

ANALYTICAL AND EXPERIMENTAL DETERMINATION  
OF LOCALIZED STRUCTURE TO BE USED IN  
LABORATORY VIBRATION TESTING OF SHELL  
STRUCTURE-MOUNTED COMPONENTS, SATURN V

NOR 67-19

January 1967

PROGRESS REPORT COVERING  
THE PERIOD MAY 1966 TO NOVEMBER 1966

PROCEDURE FOR DESIGNING A LOCALIZED SHELL AND THE  
APPLICATION OF THE FINITE DIFFERENCE COMPUTER PROGRAM

Prepared For  
George C. Marshall Space Flight Center  
National Aeronautics and Space Administration  
Huntsville, Alabama

NASA CONTRACT NAS8-20025

FACILITY FORM 602	<u>23253</u>	_____
	(ACCESSION NUMBER)	(THRU)
	<u>155</u>	_____
(PAGES)	(CODE)	_____
<u>CR-83527</u>	<u>32</u>	_____
(NASA CR OR TMX OR AD NUMBER)	(CATEGORY)	

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## ABSTRACT

The progress report covers the work performed for the contract entitled "Analytical and Experimental Determination of Localized Structure to be Used in Laboratory Vibration Testing of Shell Structure-Mounted Components, SATURN V." The work was carried out during the period of May 1966 to November 1966 inclusive. In the report, the detailed procedure in designing a localized shell is described. Also presented is a computer program using the finite difference method which serves to guide the engineers in designing and predicting the vibration responses of the localized shell.

## FOREWORD

The progress report was prepared by Northrop Corporation, Norair Division, Hawthorne, California, under contract no. NAS8-20025, "Analytical and Experimental Determination of Localized Structure to be Used in Laboratory Vibration Testing of Shell Structure-Mounted Components, SATURN V."

The subject contract is administered under the direction of the Structures Branch, Propulsion and Vehicle Engineering Laboratory, George C. Marshall Space Flight Center of the National Aeronautics and Space Administration by Mr. J.H. Farrow and Mr. R. Jewell, principal and alternate technical representatives, respectively. Mr. L.D. Saint is the program monitor.

The program manager at Northrop Norair is Dr. Chintsun Hwang, M. T. M., Structures and Dynamics Research Branch. Dr's. W.S. Pi, N.M. Bhatia and Mr. J.R. Yamane participate in the project. Mr. P.E. Finwall is responsible for the experimental tasks of the program.

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SECTION I  
INTRODUCTION

During the first year of the contract on localized shell structure to be used in laboratory vibration testing, analytical techniques were used to guide the actual shell design which has been progressing on a trial basis. In the process, it was realized that once a technique has been established, a documented procedure is needed for the test engineers to design and test the localized structures. The present report serves as the document to guide the users in carrying out the design in a rational manner.

The report presents the analytical and experimental techniques in a mixed fashion. They are described in a logical sequence corresponding to the design process. The users are assumed to have available the first year progress report of the same contract which is dated May 1966.

The major supporting computer program for the design procedure is the finite difference program. The basic theory, program mechanization, the program listing and the input-output format are included in the report.



## SECTION II

### SHELL RESPONSE AND IMPEDANCE STUDY

In order to design a localized shell structure for laboratory vibration testing purposes, preliminary tests are conducted using the complete shell structure such as the Instrument Unit with proper supporting conditions. Applying a frequency sweep technique, the shell responses and the point impedance function are plotted. Typical response data for a scale model is plotted in Figure 1. Typical impedance data are shown in Figure 2. Similar plots may be obtained for a full scale structure. Note that, in general, each peak response of Figure 1 corresponds approximately to a minimum impedance point of Figure 2. The above response and impedance plots are for a specific driving point. In the present case, the driving point is at the center of the localized shell panel. Considering the nodal line distribution, as well as the possibility of certain non-symmetrical patterns of the natural modes, it is advisable to either move the driving point, or to plot the cross-impedance of the shell structure during the test.

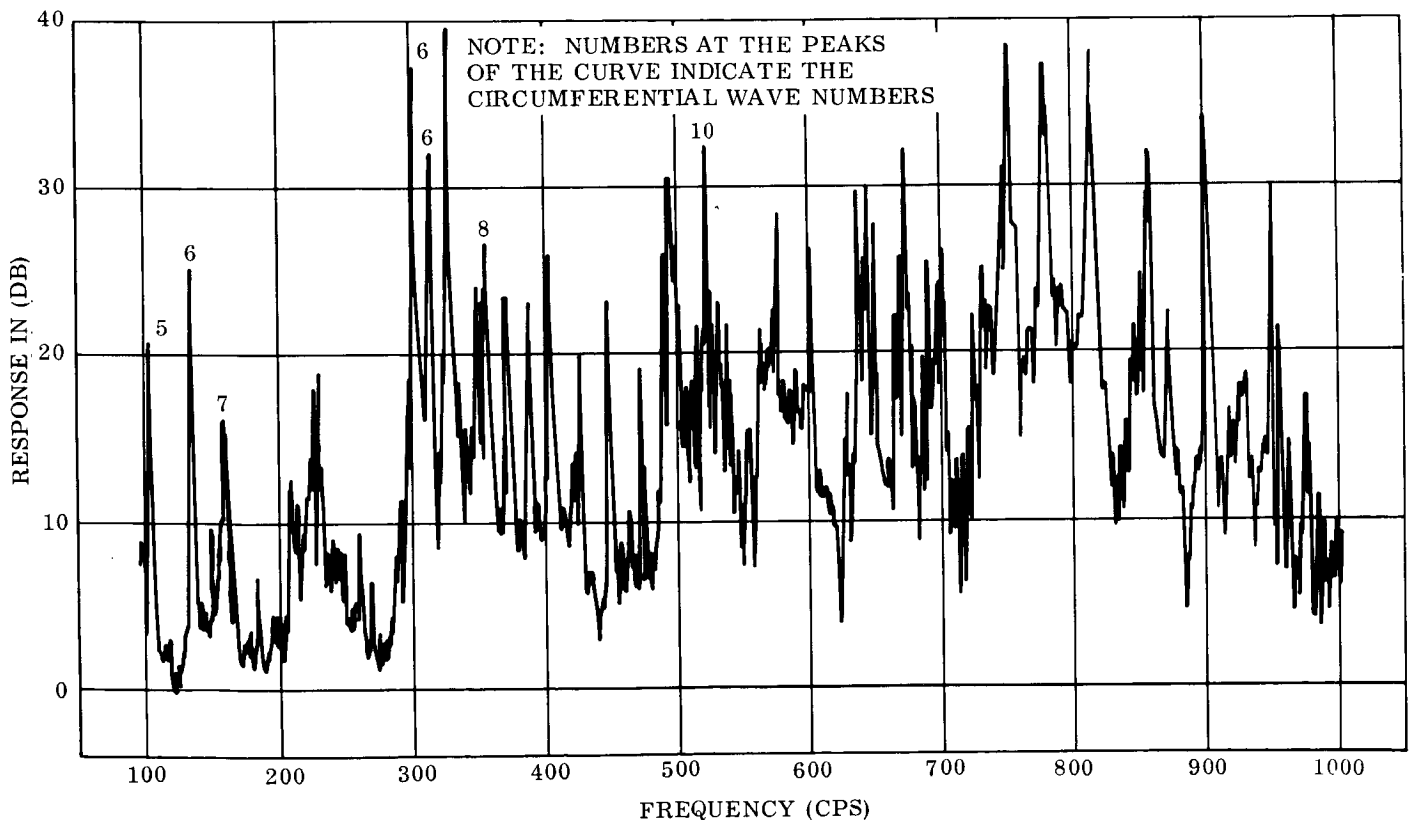


FIGURE 1. MICROPHONE RESPONSE VS FREQUENCY FOR POINT AT CENTER OF INSTRUMENT UNIT 5° FROM ELECTROMAGNETIC DRIVER

The circumferential harmonic number is obtained by a microphone type pickup traversing along the circumference of the shell structure. For convenience in correlating the test data with the analytical data, it is preferable to test the shell with no components attached.

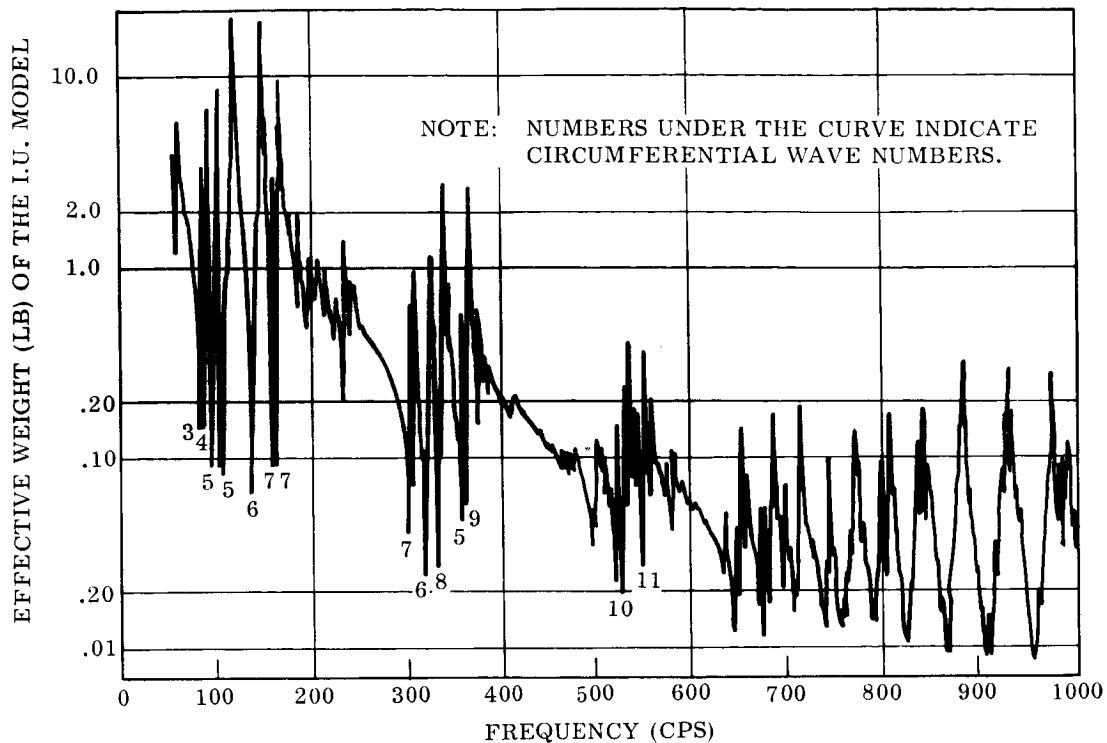


FIGURE 2. TYPICAL DRIVING POINT IMPEDANCES FOR THE I.U. SCALE MODEL

The purpose of the procedure is to design a segmented shell with equivalent dynamic characteristics as the complete shell. Experience has shown that if the segmentation procedure generates a configuration with satisfactory dynamic characteristics for the unloaded shell, then the dynamic response of the same segmented shell with component attachment will be satisfactory. In other words, the segmented shell will simulate the complete shell structure when the components are attached.

Referring to Figure 1, the prominent responses of the complete shell correspond to its natural frequency modes. In order to design a dynamically similar shell segment, the criterion is to retain as many natural frequency modes as possible in the segmented shell. For this purpose, it is necessary to investigate the detailed deformation pattern, the internal stresses, etc. of each major mode. The detailed shell response information collected in this manner is used for segmentation design. In general, the lower the natural frequency, the longer is the characteristic wave length of the deformation. As a result, the corresponding mode response of a specific localized panel is more dependent on the remaining portion of the shell structure. For high frequency modes, the deformation wave lengths are relatively short. The local response of a panel is not overly influenced by the remaining structure. As a result, in designing a localized structure, it takes less elaboration to retain the high frequency modes as compared to the low frequency modes. It is thus advisable to pay more emphasis in retaining the lower frequency modes. For a shell structure of the size of the Instrument Unit, the natural frequency modes under 100 cps are considered most significant and justify special attention.

SECTION III  
GENERAL STIFFENED SHELL PROGRAM

In order to investigate the shell detailed deformation pattern and the internal stresses, the general stiffened shell computer program may be used. The program can handle an arbitrarily shaped shell of revolution with a number of ring stiffeners. The modal data corresponding to a circumferential harmonic number is printed out in a table together with the natural frequency. The detailed procedure of using the computer program is given in the contract yearly progress report, NOR 66-201, Vo. II, dated May 1966. The discussion here is limited to the application of the computer program for localized shell design purposes.

