y	813,384	976,061 pers-km/y	
Travel Time (Total)	veh-h/y	15,716	18,859 pers-h/y	
Cost (Total) Fuel Consumption (Total) Carbon Dioxide (Total) Hydrocarbons (Total) Carbon Monoxide (Total) NOx (Total)	\$/y L/y kg/y kg/y kg/y kg/y	791,702 92,062 216,987 23 340 198	791,702 \$/y	

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### V Site: 101E [PM Development 2023 Weekday Peak 5:15pm - 6:15pm (Site Folder: Overall Avenue and Horse Park Drive)]

Output produced by SIDRA INTERSECTION Version: 9.1.1.200

New Site Site Category: (None) Roundabout

Intersection Performance - Hourly Values											
Performance Measure	Vehicles:	All MCs	Persons								
Travel Speed (Average) Travel Distance (Total) Travel Time (Total) Desired Speed Speed Efficiency Travel Time Index Congestion Coefficient	km/h veh-km/h veh-h/h km/h	49.6 1749.3 35.3 73.1 0.68 6.42 1.48	49.6 km/h 2099.2 pers-km/h 42.3 pers-h/h								
Demand Flows (Total) Arrival Flows (Total) Percent Heavy Vehicles (Demand) Percent Heavy Vehicles (Arrivals) Degree of Saturation Practical Spare Capacity Effective Intersection Capacity	veh/h veh/h % % veh/h	2200 2200 1.5 1.5 0.924 -8.0 2382	2640 pers/h								
Control Delay (Total) Control Delay (Average) Control Delay (Worst Lane by MC) Control Delay (Worst Movement by MC) Geometric Delay (Average) Stop-Line Delay (Average) Idling Time (Average) Intersection Level of Service (LOS)	veh-h/h sec sec sec sec sec sec	8.82 14.4 19.7 28.9 6.0 8.4 1.8 LOS A	10.58 pers-h/h 14.4 sec 28.9 sec								
95% Back of Queue - Veh (Worst Lane) 95% Back of Queue - Dist (Worst Lane) Ave. Que Storage Ratio (Worst Lane) Effective Stops (Total) Effective Stop Rate Proportion Queued Performance Index	veh m veh/h	22.9 162.2 0.18 1938 0.88 0.88 121.8	2325 pers/h 0.88 0.88 121.8								
Cost (Total) Fuel Consumption (Total) Carbon Dioxide (Total) Hydrocarbons (Total) Carbon Monoxide (Total) NOx (Total)	\$/h L/h kg/h kg/h kg/h kg/h	1767.16 202.8 478.3 0.050 0.73 0.480	1767.16 \$/h								

Site Level of Service (LOS) Method: Delay (RTA NSW). Site LOS Method is specified in the Parameter Settings dialog (Options tab).

Intersection LOS value for Vehicles is based on average delay for all vehicle movements.

Roundabout Capacity Model: SIDRA Standard.

Delay Model: SIDRA Standard (Control Delay: Geometric Delay is included).

Queue Model: SIDRA queue estimation methods are used for Back of Queue and Queue at Start of Gap.

Arrival Flows used in performance calculations are adjusted to include any Initial Queued Demand effects.

In Network analysis, Arrival Flows will be reduced if Upstream Capacity Constraint exists.

Gap-Acceptance Capacity Formula: SIDRA Standard (Akçelik M3D).

Site Model Variability Index (Average value of largest changes in Lane Degrees of Saturation from the third to the last Main (Timing-Capacity) Iterations): 5.6 %

Number of Iterations: 8 (Maximum: 10)

Largest change in Lane Degrees of Saturation for the last three Flow-Capacity Iterations: 2.6% 1.3% 0.7%

Intersection Performance - Annual Values											
Performance Measure	Vehicles:	All MCs	Persons								
Demand Flows (Total)	veh/y	1,056,000	1,267,200 pers/y								
Delay (Total)	veh-h/y	4,231	5,078 pers-h/y								

Effective Stops (Total)	veh/y	930,130	1,116,157 pers/y
Travel Distance (Total)	veh-km/y	839,676	1,007,612 pers-km/y
Travel Time (Total)	veh-h/y	16,937	20,325 pers-h/y
Cost (Total)	\$/y	848,237	848,237 \$/y
Fuel Consumption (Total)	L/y	97,366	
Carbon Dioxide (Total)	kg/y	229,608	
Hydrocarbons (Total)	kg/y	24	
Carbon Monoxide (Total)	kg/y	353	
NOx (Total)	kg/y	230	

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### V Site: 101E [PM Future 2033 Weekday Peak 5:15pm - 6:15pm (Site Folder: Overall Avenue and Horse Park Drive)]

Output produced by SIDRA INTERSECTION Version: 9.1.1.200

New Site Site Category: (None) Roundabout

Intersection Performance - Hourly Va	alues		
Performance Measure	Vehicles:	All MCs	Persons
Travel Speed (Average) Travel Distance (Total) Travel Time (Total) Desired Speed Speed Efficiency Travel Time Index Congestion Coefficient	km/h veh-km/h veh-h/h km/h	20.8 2126.3 102.3 73.2 0.28 2.05 3.52	20.8 km/h 2551.6 pers-km/h 122.7 pers-h/h
Demand Flows (Total) Arrival Flows (Total) Percent Heavy Vehicles (Demand) Percent Heavy Vehicles (Arrivals) Degree of Saturation Practical Spare Capacity Effective Intersection Capacity	veh/h veh/h % % veh/h	2673 2673 1.4 1.4 1.196 -29.0 2234	3207 pers/h
Control Delay (Total) Control Delay (Average) Control Delay (Worst Lane by MC) Control Delay (Worst Movement by MC) Geometric Delay (Average) Stop-Line Delay (Average) Idling Time (Average) Intersection Level of Service (LOS)	veh-h/h sec sec sec sec sec sec sec	70.30 94.7 193.0 203.2 6.0 88.7 54.6 LOS F	84.36 pers-h/h 94.7 sec 203.2 sec
95% Back of Queue - Veh (Worst Lane) 95% Back of Queue - Dist (Worst Lane) Ave. Que Storage Ratio (Worst Lane) Effective Stops (Total) Effective Stop Rate Proportion Queued Performance Index	veh m veh/h	143.3 1014.8 0.91 6139 2.30 0.94 518.4	7367 pers/h 2.30 0.94 518.4
Cost (Total) Fuel Consumption (Total) Carbon Dioxide (Total) Hydrocarbons (Total) Carbon Monoxide (Total) NOx (Total)	\$/h L/h kg/h kg/h kg/h kg/h	4465.46 345.0 813.1 0.089 0.98 0.734	4465.46 \$/h

Site Level of Service (LOS) Method: Delay (RTA NSW). Site LOS Method is specified in the Parameter Settings dialog (Options tab).

Intersection LOS value for Vehicles is based on average delay for all vehicle movements.

Roundabout Capacity Model: SIDRA Standard.

Delay Model: SIDRA Standard (Control Delay: Geometric Delay is included).

Queue Model: SIDRA queue estimation methods are used for Back of Queue and Queue at Start of Gap.

Arrival Flows used in performance calculations are adjusted to include any Initial Queued Demand effects.

In Network analysis, Arrival Flows will be reduced if Upstream Capacity Constraint exists.

Gap-Acceptance Capacity Formula: SIDRA Standard (Akçelik M3D).

Site Model Variability Index (Average value of largest changes in Lane Degrees of Saturation from the third to the last Main (Timing-Capacity) Iterations): 4.3 %

Number of Iterations: 8 (Maximum: 10)

Largest change in Lane Degrees of Saturation for the last three Flow-Capacity Iterations: 2.1% 1.1% 0.6%

Intersection Performance - Annual Values											
Performance Measure	Vehicles:	All MCs	Persons								
Demand Flows (Total)	veh/y	1,282,863	1,539,436 pers/y								
Delay (Total)	veh-h/y	33,744	40,492 pers-h/y								

Effective Stops (Total)	veh/y	2,946,848	3,536,218 pers/y	
Travel Distance (Total)	veh-km/y	1,020,645	1,224,774 pers-km/y	
Travel Time (Total)	veh-h/y	49,081	58,898 pers-h/y	
Cost (Total) Fuel Consumption (Total) Carbon Dioxide (Total) Hydrocarbons (Total) Carbon Monoxide (Total) NOx (Total)	\$/y L/y kg/y kg/y kg/y kg/y	2,143,419 165,618 390,281 43 471 353	2,143,419 \$/y	