The computer output consists essentially of a table of eight variables along the meridian of the ring-stiffener shell. The eight variables are:

$$w_n, u_n, v_n, \beta_n, Q_n, N_{\varphi n}, N_n, M_{\varphi n}$$

Among the eight variables,  $u$ ,  $v$ ,  $w$  are the displacement components,  $\beta$  is the angle of rotation of the tangent to the meridian in a meridian plane. The subscript  $n$  indicates the circumferential harmonic number, or the number of full wave patterns along the circumference of the shell. The four remaining variables  $Q$ ,  $N_{\varphi}$ ,  $N$ ,  $M_{\varphi}$  are the stress variables. The details of the stress variables will be explained later in the section.

Table 1 is an example of the printout data from the general stiffened shell computer program. The data is for the complete Instrument Unit scale model including the supporting shell structures. It represents the first mode corresponding to circumferential harmonic number  $n = 4$ . The natural frequency is 87.245 cps. The tabulated data is set in eight columns representing the eight variables listed above. Each row of the tabulated data refers to a section of the shell structure from one end to the other as defined by the input data. In Table 1, the data referring to the Instrument Unit proper is listed in rows 11-14. The data is in non-dimensional form. The dimensional values used to normalize the variables are listed directly above the table. The data  $w_{\max}$  in inches has been used to normalize the first through third columns  $w$ ,  $u$ ,  $v$ . The data  $\beta_{\max}$  in radian has been used to normalize  $\beta$  data in the fourth column. The  $N_{\varphi \max}$  data in (lb./in.) has been used to normalize  $Q$ ,  $N_{\varphi}$ ,  $N$  data in the fifth through seventh columns. The  $M_{\varphi \max}$  data in (lb. in./in.) has been used to normalize the  $M_{\varphi}$  data in the eighth column. To obtain a dimensional value of the modal data, the non-dimensional quantity of Table 1 is to be multiplied by the proper normalizing data  $w_{\max}$ ,  $\beta_{\max}$ , etc.

BETA LISTING

0.312174E 00 -0.739306E-01 0.122257E 01 -0.604736E-02  
 -0.739306E-01 0.194476E 00 -0.267588E-03  
 0.122257E 01 -0.147675E 00 0.490030E 01 -0.247557E-01  
 -0.604736E-02 -0.267588E-03 -0.247557E-01 0.274801E-03

VALUES THAT NORMALIZATION ARE DONE WITH NFIXMAX = 0.151059E-01 MFI MAX = 0.113649E-03  
 OWMAX = 0.140415E-04 RMAX = 0.684696E-06

R \* 24 MATRIX OF THE MODE SHAPES

U	V	BETA	Q	N-PHI	N	M-PHI
0.37840181E 00	-0.36393126E-01	0.31083412E 00	0.	0.	0.	0.
0.47513054E 00	-0.35560408E-01	-0.11886386E 00	-0.98100505E-03	0.16725558E 00	-0.29706668E 00	0.19596841E 00
0.55707621E 00	-0.3443212E-01	-0.13983338E 00	0.31612480E-03	0.30769581E 00	-0.27367588E 00	0.10110404E 00
0.63558159E 00	-0.32619178E-01	-0.15980066E 00	0.24788035E-03	0.43775138E 00	-0.25290660E 00	0.12297824E 00
0.70884512E 00	-0.30188715E-01	-0.17843080E 00	0.23050135E-03	0.55882659E 00	-0.22925393E 00	0.13863350E 00
0.77559607E 00	-0.27198534E-01	-0.19539963E 00	0.20696837E-03	0.66362156E 00	-0.20315324E 00	0.15305052E 00
0.83468316E 00	-0.23712461E-01	-0.21041662E 00	0.18003680E-03	0.75698284E 00	-0.17484386E 00	0.16577575E 00
0.88509882E 00	-0.19799646E-01	-0.22322749E 00	0.14984034E-03	0.83588826E 00	-0.14461376E 00	0.17659847E 00
0.92599560E 00	-0.15533981E-01	-0.23361713E 00	0.12307866E-03	0.89946306E 00	-0.11275743E 00	0.18574532E 00
0.97935249E 00	-0.10991342E-01	-0.24141341E 00	0.10739394E-03	0.94705676E 00	-0.80214060E-01	0.18398940E 00
0.98812903E 00	-0.62960319E-02	-0.24685618E 00	-0.31687861E-02	0.97684342E 00	-0.33198067E-01	0.37738385E 00
0.99270847E 00	-0.37376187E-02	-0.24862656E 00	-0.15883392E-05	0.99390751E 00	-0.31950956E-01	0.56674539E 00
0.10000000E 01	-0.11581917E-02	-0.24964376E 00	-0.10528009E-01	0.10000000E 01	-0.12248767E-03	0.42459228E 01
0.97392344E 00	0.63099488E-02	0.24939208E 00	-0.47636922E-01	0.99940953E 00	0.52210603E-01	0.19665190E 00
0.42941931E 00	0.13112947E-02	0.24130816E 00	-0.37742797E-04	0.98457842E 00	0.90011186E-01	0.20582457E 00
0.87292939E 00	0.16137047E-01	-0.21593741E 00	0.54761167E-04	0.94889513E 00	0.12758945E 00	0.20241015E 00
0.72418351E 00	0.20760800E-01	-0.19871607E 00	-0.10872826E-03	0.89086281E 00	0.16472886E 00	0.19568060E 00
0.63315214E 00	0.25037958E-01	-0.17860166E 00	-0.16786837E-03	0.80910057E 00	0.20108531E 00	0.18479951E 00
0.53238814E 00	0.28844589E-01	-0.15578786E 00	-0.30344990E-03	0.70226524E 00	0.23634095E 00	0.16934802E 00
0.42338389E 00	0.32049784E-01	-0.1305579E 00	-0.37755612E-03	0.56904235E 00	0.27022021E 00	0.14893757E 00
0.30852958E 00	0.34515988E-01	-0.10328442E 00	-0.47848979E-03	0.40813359E 00	0.30239929E 00	0.12209520E 00
0.18664530E 00	0.36902703E-01	-0.04500385E-01	0.26803097E-03	0.21760460E 00	0.33630150E 00	0.14079811E 00
	0.36902703E-01	0.93700373E-01	0.	0.	0.	0.

1  
 MQ UNDERFLOW AT LOCATION 41701  
 MQ UNDERFLOW AT LOCATION 42022  
 MQ UNDERFLOW AT LOCATION 42134  
 MQ UNDERFLOW AT LOCATION 55662  
 MQ UNDERFLOW AT LOCATION 42036

TABLE 1. TYPICAL PRINT-OUT OF THE GENERAL STIFFENED SHELL PROGRAM

Among the eight variables,  $w_n$ ,  $u_n$ ,  $\beta_n$ ,  $Q_n$ ,  $N_{\phi n}$ ,  $M_{\phi n}$  are in phase to each other, while  $v_n$  and  $N_n$  are out of phase. For instance, corresponding to a given meridian location of the shell, the circumferential position with the largest  $w_n$  also has largest  $u_n$ ,  $\beta_n$ ,  $Q_n$ ,  $N_{\phi n}$ ,  $M_{\phi n}$ . The displacement component  $v_n$  and the stress component  $N_n$  vanish at the same position. Moving one quarter wave length along the circumference of the shell, which corresponds to an angle of  $\left(\frac{\pi}{2n}\right)$  radian,  $w_n$ ,  $u_n$ ,  $\beta_n$ , etc vanish, while  $v_n$ ,  $N_n$  are at their maximum amplitudes. From the normal displacement point of view, the latter circumferential position is the position where a meridian nodal line passes. Altogether, there are  $(2n)$  nodal lines evenly spaced along the circumference of the shell. The nodal line distribution described above has been confirmed in tests with some exceptions. In the exceptional cases, the modal shapes are influenced due to deviation in symmetry of the shell structures or other related reasons. The exceptions include skewed nodal lines or a change of harmonic number from one shell segment to the next where a ring stiffener acts as the barrier.

The four shell stress components  $Q_n$ ,  $N_{\phi n}$ ,  $N_n$ ,  $M_{\phi n}$  deserve further explanation.  $Q_n$  is the amplitude of the modified transverse shear along a meridian. Ignoring the periodic time function  $\cos \omega t$ , the transverse shear may be expressed below:

$$Q = Q_{\phi} + \frac{1}{r} \frac{\partial M_{\theta\phi}}{\partial \theta} = Q_n \cos n\theta$$

where  $Q_{\phi}$  is the transverse shear force, the term  $\left(\frac{1}{r} \frac{\partial M_{\theta\phi}}{\partial \theta}\right)$  represents the additional transverse shear force needed at an open edge to compensate for the variation of the twisting moment  $M_{\theta\phi}$ . In general, when a shell is cut by a plane normal to its axis of revolution, the modified transverse shear force  $Q_n \cos n\theta$  is to be supplied by the supporting system in order to retain the response pattern for the segmented shell. In practice, it is not possible to design a supporting system satisfying all the edge conditions. Tests and analyses have shown that in a localized shell with substantial weight attachment along the edges, the mode shapes and natural frequencies of the original shell may be retained. Corresponding to the retained mode shapes, certain internal stresses may vary substantially from the stresses in the original shell. The reason for the deviation is partially due to the variation in boundary conditions such as the transverse shear  $Q_n$  described above. On the other hand, the added weights, which are used to retain the modal patterns of the original shell, have substantial inertia during vibration. The inertia forces have a decisive effect on the internal stresses of the localized shell. The inertia effect of the attached weights may be observed from the modal data obtained by the finite difference program described later in the report.

The sixth column of the tabulation gives the in-plane force along the meridian direction:

$$N_{\phi} = N_{\phi n} \cos n\theta$$

The seventh column of the tabulation gives the modified in-plane shear force along a section normal to the meridian

$$N = N_n \sin n\theta$$

$$= \left( N_{\theta\varphi n} + \frac{\sin\varphi}{r} M_{\theta\varphi n} \right) \sin n\theta$$

where the term including  $M_{\theta\varphi n}$  represents the additional edge shear force needed to form a couple to balance the twisting moment component due to the shell curvature. The eighth column of the tabulation gives the bending moment in the meridian plane

$$M_\varphi = M_{\varphi n} \cos n\theta$$

The remaining stresses in the shell may be computed using the following formulas:

$$N_{\theta n} = \nu N_{\varphi n} + \frac{Eh}{r} (w_n \sin\varphi + u_n \cos\varphi + n v_n)$$

$$M_{\theta n} = \nu M_{\varphi n} + \frac{Eh^3}{12r} \left( \eta_n^2 w_n + \beta_n \cos\varphi + \frac{n}{r} \sin\varphi v_n \right)$$

$$M_{\theta\varphi n} = \frac{D(1-\nu)}{2r} \left[ -2n \cos\varphi \frac{w_n}{r} + nJ u_n + H \cos\varphi v_n - 2n\beta_n \right] + \frac{\sin\varphi}{Kr} DN_n$$

$$Q_{\theta n} = -\frac{n}{r} M_{\theta n} + \frac{d}{ds} M_{\theta\varphi n} + 2 \frac{\cos\varphi}{r} M_{\theta\varphi n} + \frac{\rho\omega^2 h^3}{12r} (nw_n + v_n \sin\varphi)$$

where

$$J = \frac{1}{R_\varphi} + \frac{\sin\varphi}{r}$$

$$H = \frac{1}{R_\varphi} - \frac{\sin\varphi}{r}$$

$\rho$  is the material density,  $\omega$  is the natural frequency in radian/sec.  $D$  is the shell section modulus. The reader is referred to the first year progress report for definitions of the shell geometry.

SECTION IV  
LOCALIZED SHELL DESIGN

In designing a localized shell, it may be convenient to start with a major modal pattern of the complete shell structure. Along the circumferential direction, cut a section somewhat less than two half-wave lengths of the major mode. (Each half-wave encompasses an angle of  $\pi/n$  radian.) In case there are more than one major mode under consideration, then if physically possible, select the one with the longer wave length. In this manner, the mode(s) with the shorter wave length(s) may be retained through proper design of the localized shell and its supporting system. Along the meridian direction, the localized shell may be cut along some convenient locations. For a thin shell of the cylindrical shape, it has been found that the interaction of the shell stresses along the meridian and the circumferential directions is weak. As long as the two major dimensions of the localized shell assume a reasonable proportion, the meridian length of the localized shell is flexible. For a conical shell section, the situation is somewhat different. In either case, it is best to design a localized shell with ring stiffeners along the two circumferential edges.