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Project: J:\YEAR 2022 JOBS\220895-00 Block 9 Section 132 Casey Apartment + Sky Terrace\G - Design Calculations\CIVIL\Casey Apartments.sip9

# V Site: 101E [AM Base 2022 Weekday Peak 8:00am - 9:00am (Site Folder: Overall Avenue and Horse Park Drive)]

Output produced by SIDRA INTERSECTION Version: 9.1.1.200

New Site Site Category: (None) Roundabout

Vehic	cle Mo	ovement	t Perfo	rma	nce										
Mov ID	Turn	Mov Class		lows HV ]		rival lows HV ] %	Deg. Satn v/c	Aver. Delay sec	Level of Service	95% Ba Que [ Veh. veh		Prop. Que	Eff. Stop Rate	Aver. No. of Cycles	Aver. Speed km/h
South	: Hors	e Park D	rive												
1	L2	All MCs	69	1.5	69	1.5	0.563	8.9	LOS A	4.7	33.9	0.74	0.69	0.80	37.8
2	T1	All MCs	422	4.7	422	4.7	0.563	9.5	LOS A	4.7	33.9	0.74	0.69	0.80	60.6
3	R2	All MCs	32	0.0	32	0.0	0.563	14.4	LOS A	4.7	33.9	0.74	0.69	0.80	41.3
3u	U	All MCs	1	0.0	1	0.0	0.563	17.0	LOS B	4.7	33.9	0.74	0.69	0.80	59.7
Appro	ach		524	4.0	524	4.0	0.563	9.8	LOS A	4.7	33.9	0.74	0.69	0.80	56.6
East:	Newlo	p Street													
4	L2	All MCs	72	2.9	72	2.9	0.247	16.7	LOS B	1.9	13.5	1.00	0.86	1.00	36.0
5	T1	All MCs	104	1.0	104	1.0	0.281	13.6	LOS A	2.6	18.3	1.00	0.85	1.00	30.2
6	R2	All MCs	12	0.0	12	0.0	0.281	18.5	LOS B	2.6	18.3	1.00	0.85	1.00	38.3
6u	U	All MCs	1	0.0	1	0.0	0.281	22.2	LOS B	2.6	18.3	1.00	0.85	1.00	32.8
Appro	ach		188	1.7	188	1.7	0.281	15.1	LOS B	2.6	18.3	1.00	0.86	1.00	33.8
North	: Hors	e Park Dr	ive												
7	L2	All MCs	6	0.0	6	0.0	0.869	12.5	LOS A	16.6	119.1	0.98	0.81	1.24	51.8
8	T1	All MCs	743	3.3	743	3.3	0.869	13.1	LOS A	16.6	119.1	0.98	0.81	1.24	57.4
9	R2	All MCs	268	3.1	268	3.1	0.869	18.2	LOS B	16.6	119.1	0.98	0.81	1.24	48.3
Appro	ach		1018	3.2	1018	3.2	0.869	14.4	LOS A	16.6	119.1	0.98	0.81	1.24	55.4
West:	Overa	all Avenue	Э												
10		All MCs	126	4.2	126	4.2	0.355	4.8	LOS A	2.2	16.1	0.64	0.64	0.64	42.0
11	T1	All MCs	45	7.0	45	7.0	0.355	4.7	LOS A	2.2	16.1	0.64	0.64	0.64	36.4
12	R2	All MCs	185	1.1	185	1.1	0.355	9.4	LOS A	2.2	16.1	0.64	0.64	0.64	41.0
12u	U	All MCs	1	0.0	1		0.355	11.3	LOS A	2.2	16.1	0.64	0.64	0.64	16.1
Appro	ach		358	2.9	358	2.9	0.355	7.2	LOS A	2.2	16.1	0.64	0.64	0.64	41.0
All Ve	hicles		2088	3.2	2088	3.2	0.869	12.1	LOS A	16.6	119.1	0.86	0.76	1.01	51.8

Site Level of Service (LOS) Method: Delay (RTA NSW). Site LOS Method is specified in the Parameter Settings dialog (Options tab).

Vehicle movement LOS values are based on average delay per movement.

Intersection and Approach LOS values are based on average delay for all vehicle movements.

Roundabout Capacity Model: SIDRA Standard.

Delay Model: SIDRA Standard (Control Delay: Geometric Delay is included).

Queue Model: SIDRA queue estimation methods are used for Back of Queue and Queue at Start of Gap.