The localized shell is supported by a number of cantilever beam type springs. A typical design for a scale model is shown in Figure 3. The spring has a free hinge connection to the shell structure. The formula for computing the spring linear stiffness at the connection point is:

$$K = \frac{3EI}{l^3} \text{ lb./in. for a beam with a uniform section}$$

$$K = \frac{E}{\int_0^l \frac{(l-x)^2}{I} dx} \text{ lb./in. for a beam with a variable section}$$

In either case, the length of the cantilever beam is  $l$ .  $x$  is measured from the built-in end. The integration for the variable section beam may be carried out using the area-moment method as shown in the numerical example of Section VI. In designing the spring support, it is advisable to leave the beam length  $l$  somewhat flexible subject to final adjustment. The number of spring supports used at each edge of the localized shell is arbitrary. At or near the spring supports, it is necessary to attach a number of weights in order to bring the natural frequency of the localized shell down to the level of the original shell. An approximate formula to determine the weight needed for each spring is:

$$W \cong \frac{Kg}{4\pi^2 f^2} \text{ lb.}$$

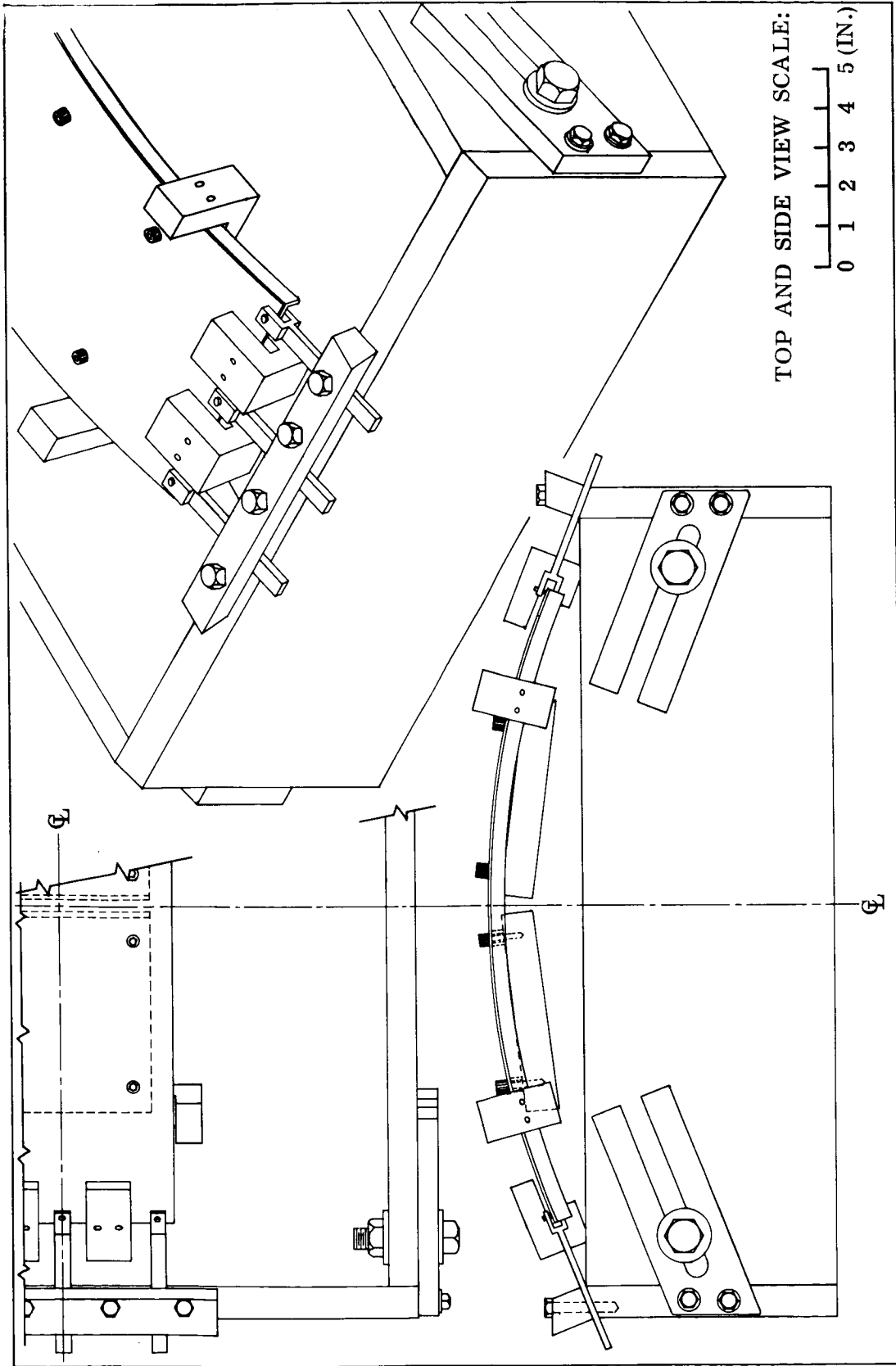


FIGURE 3. VIBRATION FIXTURE FOR S-4B INSTRUMENT UNIT SEGMENT



where  $K$  is the linear spring constant computed previously, the  $g$  is the gravitational constant,  $f(\text{cps})$  is the natural frequency of the major mode of the original shell structure. The above formula is used as a general guide. The actual frequency of the localized shell will be influenced by the shell structure which will be determined by the finite difference computer program described later in the procedure. The weights are usually attached to the edge of the shell near the spring supports. In attaching the weights, consideration should be given to the dynamic stress concentration which may cause damage to the shell structure. For a honeycomb sandwich shell, filler material should be used to replace the honeycomb material along the edge.

The shape of the attached weight is determined essentially through its moment of inertia  $I_{\theta}$  about an axis coinciding with the edge of the shell. Again, no precise formula is available. The following formula may be used to determine the approximate sectional dimensions of the attached weight:

$$M_{\theta n} L \cong I_{\theta n} \beta_{\theta n} \omega^2$$

where  $M_{\theta n}$  is determined by the equation given in Section III based on the general stiffened shell data.  $\beta_{\theta n}$  is the amplitude of the angle of rotation of the shell which may be computed by the following formula

$$\beta_{\theta n} = \frac{n w_n}{r} + \frac{\sin \varphi}{r} v_n$$

$L$  is the length of the edge influenced by the attached weight,  $\omega$  is the natural frequency in rad./sec. For instance, using the data of row 14 of Table 1 and the formula for  $M_{\theta n}$  in Section III, the following numerical values are computed:

$$M_{\theta n} = 1.818 \times 0.1136 \times 10^{-3} = 0.206 \times 10^{-3} \text{ lb. in. / in.}$$

$$\beta_{\theta n} = 2.70 \times 10^{-6} \text{ rad.}$$

$$L = 2.7 \text{ in.}$$

$$\omega = 2\pi(87.2) = 548 \text{ rad. / sec.}$$

which yield an approximate value of the moment of inertia of each attached weight:

$$I_s = I_{\theta n} = 6.85 \times 10^{-4} \text{ lb. in. sec.}^2$$

Because of fabrication considerations etc., the actual value of  $I_s$  in the test setup is  $5.85 \times 10^{-4} \text{ lb. in. sec.}^2$ . Along the circumferential edges, additional weights are attached. These weights serve to pull down the natural frequencies of the localized shell as well as to force a nodal line along the meridian direction for certain low frequency modes. Again, the manner in which the mass and the location of the attachment influence the over all modal pattern is obtained through the finite difference computer program.

SECTION V  
FINITE DIFFERENCE COMPUTER PROGRAM

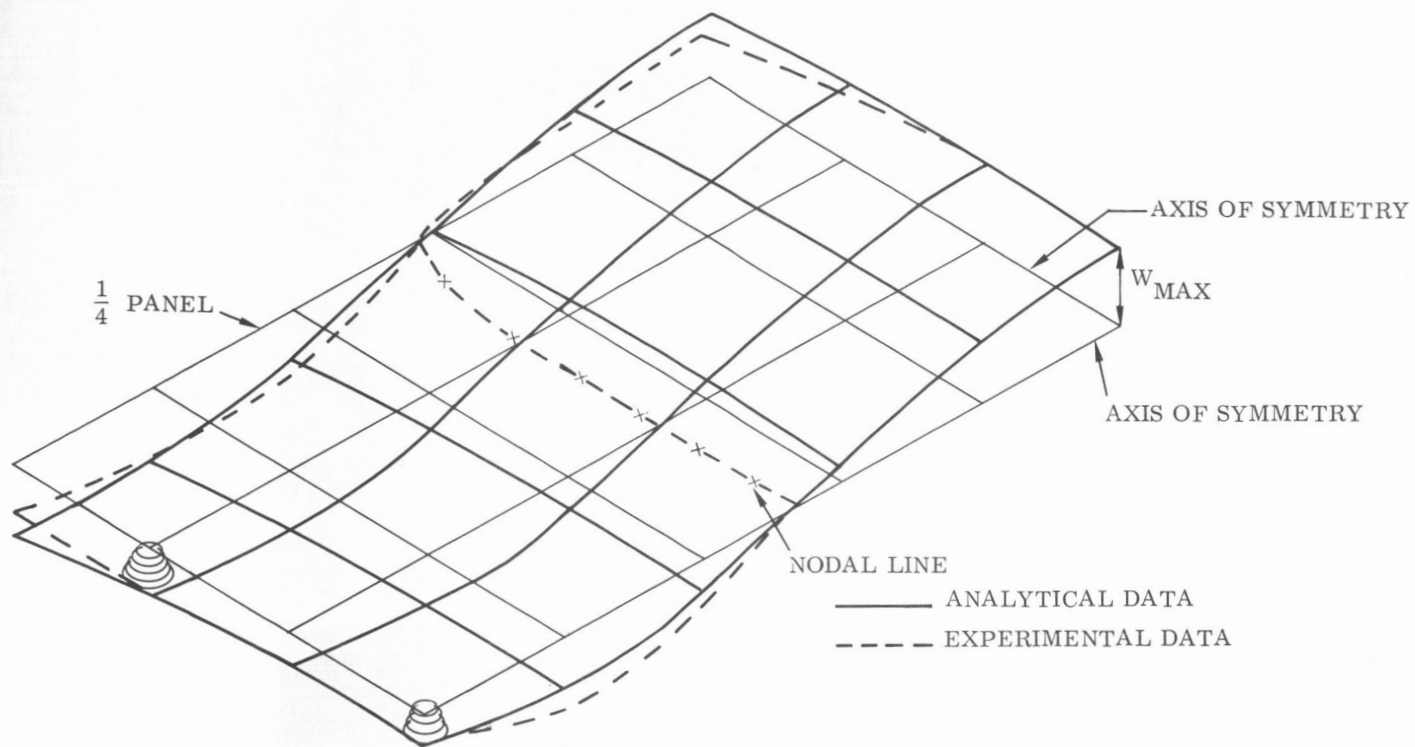
The finite difference approach, based on the coupled shell equations of Vlasov is developed for a curved panel cross-stiffened by two sets of orthogonal stiffeners. The panel considered may have a number of rigid or spring supports at arbitrary points along the edges of the panel.

Any number of concentrated weights may be attached to the edges or the internal points of the shell structure. For a component connected to the shell through a number of attachment points, the moment of inertia of the component is considered.

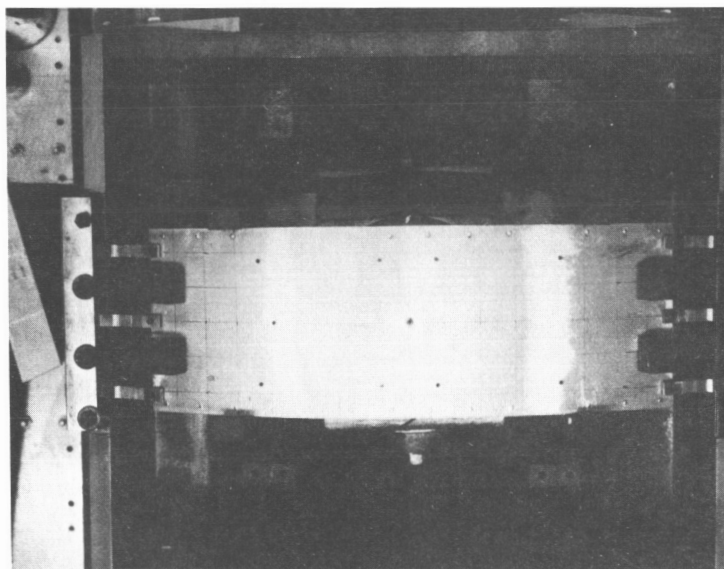
The computer program uses a grid pattern prescribed on the localized shell. The distance between two neighboring grid lines is a constant in each direction. The shell structure data, the spring support data and the attached weight data serve as input to the computer program. The program also interprets the local boundary conditions which are converted into finite difference equations.