Gap-Acceptance Capacity Formula: SIDRA Standard (Akçelik M3D).

HV (%) values are calculated for All Movement Classes of All Heavy Vehicle Model Designation.

Arrival Flows used in performance calculations are adjusted to include any Initial Queued Demand and Upstream Capacity Constraint effects.

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#### V Site: 101E [AM Development 2023 Weekday Peak 8:00am -9:00am (Site Folder: Overall Avenue and Horse Park Drive)]

Output produced by SIDRA INTERSECTION Version: 9.1.1.200

New Site Site Category: (None) Roundabout

Vehic	cle Mo	ovement	Perfo	rma	nce										
Mov ID	Turn	Mov Class		lows HV ]		rival lows HV] %	Deg. Satn v/c	Aver. Delay sec	Level of Service	95% Ba Que [ Veh. veh		Prop. Que	Eff. Stop Rate	Aver. No. of Cycles	Aver. Speed km/h
South	: Hors	e Park D	rive												
1	L2	All MCs	83	2.5	83	2.5	0.604	9.7	LOS A	5.5	39.7	0.78	0.73	0.89	37.5
2	T1	All MCs	432	4.9	432	4.9	0.604	10.3	LOS A	5.5	39.7	0.78	0.73	0.89	60.2
3	R2	All MCs	33	0.0	33	0.0	0.604	15.2	LOS B	5.5	39.7	0.78	0.73	0.89	41.0
3u	U	All MCs	2	0.0	2	0.0	0.604	17.7	LOS B	5.5	39.7	0.78	0.73	0.89	59.3
Appro	ach		549	4.2	549	4.2	0.604	10.5	LOS A	5.5	39.7	0.78	0.73	0.89	55.8
East:	Newlo	p Street													
4	L2	All MCs	75	4.2	75	4.2	0.286	18.5	LOS B	2.2	15.8	1.00	0.88	1.00	35.1
5	T1	All MCs	107	2.0	107	2.0	0.335	15.0	LOS B	3.1	22.2	1.00	0.87	1.00	28.9
6	R2	All MCs	13	0.0	13	0.0	0.335	19.9	LOS B	3.1	22.2	1.00	0.87	1.00	37.4
6u	U	All MCs	7	0.0	7	0.0	0.335	23.6	LOS B	3.1	22.2	1.00	0.87	1.00	31.5
Appro	ach		202	2.6	202	2.6	0.335	16.9	LOS B	3.1	22.2	1.00	0.88	1.00	32.6
North	: Horse	e Park Dr	ive												
7	L2	All MCs	7	0.0	7	0.0	0.912	15.9	LOS B	21.4	154.2	1.00	0.97	1.45	48.5
8	T1	All MCs	759	3.3	759	3.3	0.912	16.5	LOS B	21.4	154.2	1.00	0.97	1.45	54.4
9	R2	All MCs	279	3.4	279	3.4	0.912	21.6	LOS B	21.4	154.2	1.00	0.97	1.45	45.1
Appro	ach		1045	3.3	1045	3.3	0.912	17.8	LOS B	21.4	154.2	1.00	0.97	1.45	52.3
West:	Overa	all Avenue	9												
10	L2	All MCs	133	4.8	133	4.8	0.383	5.0	LOS A	2.5	18.0	0.67	0.65	0.67	41.9
11	T1	All MCs	47	8.9	47	8.9	0.383	4.9	LOS A	2.5	18.0	0.67	0.65	0.67	36.2
12	R2	All MCs	195	1.6	195	1.6	0.383	9.6	LOS A	2.5	18.0	0.67	0.65	0.67	40.9
12u	U	All MCs	2	0.0	2	0.0	0.383	11.5	LOS A	2.5	18.0	0.67	0.65	0.67	16.1
Appro	ach		377	3.6	377	3.6	0.383	7.4	LOS A	2.5	18.0	0.67	0.65	0.67	40.8
All Ve	hicles		2174	3.5	2174	3.5	0.912	14.1	LOS A	21.4	154.2	0.89	0.85	1.13	49.9

Site Level of Service (LOS) Method: Delay (RTA NSW). Site LOS Method is specified in the Parameter Settings dialog (Options tab).

Vehicle movement LOS values are based on average delay per movement.

Intersection and Approach LOS values are based on average delay for all vehicle movements.

Roundabout Capacity Model: SIDRA Standard.

Delay Model: SIDRA Standard (Control Delay: Geometric Delay is included).

Queue Model: SIDRA queue estimation methods are used for Back of Queue and Queue at Start of Gap.

Gap-Acceptance Capacity Formula: SIDRA Standard (Akçelik M3D).

HV (%) values are calculated for All Movement Classes of All Heavy Vehicle Model Designation.

Arrival Flows used in performance calculations are adjusted to include any Initial Queued Demand and Upstream Capacity Constraint effects.

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### V Site: 101E [AM Future 2033 Weekday Peak 8:00am - 9:00am (Site Folder: Overall Avenue and Horse Park Drive)]

Output produced by SIDRA INTERSECTION Version: 9.1.1.200

New Site Site Category: (None) Roundabout

Vehic	Vehicle Movement Performance														
Mov ID	Turn	Mov Class		lows HV ]		rival lows HV ] %	Deg. Satn v/c	Aver. Delay sec	Level of Service	95% B Que [ Veh. veh		Prop. Que	Eff. Stop Rate	Aver. No. of Cycles	Aver. Speed km/h
South	: Hors	e Park D	rive												
1	L2	All MCs	98	2.2	98	2.2	0.748	12.8	LOS A	9.3	67.2	0.91	0.85	1.20	35.7
2	T1	All MCs	525	4.8	525	4.8	0.748	13.5	LOS A	9.3	67.2	0.91	0.85	1.20	57.6
3	R2	All MCs	40	0.0	40	0.0	0.748	18.3	LOS B	9.3	67.2	0.91	0.85	1.20	39.2
3u	U	All MCs	2	0.0	2	0.0	0.748	20.9	LOS B	9.3	67.2	0.91	0.85	1.20	56.6
Appro	ach		665	4.1	665	4.1	0.748	13.7	LOS A	9.3	67.2	0.91	0.85	1.20	53.4
East:	Newlo	p Street													
4	L2	All MCs	91	3.5	91	3.5	0.403	26.6	LOS B	3.2	23.3	1.00	0.97	1.12	31.2
5	T1	All MCs	131	1.6	131	1.6	0.469	24.0	LOS B	4.8	34.0	1.00	0.99	1.18	23.5
6	R2	All MCs	15	0.0	15	0.0	0.469	28.8	LOS C	4.8	34.0	1.00	0.99	1.18	33.1
6u	U	All MCs	8	0.0	8	0.0	0.469	32.5	LOS C	4.8	34.0	1.00	0.99	1.18	26.2
Appro	ach		244	2.2	244	2.2	0.469	25.5	LOS B	4.8	34.0	1.00	0.98	1.16	27.9
North	Horse	e Park Dr	ive												
7	L2	All MCs	8	0.0	8	0.0	1.176	173.3	LOS F	139.9	1006.4	1.00	3.82	8.16	12.2
8	T1	All MCs	924	3.3	924	3.3	1.176	173.9	LOS F	139.9	1006.4	1.00	3.82	8.16	16.0
9	R2	All MCs	339	3.1	339	3.1	1.176	179.1	LOS F	139.9	1006.4	1.00	3.82	8.16	11.1
Appro	ach		1272	3.2	1272	3.2	1.176	175.3	LOS F	139.9	1006.4	1.00	3.82	8.16	14.7
West:	Overa	all Avenue	e												
10	L2	All MCs	160	4.6	160	4.6	0.523	6.7	LOS A	4.3	31.1	0.83	0.76	0.91	40.9
11	T1	All MCs	57	7.4	57	7.4	0.523	6.5	LOS A	4.3	31.1	0.83	0.76	0.91	34.8
12	R2	All MCs	236	1.3	236	1.3	0.523	11.3	LOS A	4.3	31.1	0.83	0.76	0.91	39.9
12u	U	All MCs	2	0.0	2	0.0	0.523	13.1	LOS A	4.3	31.1	0.83	0.76	0.91	15.7
Appro	ach		455	3.2	455	3.2	0.523	9.1	LOS A	4.3	31.1	0.83	0.76	0.91	39.8
All Ve	hicles		2636	3.4	2636	3.4	1.176	92.0	LOS F	139.9	1006.4	0.95	2.28	4.50	21.3

Site Level of Service (LOS) Method: Delay (RTA NSW). Site LOS Method is specified in the Parameter Settings dialog (Options tab).