All the shell equations are reduced by the program into a matrix form. After a number of internal operations which reduces the size of the eigen-matrix, the natural frequencies and the corresponding modal shapes of the localized shell are obtained using a standard eigenvalue eigen-matrix routine. The program produces the first five (5) natural modes. More modes may be generated as needed. The details of the program and its usage are described in the Appendix.

The modal data generated by the computer program are the basis of the localized shell design. For a given localized shell configuration, the computed data are compared with the test and analytical data of the complete shell structure. If the major modal patterns of the localized shell duplicate those of the complete shell, then the design may be considered satisfactory. Otherwise, the localized shell configuration including the spring supports and the weight attachments may be adjusted in order to reach a better correspondence in the modal patterns. The process may be continued until a satisfactory localized shell design is reached. Typical modal data obtained by this program and the corresponding test data are shown in Figure 4. Experimental results for both mode shape and frequency compared well with the predicted analytical values. Additional deformation data are given in Appendix I.



(a)  $f_{\text{computed}} = 213.7 \text{ cps}$



(b)  $f_{\text{test}} = 214 \text{ cps}$

FIGURE 4. TYPICAL (a) ANALYTICAL AND (b) TEST MODAL PATTERN OF THE LOCALIZED SHELL STRUCTURE

SECTION VI  
USE OF THE SCALE MODEL

The full scale shell structure of SATURN V system is large in size and is expensive. In order to ensure a satisfactory testing technique using localized shell structure, the analytical techniques are developed which are described in the previous sections. Experience indicates that no matter how extensive are the analytical techniques, there will always be some deviation between the predicted analytical data and the test data. Thus, if time and expenses are allowable, it is advisable to use a simple scale model to investigate the dynamic responses of the proposed localized shell design. The work in Northrop has shown that with proper design, the scale model and the corresponding full scale data are quite comparable when proper scale factors are used to interpret the data. The basic technique for a scale model design is given in p. 91, vol. I of Northrop Norair Yearly Progress Report NAS 8-20025. Essentially, the technique is based on the dynamic shell equations which resolve certain important design factors to establish the ratio of the natural frequencies of the scale model and the full scale structure. The same technique is used to correlate the impedance data. For the design of the shell proper, the following equation is used:

$$\frac{\omega_s^2}{\omega_f^2} = \frac{\left(\frac{D}{E}\right)_s \left(\frac{E}{\rho h}\right)_s \left(\frac{1}{L^4}\right)_s}{\left(\frac{D}{E}\right)_f \left(\frac{E}{\rho H}\right)_f \left(\frac{1}{L^4}\right)_f}$$

In the above equation,  $\omega$  is the frequency in rad./sec. Subscripts s, f indicate the scale model and the full scale structure respectively. On the righthand side of the equation,  $\frac{D}{E}$  is a measure of the bending stiffness of the shell. For the Instrument Unit, the scale model is made of a solid sheet, while the actual structure is a honeycomb sandwich. In this case, different formulas are used to compute the shell stiffness D. The factor  $\frac{E}{\rho h}$  has a dimension of acceleration. It is a measure of the relative significance of the shell internal forces and the inertia effect of the shell element. For the sandwich shell, the value h is replaced by H which is the compact thickness of the shell. In other words, H is the thickness of the sandwich shell if it is crushed into a solid sheet. The value  $\frac{1}{L^4}$  is the contribution due to the overall scaling factor, L being a typical overall dimension. As explained in the Yearly Progress Report, the equation applied to the Instrument Unit scale model yields a frequency ratio as shown below:

$$\frac{\omega_s}{\omega_f} = 3.25$$

The above frequency ratio is based on an overall scaling factor of 6.67. Now that the frequency ratio has been determined, the other design parameters are to be adjusted to reach a consistent design. For instance, the concentrated mass  $M$  is scaled according to the following relation:

$$\frac{\omega_s^2}{\omega_f^2} = \frac{D_s \left(\frac{1}{M}\right)_s \left(\frac{1}{L^2}\right)_s}{D_f \left(\frac{1}{M}\right)_f \left(\frac{1}{L^2}\right)_f}$$

The same relation may be established for a concentrated force  $F$  applied to the structure:

$$\frac{\omega_s^2}{\omega_f^2} = \frac{D_s \left(\frac{1}{F}\right)_s \left(\frac{1}{L^2}\right)_s}{D_f \left(\frac{1}{F}\right)_f \left(\frac{1}{L^2}\right)_f}$$

where  $F$  is the amplitude of the concentrated force with circular frequency  $\omega$ . The scale model of the segmented Instrument Unit is shown in Figure 3. Based on the above scaling relations, the design of the corresponding full-scale localized shell is shown in Figure 5. In the following, the determination of the attached weights and the spring supports for the full scale structure are explained.

The attached weights to the scale model Instrument Unit segment weigh 1/2 lb. each (see Fig. 3). The width of the weights is 1 in. Based on the actual dimensions of the weights, the polar moment of inertia  $I_s$  of each block along an axis parallel to the edge of attachment is:

$$I_s = 5.85 \times 10^{-4} \text{ lb. in. sec.}^2$$

so that

$$gI_s = 0.2258 \text{ lb. -in.}^2$$

Scale relation for the mass moment inertia may be based on the following:

$$\frac{I_f}{I_s} = \frac{(ML^2)_f}{(ML^2)_s}$$

Alternatively, the scale relation may be determined based on edge moment:

$$\begin{aligned} \text{Edge moment} &= \frac{I}{L} \beta \omega^2 \\ &\propto \frac{I \omega^2 w}{L^2} \propto D \frac{w}{L^2} \end{aligned}$$

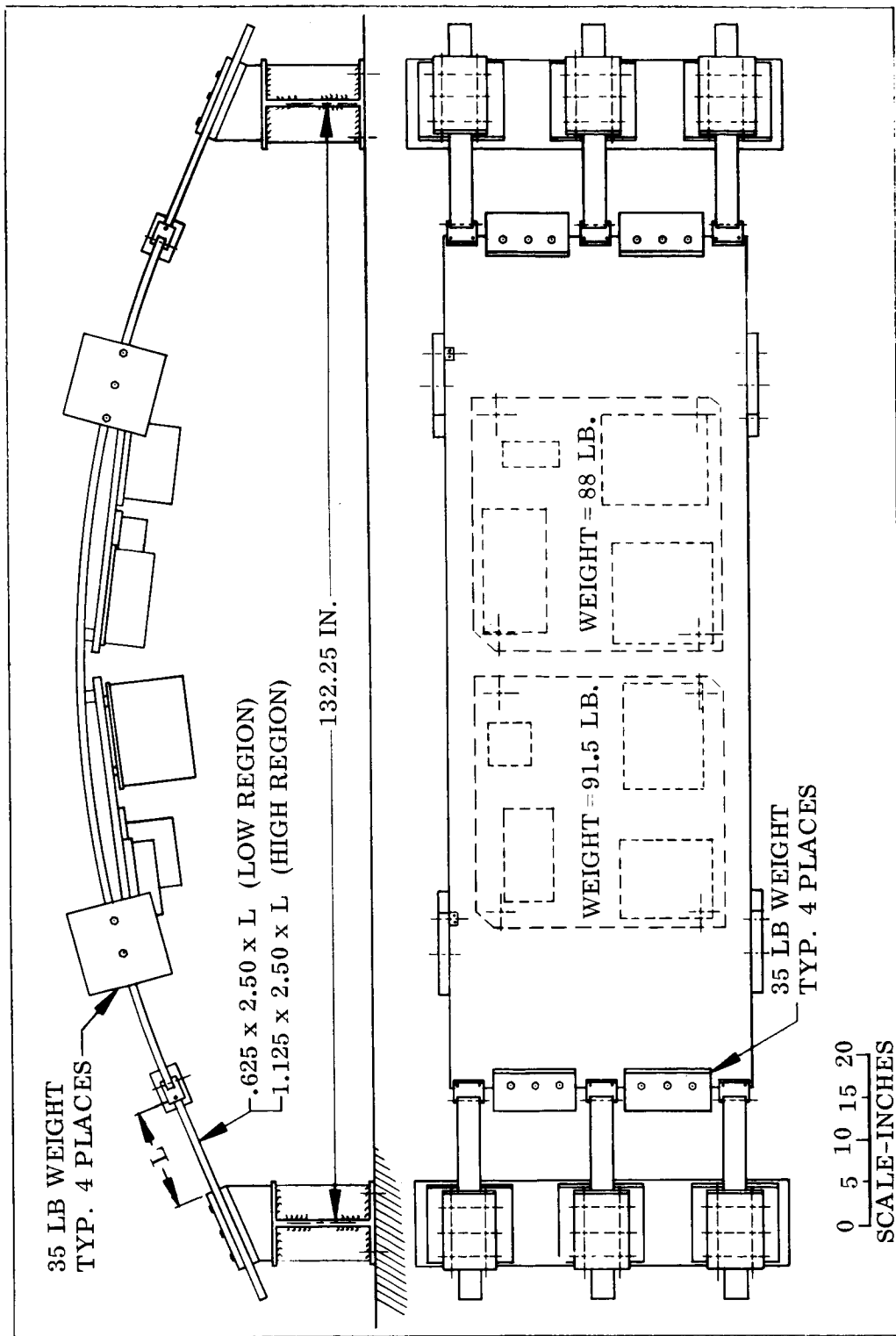


FIGURE 5. VIBRATION JIG FOR S4B 45° INSTRUMENT UNIT SEGMENT

so that

$$\frac{I_f}{I_s} = \left(\frac{D}{\omega^2}\right)_f / \left(\frac{D}{\omega^2}\right)_s = \frac{(ML^2)_f}{(ML^2)_s}$$

For the full scale piece, the attachment weighs 35.25 lb. each. The required ratio between the mass moments of inertia is:

$$\frac{I_f}{I_s} = 70.5 \times 6.67^2 = 3130$$

With no consideration to stress concentration, position of edge stiffener, and convenience in fabrication, the dimensions in Figure 6 are first suggested for the full scale piece attachments using steel blocks. Based on the dimensions shown, the moment of inertia is computed:

$$gI_f = \frac{70.5}{12} \left[ 1.265^2 + \frac{10.3}{9.3} (10.3)^2 - 1^2 \right] = 702 \text{ lb. -in.}^2$$

so that

$$\frac{gI_f}{gI_s} = \frac{702}{.2258} = 3110$$

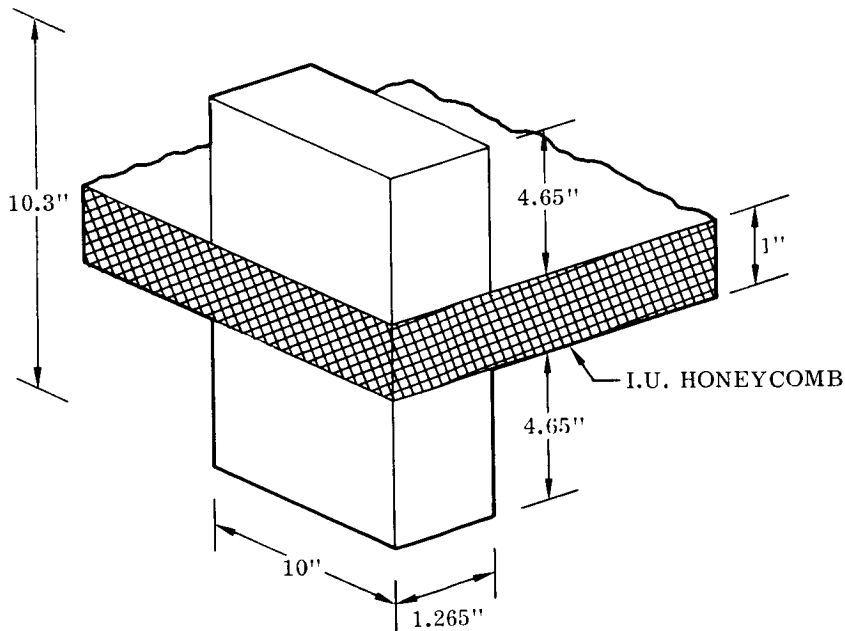


FIGURE 6. SUGGESTED DIMENSIONS FOR ATTACHED WEIGHTS

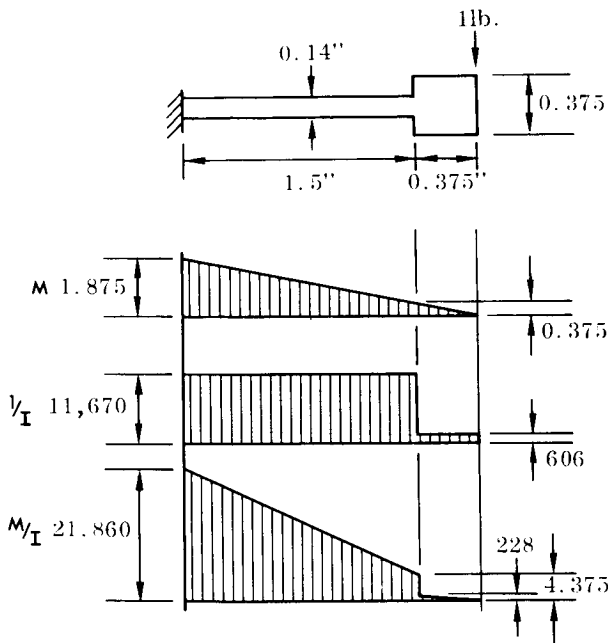
As may be observed from Figure 5, the actual dimensions of the attached weights have been modified from the suggested configuration for the four longitudinal edge pieces. The modification was due to design and fabrication considerations.