Vehicle movement LOS values are based on average delay per movement.

Intersection and Approach LOS values are based on average delay for all vehicle movements.

Roundabout Capacity Model: SIDRA Standard.

Delay Model: SIDRA Standard (Control Delay: Geometric Delay is included).

Queue Model: SIDRA queue estimation methods are used for Back of Queue and Queue at Start of Gap.

Gap-Acceptance Capacity Formula: SIDRA Standard (Akçelik M3D).

HV (%) values are calculated for All Movement Classes of All Heavy Vehicle Model Designation.

Arrival Flows used in performance calculations are adjusted to include any Initial Queued Demand and Upstream Capacity Constraint effects.

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# V Site: 101E [PM Base 2022 Weekday Peak 5:15pm - 6:15pm (Site Folder: Overall Avenue and Horse Park Drive)]

Output produced by SIDRA INTERSECTION Version: 9.1.1.200

New Site Site Category: (None) Roundabout

Vehi	cle Mo	ovement	t Perfo	rma	nce	_									
Mov ID	Turn	Mov Class	FI			rival ows HV ] %	Deg. Satn v/c	Aver. Delay sec	Level of Service	95% B Que [ Veh. veh		Prop. Que	Eff. Stop Rate	Aver. No. of Cycles	Aver. Speed km/h
South: Horse Park Drive															
1	L2	All MCs	142	0.7	142	0.7	0.879	14.7	LOS B	17.3	122.3	1.00	0.89	1.39	34.7
2	T1	All MCs	748	1.3	748	1.3	0.879	15.2	LOS B	17.3	122.3	1.00	0.89	1.39	56.7
3	R2	All MCs	71	1.5	71	1.5	0.879	20.3	LOS B	17.3	122.3	1.00	0.89	1.39	48.3
Appro	bach		961	1.2	961	1.2	0.879	15.5	LOS B	17.3	122.3	1.00	0.89	1.39	53.0
East:	Newlo	p Street													
4	L2	All MCs	52	0.0	52	0.0	0.074	7.2	LOS A	0.5	3.2	0.74	0.67	0.74	42.4
5	T1	All MCs	93	0.0	93	0.0	0.108	5.7	LOS A	0.8	5.4	0.76	0.62	0.76	37.1
6	R2	All MCs	8	0.0	8	0.0	0.108	10.7	LOS A	0.8	5.4	0.76	0.62	0.76	42.7
Appro	bach		153	0.0	153	0.0	0.108	6.5	LOS A	0.8	5.4	0.75	0.64	0.75	40.0
North	: Horse	e Park Dr	ive												
7	L2	All MCs	15	0.0	15	0.0	0.551	7.0	LOS A	4.4	31.5	0.61	0.61	0.61	56.0
8	T1	All MCs	426	1.2	426	1.2	0.551	7.5	LOS A	4.4	31.5	0.61	0.61	0.61	61.6
9	R2	All MCs	206	2.0	206	2.0	0.551	12.7	LOS A	4.4	31.5	0.61	0.61	0.61	52.4
Appro	bach		647	1.5	647	1.5	0.551	9.2	LOS A	4.4	31.5	0.61	0.61	0.61	59.0
West	: Overa	all Avenue	е												
10	L2	All MCs	191	2.2	191	2.2	0.567	10.1	LOS A	5.4	38.2	0.98	0.88	1.17	39.5
11	T1	All MCs	89	1.2	89	1.2	0.567	9.8	LOS A	5.4	38.2	0.98	0.88	1.17	32.6
12	R2	All MCs	83	0.0	83	0.0	0.567	14.7	LOS B	5.4	38.2	0.98	0.88	1.17	38.4
Appro	bach		363	1.4	363	1.4	0.567	11.1	LOS A	5.4	38.2	0.98	0.88	1.17	38.1
All Ve	ehicles		2124	1.2	2124	1.2	0.879	12.2	LOS A	17.3	122.3	0.86	0.79	1.07	51.8

Site Level of Service (LOS) Method: Delay (RTA NSW). Site LOS Method is specified in the Parameter Settings dialog (Options tab).

Vehicle movement LOS values are based on average delay per movement.

Intersection and Approach LOS values are based on average delay for all vehicle movements.

Roundabout Capacity Model: SIDRA Standard.