The scaling formula for the spring supports of the localized shell is:

$$\frac{K_s}{K_f} = \frac{(\omega^2 M)_s}{(\omega^2 M)_f} = \frac{(D/L^2)_s}{(D/L^2)_f} = \frac{1}{6.67}$$

The spring constants  $K_s$  for the I. U. model are obtained in the following manner for a soft spring and a stiff spring.

Case I SOFT SPRING,  $50 \leq (\omega_s/2\pi) \leq 250$  CPS.



$$b = 0.375''$$

$$E_s = 10.3 \times 10^6 \text{ PSI}$$

$$\frac{E_s}{K_s} = \frac{1}{3} \times 228 \times (0.375)^2$$

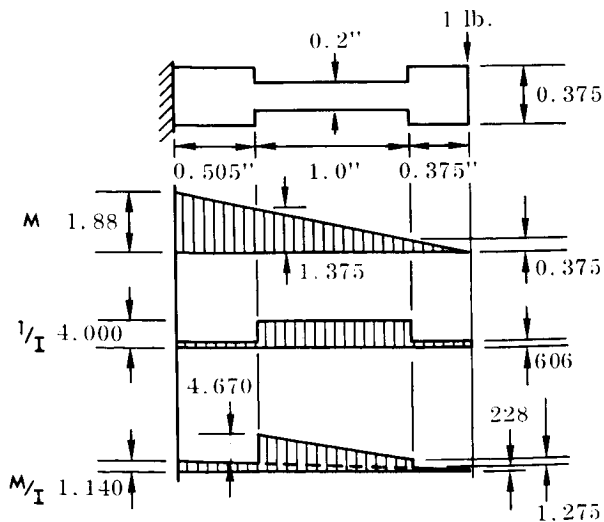
$$+ \frac{1}{2} \times 4375 \times 1.5 \times 0.875$$

$$+ \frac{1}{2} \times 21860 \times 1.5 \times 1.375$$

$$= 25,480 \text{ IN.}^{-1}$$

$$K_s = 404.2 \text{ LB/IN.}$$

CASE II STIFF SPRING,  $(\omega_s/2\pi) > 250$  CPS



$$b = 0.375''$$

$$E_s = 10.3 \times 10^6 \text{ PSI}$$

$$\frac{E_s}{K_s} = \frac{1}{3} \times 1140 \times 1.88^2$$

$$+ \frac{1}{2} \times 1275 \times 1 \times 0.708$$

$$+ \frac{1}{2} \times 4670 \times 1 \times 1.042$$

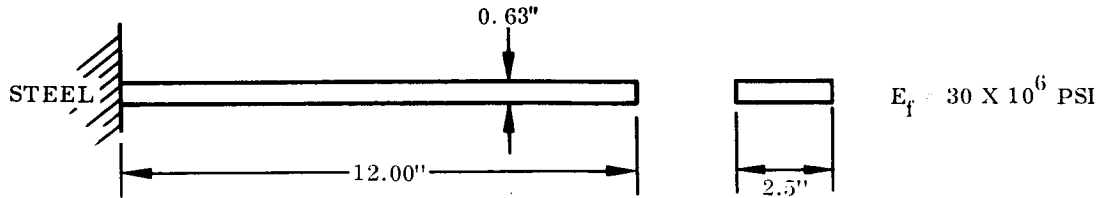
$$= 4230 \text{ IN.}^{-1}$$

$$K_s = 2440 \text{ LB/IN.}$$



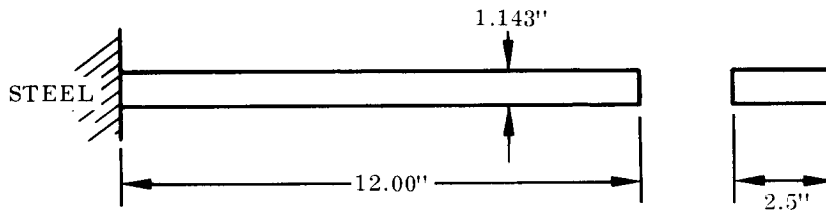
Based on the scaling formula given previously, two spring supports for the full-scale unit Instrument Unit panel are designated below using carbon steel stock as beam material.

Case I. LOW FREQUENCY REGION ( $\omega_f/2\pi \leq 77$  CPS).



$$K_f = 2700 \text{ LB. /IN.} \quad 6.67 K_s$$

CASE II, HIGH FREQUENCY REGION. ( $\omega_f/2\pi > 77$  CPS)



$$K_f = 16250 \text{ LB. /IN.} \quad 6.67 K_s$$

The driving point impedance plot of the full scale shell with component attachments is shown in Figure 7. Also plotted is the corresponding impedance data for the scale model test specimen with proper adjustment in the scales used. From the figure, it may be seen that the general impedance pattern is preserved to an agreeable degree through the dynamic scaling technique. In the high frequency region, the deviation is believed due to the high structural damping of the intricately fabricated full scale shell structure as compared to the simple scale model.

The arrowheads shown in Figure 7 indicate the computed eigenfrequencies of the full scale I. U. segment. The frequencies are computed for mode shapes symmetrical with respect to both center lines of the segment using the finite difference program. Among the computed eigenfrequencies, none corresponds to the test frequency for the first low impedance point. Possible reasons of the omission of the frequency may be due to the non-symmetry of the mode shape, the local vibration coupled to the mechanical excitation device, etc. This point will be further investigated.

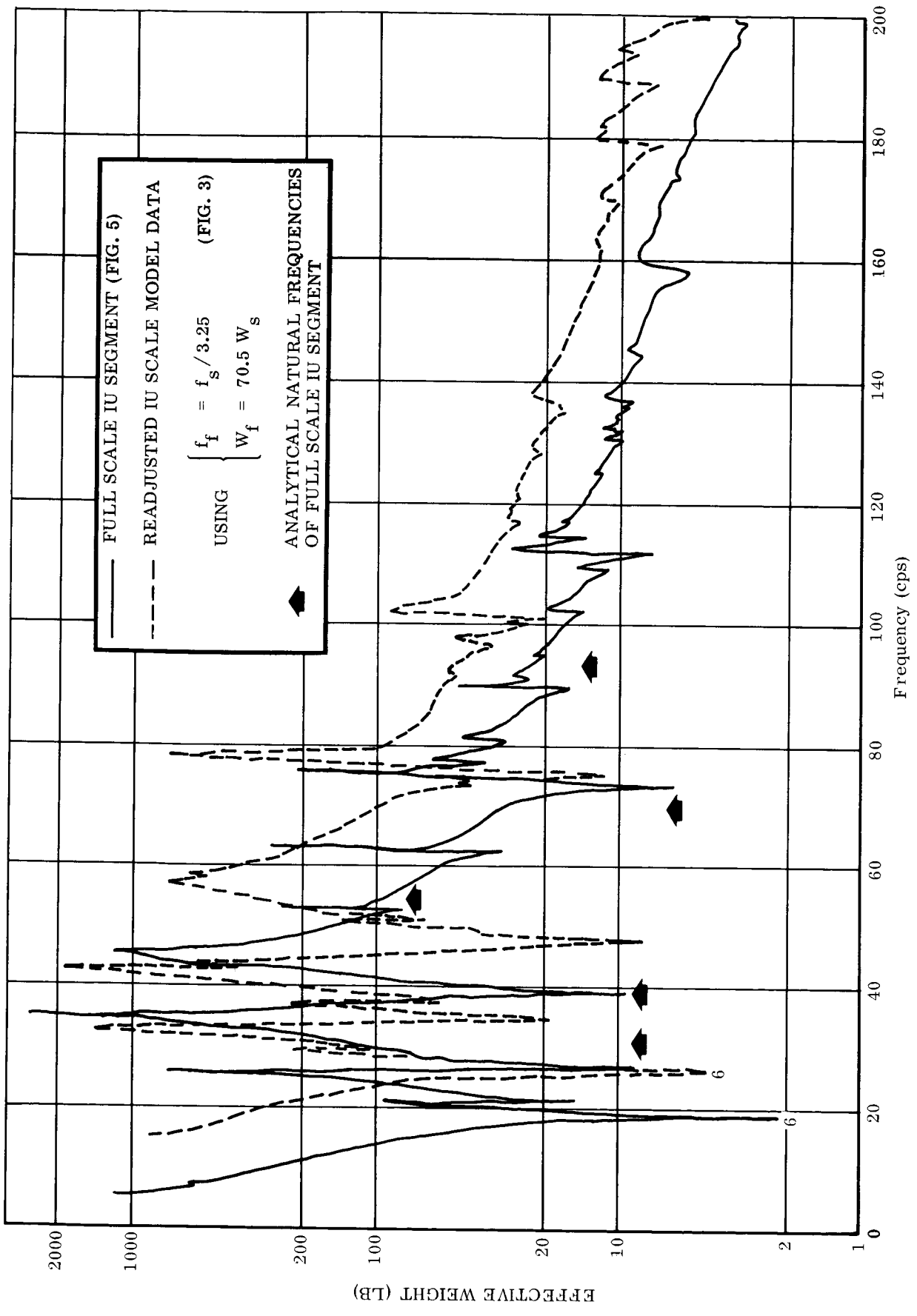


FIGURE 7. DRIVING POINT IMPEDANCE OF INSTRUMENT UNIT SEGMENT WITH COMPONENTS ATTACHED, SUPPORT CONFIGURATION (I)

## APPENDIX I

### THE FINITE DIFFERENCE COMPUTER PROGRAM USED IN LOCALIZED SHELL VIBRATION TESTS

The finite difference approach is developed for a curved panel cross-stiffened by two sets of equidistant stiffeners. The reinforcement, which is assumed to be symmetrical about an axis normal to the middle surface of the shell, is attached to the skin of the cylindrical panel in either circumferential or axial direction. The panel to be considered may have a number of rigid or spring supports at discrete points along the edge of the panel.

#### A. BASIC EQUATIONS

##### 1. Strain-Displacement Relations

The extensional strain-displacement relations for the middle surface of a cylindrical shell are given by

$$\epsilon_x = \frac{\partial u}{\partial x} \quad (1)$$

$$\epsilon_\varphi = \frac{1}{a} \left( \frac{\partial v}{\partial \varphi} + w \right) \quad (2)$$

$$\gamma_{x\varphi} = \frac{1}{a} \frac{\partial u}{\partial \varphi} + \frac{\partial v}{\partial x} \quad (3)$$

where

$u, v, w$  = displacement components in the axial, circumferential and radial directions.

$x$  = distance along axial direction

$\varphi$  = cylindrical angle (radian)

$a$  = radius of cylinder (see Figure 8)

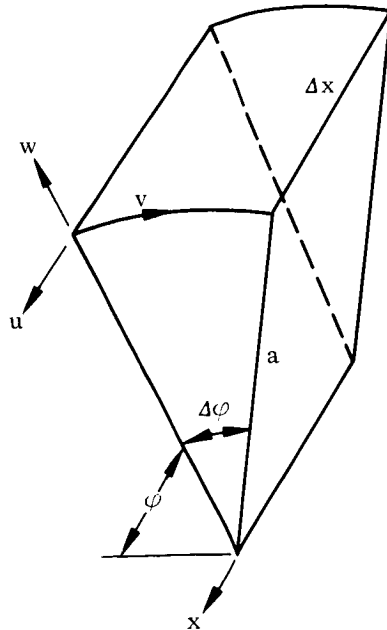


FIGURE 8. ELEMENT OF A CYLINDRICAL SHELL

The changes of curvature of the cylindrical panel may be represented as follows:

$$k_x = -\frac{\partial^2 w}{\partial x^2} \quad (4)$$

$$k_\varphi = -\frac{1}{a} \left( \frac{\partial^2 w}{\partial \varphi^2} - \frac{\partial v}{\partial \varphi} \right) \quad (5)$$

$$k_{x\varphi} = -\frac{1}{a} \left( \frac{\partial^2 w}{\partial x \partial \varphi} - \frac{\partial v}{\partial x} \right) \quad (6)$$

For a cylindrical panel, experience has shown that the effect of the  $v$ -displacement on the bending moments is negligible. With this simplification, the following expressions are obtained:

$$k_x = -\frac{\partial^2 w}{\partial x^2} \quad (4)$$

$$k_\varphi = -\frac{1}{a} \frac{\partial^2 w}{\partial \varphi^2} \quad (5')$$

$$k_{x\varphi} = -\frac{1}{a} \frac{\partial^2 w}{\partial x \partial \varphi} \quad (6')$$

## 2. Stress-Strain Relations

For a cylindrical panel cross-stiffened by two sets of equidistant stiffeners, it is assumed that the reinforcement is symmetrical about an axis normal to the middle surface of the panel. Using the above assumption, the following approximate stress-strain relations (Ref. 1) may be established:

$$N_x = \frac{E A_{sx}}{a \Delta \varphi} \epsilon_x + \frac{Eh}{1 - \nu^2} (\epsilon_x + \nu \epsilon_\varphi) \quad (7)$$

$$N_\varphi = \frac{E A_{s\varphi}}{\Delta x} \epsilon_\varphi + \frac{Eh}{1 - \nu^2} (\epsilon_\varphi + \nu \epsilon_x) \quad (8)$$

$$N_{x\varphi} = N_{\varphi x} \simeq Gh \gamma_{x\varphi} \quad (9)$$

$$M_x = \frac{E k_x}{a \Delta \varphi} \int_{A_{sx}} z_x^2 dA + \frac{E}{1 - \nu^2} (k_x + \nu k_\varphi) \int_{\text{panel}} z_x^2 dz \quad (10)$$

$$M_\varphi = \frac{E k_\varphi}{\Delta x} \int_{A_{s\varphi}} z_\varphi^2 dA + \frac{E}{1 - \nu^2} (k_\varphi + \nu k_x) \int_{\text{panel}} z_\varphi^2 dz \quad (11)$$

$$M_{x\varphi} = -M_{\varphi x} \simeq -\frac{Eh^3}{12(1 + \nu)} k_{x\varphi} \quad (12)$$

---

Ref. 1 S. Timoshenko, S. Woinowsky-Krieger, "Theory of Plates and Shells," pp. 364-370.

where

$A_{sx}, A_{s\phi}$  = cross sectional area of stiffener\*

$I_{sx}, I_{s\phi}$  = moment of inertia of stiffener about its own centroid axis\*

$z_x, z_\phi$  = distance measured from the neutral axis of shell-stiffener combination (see Figure 9)

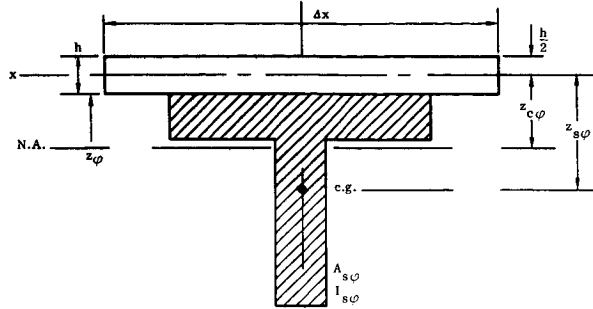


FIGURE 9. CROSS-SECTIONAL VIEW OF A SHELL-STIFFENER COMBINATION

$h$  = thickness of a solid panel or the total facing thickness of a honeycomb sandwich panel

$E$  = Young's modulus

$\nu$  = Poisson's ratio

The following sectional rigidity properties are defined:

$$K = \frac{Eh}{1 - \nu^2}$$

$$K_x = K + \frac{E A_{sx}}{a \Delta \phi}$$

$$K_\phi = K + \frac{E A_{s\phi}}{\Delta x}$$

$$D = \frac{Eh^3}{12(1 - \nu^2)} \quad \text{for solid panel}$$

$$D = \frac{Eh(h + 2c)^2}{16(1 - \nu^2)} \quad \text{for honeycomb sandwich panel} \quad (13)$$

$$D'_x = D + K z_{cx}^2$$

$$D'_\phi = D + K z_{c\phi}^2$$

$$D_x = D'_x + \frac{E}{a \Delta \phi} \left[ I_{sx} + A_{sx} (z_{sx} - z_{cx})^2 \right]$$

$$D_\phi = D'_\phi + \frac{E}{\Delta x} \left[ I_{s\phi} + A_{s\phi} (z_{s\phi} - z_{c\phi})^2 \right]$$

\*Note: For a stiffener which is placed along one boundary of the panel, double the values of the corresponding cross sectional area and the moment of inertia as input.

where

$z_{sx}, z_{s\phi}$  = distances from c.g. 's of the stiffeners to the middle surface of the panel

$z_{cx}, z_{c\phi}$  = distances between the centroids of the stiffeners to neutral axis of shell-stiffener combination (see Figure 9).

$c$  = core thickness of a honeycomb sandwich panel

Using the newly defined rigidity expressions, the stress-strain relations may be re-written as:

$$N_x = K_x \epsilon_x + \nu K \epsilon_\phi \quad (7')$$

$$N_\phi = K_\phi \epsilon_\phi + \nu K \epsilon_x \quad (8')$$

$$N_{x\phi} = N_{\phi x} = \frac{(1 - \nu)}{2} K \gamma_{x\phi} \quad (9')$$

$$M_x = D_x k_x + \nu D'_x k_\phi \quad (10')$$

$$M_\phi = D_\phi k_\phi + \nu D'_\phi k_x \quad (11')$$

$$M_{x\phi} = -M_{\phi x} = -D(1 - \nu) k_{x\phi} \quad (12')$$

Figure 10 shows the notation for coordinates, displacements, force and moment components for a cylindrical shell element.

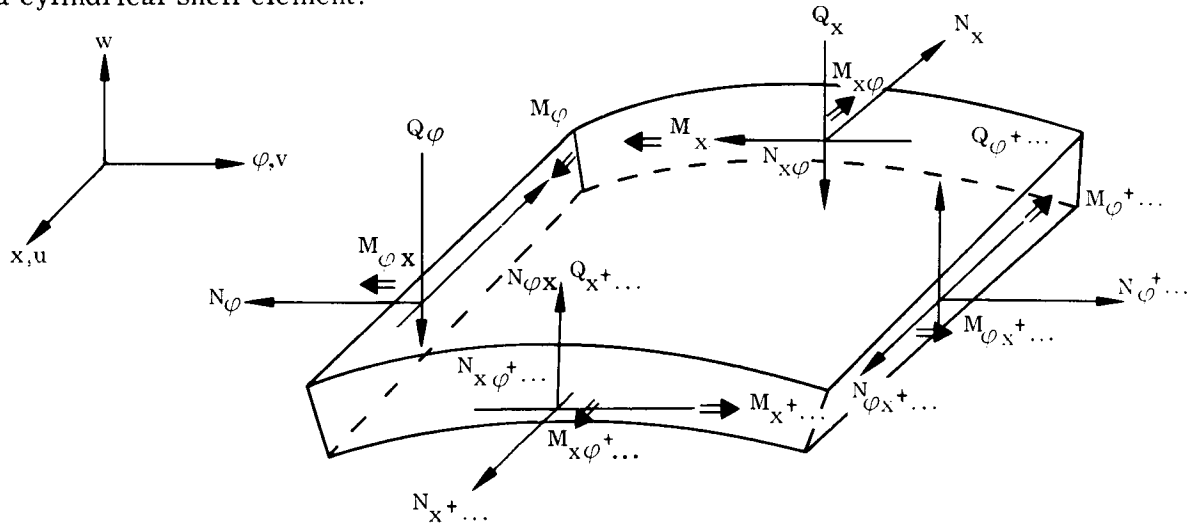


FIGURE 10. TYPICAL SHELL ELEMENT SHOWING THE SIGN CONVENTION OF COORDINATES, DISPLACEMENTS AND INTERNAL STRESSES

### 3. Equilibrium Equation

For a cylindrical shell element, the equilibrium condition in the radial direction is

$$\frac{\partial^2 M_x}{\partial x^2} + \frac{\partial^2 M_{\phi x}}{a \partial x \partial \phi} - \frac{\partial^2 M_{x\phi}}{a \partial x \partial \phi} + \frac{\partial^2 M_\phi}{a^2 \partial \phi^2} - \frac{1}{a} N_\phi + q = 0 \quad (14)$$

where  $q$  is the external load density applied on the shell element in the radial direction. By substituting equation (10')-(12') into (14), the following expression is reached:

$$D_x \frac{\partial^4 w}{\partial x^4} + 2H \frac{1}{a} \frac{\partial^4 w}{\partial x^2 \partial \varphi^2} + D_\varphi \frac{1}{a} \frac{\partial^4 w}{\partial \varphi^4} + \frac{1}{a} N_\varphi = q \quad (14')$$

where  $2H = \nu (D'_x + D'_\varphi) + 2D(1 - \nu)$

#### 4. Stress Function and Compatibility Condition

A stress function  $\phi$  is introduced in this analysis in order to reduce the number of unknowns from three ( $u$ ,  $v$ , and  $w$ ) to two ( $w$  and  $\phi$ ). The membrane forces  $N_x$ ,  $N_\varphi$  and  $N_{x\varphi}$  can be expressed in terms of the stress function  $\phi$ :

$$N_x = - \frac{\partial^2 \phi}{a^2 \partial \varphi^2} \quad (16)$$

$$N_\varphi = - \frac{\partial^2 \phi}{\partial x^2} \quad (17)$$

$$N_{x\varphi} = N_{\varphi x} = \frac{\partial^2 \phi}{a \partial x \partial \varphi} \quad (18)$$

In order to use the stress function approach, an equation of compatibility is needed to define  $\phi$ . This is accomplished by combining equations (1)-(3) to obtain the following relation:

$$\frac{\partial^2 \epsilon_\varphi}{\partial x^2} + \frac{1}{a^2} \frac{\partial^2 \epsilon_x}{\partial \varphi^2} - \frac{1}{a} \frac{\partial^2 \gamma_{x\varphi}}{\partial x \partial \varphi} - \frac{1}{a} \frac{\partial^2 w}{\partial x^2} = 0 \quad (19)$$

Using equations (7')-(9') and (16)-(18), the strain components  $\epsilon_x$ ,  $\epsilon_\varphi$  and  $\gamma_{x\varphi}$  may be expressed as:

$$\epsilon_\varphi = - \frac{1}{K_x K_\varphi - \nu^2 K^2} \left[ K_x \frac{\partial^2 \phi}{\partial x^2} - \nu K \frac{\partial^2 \phi}{a^2 \partial \varphi^2} \right] \quad (20)$$

$$\epsilon_x = - \frac{1}{K_x K_\varphi - \nu^2 K^2} \left[ K_\varphi \frac{\partial^2 \phi}{a^2 \partial \varphi^2} - \nu K \frac{\partial^2 \phi}{\partial x^2} \right] \quad (21)$$

$$\gamma_{x\varphi} = \frac{2}{(1 - \nu) K} \frac{\partial^2 \phi}{a \partial x \partial \varphi} \quad (22)$$

Substituting equations (20)-(22) into equation (19) yields the compatibility condition:

$$K_x \frac{\partial^4 \phi}{\partial x^4} + \frac{K_x K_\varphi - \nu K^2}{(1 - \nu) K} \frac{2}{a^2} \frac{\partial^4 \phi}{\partial x^2 \partial \varphi^2} + K_\varphi \frac{\partial^4 \phi}{a^4 \partial \varphi^4} + \frac{K_x K_\varphi - \nu^2 K^2}{a} \frac{\partial^2 w}{\partial x^2} = 0 \quad (23)$$

The equilibrium equation (14') can be rewritten in the form of

$$D_x \frac{\partial^4 w}{\partial x^4} + 2H \frac{1}{a} \frac{\partial^4 w}{\partial x^2 \partial \varphi^2} + D_\varphi \frac{1}{a} \frac{\partial^4 w}{\partial \varphi^4} - \frac{1}{a} \frac{\partial^2 \phi}{\partial x^2} = q \quad (24)$$

Equations (23) and (24) are the two governing differential equations mechanized in the computer program using the finite difference approach. It can be shown that for the special case of a cylindrical shell without any stiffener, the above equations are identical to the coupled shell equations of Vlasov applied to the cylindrical panel. The Vlasov's equations are:

$$\begin{cases} D \nabla^2 \nabla^2 w - \frac{1}{a} \frac{\partial^2 \phi}{\partial x^2} = q \\ \nabla^2 \nabla^2 \phi + \frac{Eh}{a} \frac{\partial^2 w}{\partial x^2} = 0 \end{cases}$$

where  $\nabla^2$  is the Laplace operator.