Delay Model: SIDRA Standard (Control Delay: Geometric Delay is included).

Queue Model: SIDRA queue estimation methods are used for Back of Queue and Queue at Start of Gap.

Gap-Acceptance Capacity Formula: SIDRA Standard (Akçelik M3D).

HV (%) values are calculated for All Movement Classes of All Heavy Vehicle Model Designation.

Arrival Flows used in performance calculations are adjusted to include any Initial Queued Demand and Upstream Capacity Constraint effects.

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### Site: 101E [PM Development 2023 Weekday Peak 5:15pm - 6:15pm (Site Folder: Overall Avenue and Horse Park Drive)]

Output produced by SIDRA INTERSECTION Version: 9.1.1.200

New Site Site Category: (None) Roundabout

Vehi	cle Mo	ovement	t Perfo	rma	nce										
Mov ID	Turn	Mov Class		lows HV ]		rival lows HV ] %	Deg. Satn v/c	Aver. Delay sec	Level of Service	95% B Que [ Veh. veh		Prop. Que	Eff. Stop Rate	Aver. No. of Cycles	Aver. Speed km/h
South: Horse Park Drive															
1	L2	All MCs	157	1.3	157	1.3	0.924	18.9	LOS B	22.9	162.2	1.00	1.07	1.63	32.4
2	T1	All MCs	765	1.4	765	1.4	0.924	19.4	LOS B	22.9	162.2	1.00	1.07	1.63	53.1
3	R2	All MCs	74	2.9	74	2.9	0.924	24.6	LOS B	22.9	162.2	1.00	1.07	1.63	44.4
Appro	bach		996	1.5	996	1.5	0.924	19.7	LOS B	22.9	162.2	1.00	1.07	1.63	49.3
East:	Newlo	p Street													
4	L2	All MCs	53	0.0	53	0.0	0.079	7.4	LOS A	0.5	3.5	0.76	0.68	0.76	42.2
5	T1	All MCs	95	0.0	95	0.0	0.115	6.0	LOS A	0.8	5.8	0.78	0.63	0.78	36.9
6	R2	All MCs	9	0.0	9	0.0	0.115	11.0	LOS A	0.8	5.8	0.78	0.63	0.78	42.5
Appro	bach		157	0.0	157	0.0	0.115	6.8	LOS A	0.8	5.8	0.77	0.65	0.77	39.8
North	: Horse	e Park Dr	ive												
7	L2	All MCs	16	0.0	16	0.0	0.576	7.2	LOS A	4.8	34.0	0.64	0.62	0.64	55.8
8	T1	All MCs	436	1.4	436	1.4	0.576	7.7	LOS A	4.8	34.0	0.64	0.62	0.64	61.3
9	R2	All MCs	216	2.4	216	2.4	0.576	12.9	LOS A	4.8	34.0	0.64	0.62	0.64	52.2
Appro	bach		667	1.7	667	1.7	0.576	9.4	LOS A	4.8	34.0	0.64	0.62	0.64	58.8
West	Overa	all Avenue	Э												
10	L2	All MCs	198	2.7	198	2.7	0.616	11.7	LOS A	6.2	44.4	1.00	0.93	1.25	38.5
11	T1	All MCs	93	2.3	93	2.3	0.616	11.4	LOS A	6.2	44.4	1.00	0.93	1.25	31.1
12	R2	All MCs	89	0.0	89	0.0	0.616	16.2	LOS B	6.2	44.4	1.00	0.93	1.25	37.3
Appro	bach		380	1.9	380	1.9	0.616	12.7	LOS A	6.2	44.4	1.00	0.93	1.25	37.0
All Ve	hicles		2200	1.5	2200	1.5	0.924	14.4	LOS A	22.9	162.2	0.88	0.88	1.21	49.6

Site Level of Service (LOS) Method: Delay (RTA NSW). Site LOS Method is specified in the Parameter Settings dialog (Options tab).

Vehicle movement LOS values are based on average delay per movement.

Intersection and Approach LOS values are based on average delay for all vehicle movements.

Roundabout Capacity Model: SIDRA Standard.

Delay Model: SIDRA Standard (Control Delay: Geometric Delay is included).

Queue Model: SIDRA queue estimation methods are used for Back of Queue and Queue at Start of Gap.

Gap-Acceptance Capacity Formula: SIDRA Standard (Akçelik M3D).