### 5. Boundary Conditions for a Panel with Spring Supports

The cylindrical panel under consideration has spring supports at discrete points along the edges of the panel. These springs are assumed to be restrained against deflection but not restrained against rotation. Lateral deflection at spring supporting point may be either restricted or free. The boundary conditions to be considered are:

Along the edge  $x = \text{constant}$ ,

$$M_x = 0 \quad (25)$$

$$V_x = Q_x - \frac{1}{a} \frac{\partial M_{x\phi}}{\partial \phi} = \pm \frac{\bar{K}_x}{a \Delta \phi} w \quad (26)$$

$$N_x = 0 \text{ or } u = 0 \quad (27 \text{ a, b})$$

$$S_x = N_{x\phi} + \frac{1}{a} M_{x\phi} = 0 \quad (28)$$

Along the edge  $\phi = \text{constant}$ ,

$$M_\phi = 0 \quad (29)$$

$$V_\phi = Q_\phi + \frac{\partial M_{\phi x}}{\partial x} = \pm \frac{K_\phi}{\Delta x} w \quad (30)$$

$$N_\phi = 0 \text{ or } v = 0 \quad (31 \text{ a, b})$$

$$N_{\phi x} = 0 \quad (32)$$

At the corner point,  $x = \text{constant}$ ,  $\phi = \text{constant}$ ,

$$2M_{x\phi} = +\bar{K}_1 w \quad (33)$$

where  $V_x, V_\phi$  are the effective transverse shear forces,  $S_x$  is the effective in-plane shear force,  $\bar{K}_x, \bar{K}_\phi$  and  $\bar{K}_1$  are the spring constants for the point supports whose unit is (force/length). The spring constants equal to zero at a point where the edge is free.



The problem at hand is to determine the natural frequencies and the corresponding mode shapes of the panel. Thus, the loading on the panel is the inertial force:

$$q = \bar{m} \omega^2 w \quad (34)$$

where  $\bar{m}$  is the mass per unit area of the panel and  $\omega$  is the natural circular frequency of the panel. For an interior point where a weight  $W$  is attached, then the additional loading due to this weight is:

$$q_a = \frac{W\omega^2}{ga\Delta x\Delta\varphi} w \quad (35)$$

Similarly, for a boundary point where there is an attached weight  $W_x$ ,  $W_\varphi$  or  $W_1$ , equations (26), (30), and (33) are as shown below:

$x = \text{constant}$ ,

$$V_x = \pm \left( \bar{K}_x - \frac{W_x}{g} \omega^2 \right) \frac{w}{a\Delta\varphi} \quad (26')$$

$\varphi = \text{constant}$ ,

$$V_\varphi = \pm \left( \bar{K}_\varphi - \frac{W_\varphi}{g} \omega^2 \right) \frac{w}{\Delta x} \quad (30')$$

$x = \text{constant}$ ,  $\varphi = \text{constant}$ ,

$$2M_{x\varphi} = \pm \left( \bar{K}_1 - \frac{W_1}{g} \omega^2 \right) w \quad (33')$$

The signs used in equations (26'), (30'), and (33') depend on the relative positions between the boundary lines and the directions of the coordinates. Figure 11 shows the sign convention to be used in the equations corresponding to the coordinates system of Figure 10.

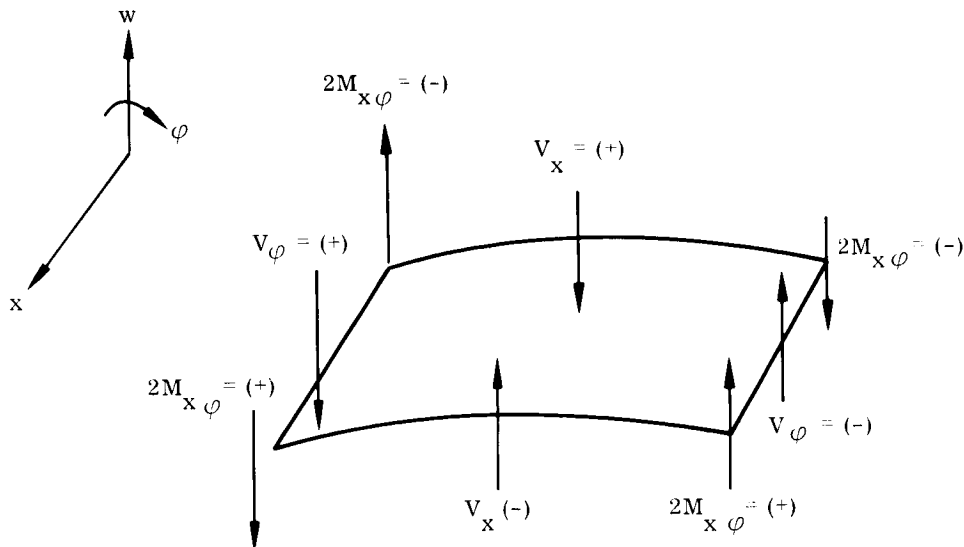


FIGURE 11. SIGN CONVENTION OF TRANSVERSE FORCES

## B. FINITE DIFFERENCE EXPRESSIONS

### 1. Grid Setup and Basic Finite Difference Operators

The geometry of the scaled-down Instrument Unit panel under test is shown in Figure 12. Since the panel is symmetrical with respect to both the  $x = \frac{b}{2}$  and  $\varphi = \frac{\varphi_0}{2}$  axes, one-quarter of this panel is considered for the finite difference solution. The quarter panel is shown in shade in Figure 12.

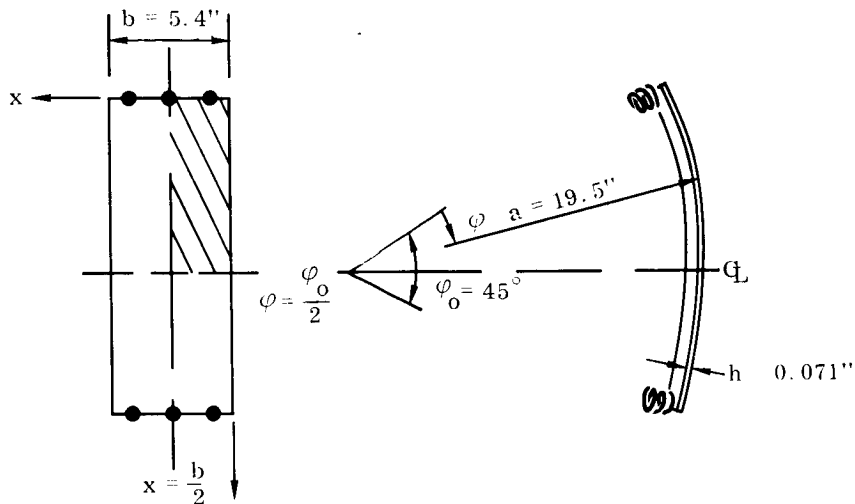


FIGURE 12. GEOMETRY OF SEGMENTED INSTRUMENT UNIT SCALE MODEL

Figure 13 shows the grid pattern with a station numbering system appropriate for finding the modes which are symmetrical about both the  $x = \frac{b}{2}$  and  $\varphi = \frac{\varphi_0}{2}$  axes. It should be emphasized that the grid pattern used is not a unique and fixed pattern. Different patterns may be used for various specimens, boundary conditions, etc. The computer program is designed with such a flexibility in mind.

Using the grid pattern of Figure 13, it may be visualized that the deformation function  $w$  and the auxiliary function  $\phi$  may be plotted over the grid as two surfaces. The finite difference computer program translates the coupled equations (23) and (24) into linear equations of  $w$  and  $\phi$  at various grid points. In order to facilitate the translation, the conversion of the partial derivatives into finite difference relations is first explained.

Using a grid with a grid pitch  $\Delta x$  in the  $x$  direction and a  $\Delta\varphi$  in the  $\varphi$  direction, the coefficients of the basic finite difference expressions for the partial derivatives of a function at a point  $i$  are as follows:

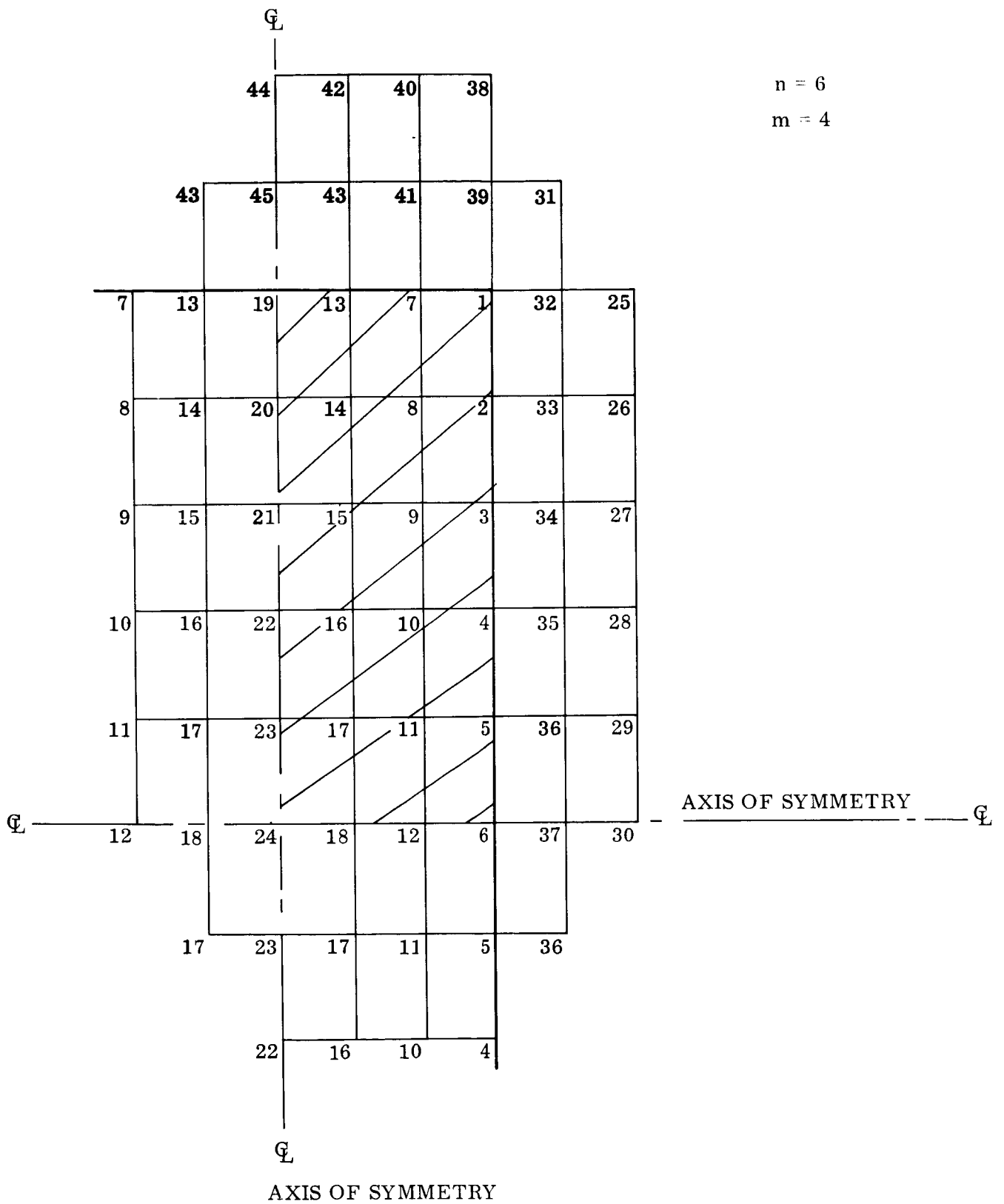


FIGURE 13. GRID PATTERN USED IN THE FINITE DIFFERENCE PROGRAM

$$2(\Delta x) \frac{\partial}{\partial x} = \begin{array}{|c|} \hline \begin{array}{ccc} 1 & 0 & -1 \\ & (i) & \end{array} \\ \hline \end{array}$$