HV (%) values are calculated for All Movement Classes of All Heavy Vehicle Model Designation.

Arrival Flows used in performance calculations are adjusted to include any Initial Queued Demand and Upstream Capacity Constraint effects.

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#### **W** Site: 101E [PM Future 2033 Weekday Peak 5:15pm - 6:15pm (Site Folder: Overall Avenue and Horse Park Drive)]

**Output produced by SIDRA INTERSECTION Version: 9.1.1.200** 

New Site Site Category: (None) Roundabout

Vehi	cle Mo	ovement	t Per <u>fo</u>	rma	nce										
Mov ID	Turn	Mov Class	FI			rival lows HV ] %	Deg. Satn v/c	Aver. Delay sec	Level of Service		ack Of eue Dist ] m	Prop. Que	Eff. Stop Rate	Aver. No. of Cycles	Aver. Speed km/h
South: Horse Park Drive															
1	L2	All MCs	188	1.1	188	1.1	1.196	192.2	LOS F	143.3	1014.8	1.00	4.05	9.18	8.8
2	T1	All MCs	932	1.4	932	1.4	1.196	192.7	LOS F	143.3	1014.8	1.00	4.05	9.18	14.8
3	R2	All MCs	89	2.4	89	2.4	1.196	197.9	LOS F	143.3	1014.8	1.00	4.05	9.18	10.4
Appro	bach		1209	1.4	1209	1.4	1.196	193.0	LOS F	143.3	1014.8	1.00	4.05	9.18	13.5
East:	Newlo	p Street													
4	L2	All MCs	64	0.0	64	0.0	0.123	9.4	LOS A	0.9	6.0	0.88	0.75	0.88	40.7
5	T1	All MCs	116	0.0	116	0.0	0.177	7.6	LOS A	1.4	10.1	0.92	0.71	0.92	35.8
6	R2	All MCs	11	0.0	11	0.0	0.177	12.6	LOS A	1.4	10.1	0.92	0.71	0.92	41.9
Appro	bach		191	0.0	191	0.0	0.177	8.5	LOS A	1.4	10.1	0.91	0.72	0.91	38.6
North	: Hors	e Park Dr	ive												
7	L2	All MCs	19	0.0	19	0.0	0.724	9.3	LOS A	8.7	61.8	0.81	0.71	0.92	54.3
8	T1	All MCs	532	1.4	532	1.4	0.724	9.8	LOS A	8.7	61.8	0.81	0.71	0.92	60.0
9	R2	All MCs	261	2.0	261	2.0	0.724	15.0	LOS B	8.7	61.8	0.81	0.71	0.92	50.7
Appro	bach		812	1.6	812	1.6	0.724	11.4	LOS A	8.7	61.8	0.81	0.71	0.92	57.4
West	Overa	all Avenue	Э												
10	L2	All MCs	240	2.2	240	2.2	0.774	17.9	LOS B	10.0	70.6	1.00	1.14	1.55	34.8
11	T1	All MCs	113	1.9	113	1.9	0.774	17.6	LOS B	10.0	70.6	1.00	1.14	1.55	26.4
12	R2	All MCs	108	0.0	108	0.0	0.774	22.4	LOS B	10.0	70.6	1.00	1.14	1.55	33.6
Appro	bach		461	1.6	461	1.6	0.774	18.9	LOS B	10.0	70.6	1.00	1.14	1.55	33.1
All Ve	hicles		2673	1.4	2673	1.4	1.196	94.7	LOS F	143.3	1014.8	0.94	2.30	4.77	20.8

Site Level of Service (LOS) Method: Delay (RTA NSW). Site LOS Method is specified in the Parameter Settings dialog (Options tab).

Vehicle movement LOS values are based on average delay per movement.

Intersection and Approach LOS values are based on average delay for all vehicle movements.

Roundabout Capacity Model: SIDRA Standard.

Delay Model: SIDRA Standard (Control Delay: Geometric Delay is included).

Queue Model: SIDRA queue estimation methods are used for Back of Queue and Queue at Start of Gap.

Gap-Acceptance Capacity Formula: SIDRA Standard (Akcelik M3D).

HV (%) values are calculated for All Movement Classes of All Heavy Vehicle Model Designation.

Arrival Flows used in performance calculations are adjusted to include any Initial Queued Demand and Upstream Capacity Constraint effects.

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