$$2(\Delta \varphi) \frac{\partial}{\partial \varphi} = \begin{array}{|c|} \hline -1 \\ \hline \begin{array}{c} 0 \\ (i) \\ 1 \end{array} \\ \hline \end{array}$$

$$(\Delta x)^2 \frac{\partial^2}{\partial x^2} = \begin{array}{|c|} \hline \begin{array}{ccc} 1 & -2 & 1 \\ & (i) & \end{array} \\ \hline \end{array}$$

$$(\Delta \varphi)^2 \frac{\partial^2}{\partial \varphi^2} = \begin{array}{|c|} \hline 1 \\ \hline \begin{array}{c} -2 \\ (i) \\ 1 \end{array} \\ \hline \end{array}$$

$$(4\Delta x \Delta \varphi) \frac{\partial^2}{\partial x \partial \varphi} = \begin{array}{|c|} \hline \begin{array}{ccc} -1 & 0 & 1 \\ 0 & 0 & 0 \\ & (i) & \end{array} \\ \hline \begin{array}{ccc} 1 & 0 & -1 \end{array} \\ \hline \end{array}$$

$$2(\Delta x)^3 \frac{\partial^3}{\partial x^3} = \begin{array}{|c|} \hline \begin{array}{ccccc} 1 & -2 & 0 & 2 & -1 \\ & & (i) & & \end{array} \\ \hline \end{array}$$

$$2(\Delta \varphi)^3 \frac{\partial^3}{\partial \varphi^3} = \begin{array}{|c|} \hline -1 \\ \hline 2 \\ \hline \begin{array}{c} 0 \\ (i) \\ -2 \\ 1 \end{array} \\ \hline \end{array}$$

$$2(\Delta x)^2(\Delta \varphi) \frac{\partial^3}{\partial x^2 \partial \varphi} =$$

-1	2	-1
0	0	0
	(i)	
1	-2	1

$$2(\Delta x)(\Delta \varphi)^2 \frac{\partial^3}{\partial x \partial \varphi^2} =$$

1	0	-1
-2	0	2
	(i)	
1	0	-1

$$(\Delta x)^4 \frac{\partial^4}{\partial x^4} =$$

1	-4	6	-4	1
		(i)		

$$(\Delta \varphi)^4 \frac{\partial^4}{\partial \varphi^4} =$$

1
-4
6
(i)
-4
1

$$(\Delta x \Delta \varphi)^2 \frac{\partial^4}{\partial x^2 \partial \varphi^2} =$$

1	-2	1
-2	4	-2
	(i)	
1	-2	1

## 2. Equilibrium and Compatibility Equations.

Consider Equation (24); it may be rewritten in the following form:

$$\left(\frac{a\Delta\varphi}{\Delta x}\right)^4 \left(\frac{D}{D}\right) (\Delta x)^4 \frac{\partial^4 w}{\partial x^4} + \left(\frac{a\Delta\varphi}{\Delta x}\right)^2 \left(\frac{2H}{D}\right) (\Delta x \Delta \varphi)^2 \frac{\partial^4 w}{\partial x^2 \partial \varphi^2} + \left(\frac{D}{D}\right) (\Delta \varphi)^4 \frac{\partial^4 w}{\partial \varphi^4}$$

$$-\left(\frac{a\Delta\varphi}{\Delta x}\right)^2 \frac{a(\Delta\varphi)^2}{D} (\Delta x)^2 \frac{\partial^2 \phi}{\partial x^2} = \frac{\bar{m}(a\Delta\varphi)^4 \omega^2}{D} \left[1 + \frac{W}{\bar{m}g\Delta x a \Delta\varphi}\right] w \quad (36)$$

Using symbols  $\lambda = \frac{a\Delta\varphi}{\Delta x}$ ,  $\Omega = \frac{\bar{m}(a\Delta\varphi)^4 \omega^2}{D}$ , Equation (36) is:

$$\lambda^4 \frac{D_x}{D} (\Delta x)^4 \frac{\partial^4 w}{\partial x^4} + \lambda^2 \left(\frac{2H}{D}\right) (\Delta x \Delta\varphi)^2 \frac{\partial^4 w}{\partial x^2 \partial \varphi^2} + \left(\frac{D_\varphi}{D}\right) (\Delta\varphi)^4 \frac{\partial^4 w}{\partial \varphi^4} - \frac{\lambda^2 a (\Delta\varphi)^2}{D} (\Delta x)^2 \frac{\partial^2 \phi}{\partial x^2} = \Omega \left(1 + \frac{W}{\bar{m}g\Delta x a \Delta\varphi}\right) w \quad (36')$$

Applying the finite difference operators to replace the partial derivatives, the coefficients for various points on the w-grid and  $\phi$ -grid are determined in the following manner. Referring to Equation (36'), the partial derivatives are translated into finite differences on the w-grid and the  $\phi$ -grid corresponding to a grid point marked by  $(w_i)$  or  $(\phi_i)$ . In the formulation, the inertia term in the equation is omitted for the time being.

$\frac{D_\varphi}{D}$			
$\frac{2H}{D} \lambda^2$	$-4\left(\frac{H}{D} \lambda^2 + \frac{D_\varphi}{D}\right)$	$\frac{2H}{D} \lambda^2$	
$\frac{D_x}{D} \lambda^4$	$-4\lambda^2\left(\frac{D_x}{D} \lambda^2 + \frac{H}{D}\right)$	$6\frac{D_x}{D} \lambda^4 + 8\frac{H}{D} \lambda^2 + 6\frac{D_\varphi}{D}$ $(w_i)$	$-4\lambda^2\left(\frac{D_x}{D} \lambda^2 + \frac{H}{D}\right) \frac{D_x}{D} \lambda^4$
$\frac{2H}{D} \lambda^2$	$-4\left(\frac{H}{D} \lambda^2 + \frac{D_\varphi}{D}\right)$	$\frac{2H}{D} \lambda^2$	
$\frac{D_\varphi}{D}$			

$$\boxed{-\frac{\lambda^2(a\Delta\phi)^2}{aD} \quad 2\frac{\lambda^2(a\Delta\phi)^2}{aD} \quad -\frac{\lambda^2(a\Delta\phi)^2}{aD}}_{(\phi_1)}$$

The above finite difference operators are applied to all stations on one-quarter of the panel including boundary points (Sta. 1-24). In laying out the coefficients of the finite difference equations in a systematic manner, visualize a square matrix of size (rxr), where r is the number of the total unknown w and  $\phi$  values for all the grid points. The columns of the matrix correspond to the coefficients of  $w_1, w_2, \dots, \phi_1, \phi_2, \dots$  arranged in that order. For Equation (36') applied to a grid station, a row of matrix is established listing all the coefficients of the finite difference equation as described above. Other rows are added to fill up the (rxr) matrix representing the equilibrium equation, the compatibility equation and the boundary conditions. The detail of the matrix organization will be further explained later in the section.

The finite difference operators for compatibility equation (23) are illustrated below:

$$\boxed{\begin{array}{ccccc} & & \frac{K_\phi}{K} & & \\ & 2P\lambda^2 & & -4\left(\frac{K_\phi}{K} + P\lambda^2\right) & & 2P\lambda^2 \\ \frac{K_x}{K}\lambda^4 & & -4\lambda^2\left(P + \frac{K_x}{D}\lambda^2\right) & & 6\frac{K_\phi}{K} + 8P\lambda^2 + 6\frac{K_x}{K}\lambda^4 & & -4\lambda^2\left(P + \frac{K_x}{K}\lambda^2\right) & & \frac{K_x}{K}\lambda^4 \\ & & & & (\phi_i) & & & & \\ & 2P\lambda^2 & & -4\left(\frac{K_\phi}{K} + P\lambda^2\right) & & 2P\lambda^2 & & & \\ & & & & \frac{K_\phi}{K} & & & & \end{array}}$$

$$\boxed{R(a\Delta\phi)^2\lambda^2 \quad -2R(a\Delta\phi)^2\lambda^2 \quad R(a\Delta\phi)^2\lambda^2}_{(w_i)} \quad (23')$$

where 
$$P = \frac{K_x K_\phi - \nu K^2}{(1 - \nu)K^2}$$

$$R = \frac{K_x K_\phi - \nu^2 K^2}{aK}$$

Experience shows that it is sufficient to apply Equation (23) only to the interior points (sta. 8-12, 14-18, 20-24) in order to fill the final coefficient matrix described above.

### 3. Boundary Conditions.

Equations (25) - (33) are rewritten in terms of  $w$  and  $\phi$  for this particular panel as shown below:

$$\left[ D_x \frac{\partial^2 w}{\partial x^2} + \nu D_x' \frac{\partial^2 w}{a^2 \partial \varphi^2} \right]_{x=0} = 0 \quad (37)$$

$$\left\{ D_x \frac{\partial^3 w}{\partial x^3} + \frac{1}{a} \alpha \frac{\partial^3 w}{\partial x \partial \varphi^2} + \frac{\bar{K}_x w}{a \Delta \varphi} \right\}_{x=0} = \left[ \frac{W_x \omega^2}{g a \Delta \varphi} w \right]_{x=0} \quad (38)$$

$$\left. \frac{\partial^2 \phi}{\partial \varphi^2} \right|_{x=0} = 0 \quad \text{or} \quad u \Big|_{x=0} = 0 \quad (39 \text{ a, b})$$

$$\left[ \frac{\partial^2 \phi}{\partial x \partial \varphi} + \frac{D(1-\nu)}{a} \frac{\partial^2 w}{\partial x \partial \varphi} \right]_{x=0} = 0 \quad (40)$$

$$\left[ D_\varphi \frac{\partial^2 w}{a^2 \partial \varphi^2} + \nu D_\varphi' \frac{\partial^2 w}{\partial x^2} \right]_{\varphi=0} = 0 \quad (41)$$

$$\left\{ D_\varphi \frac{\partial^3 w}{a^3 \partial \varphi^3} + \frac{1}{a} \beta \frac{\partial^3 w}{\partial x^2 \partial \varphi} + \frac{\bar{K}_\varphi w}{\Delta x} \right\}_{\varphi=0} = \left[ \frac{W_\varphi \omega^2}{g \Delta x} w \right]_{\varphi=0} \quad (42)$$

$$\left. \frac{\partial^2 \phi}{\partial x^2} \right|_{\varphi=0} = 0 \quad \text{or} \quad v \Big|_{\varphi=0} = 0 \quad (43 \text{ a, b})$$

$$\left. \frac{\partial^2 \phi}{\partial x \partial \varphi} \right|_{\varphi=0} = 0 \quad (44)$$

$$\left[ 2D(1-\nu) \frac{\partial^2 w}{a \partial x \partial \varphi} + K_1 w \right]_{\substack{x=0 \\ \varphi=0}} = \left[ \frac{W_1 \omega^2}{g} w \right]_{\substack{x=0 \\ \varphi=0}} \quad (45)$$

where  $\alpha = \nu D_x' + 2(1-\nu)D$

$\beta = \nu D_\varphi' + 2(1-\nu)D$

Since only the mode shapes which are symmetric with respect to both  $x = \frac{b}{2}$  and  $\varphi = \frac{\varphi_0}{2}$  axes are considered, the following conditions governing  $u$ ,  $v$ ,  $\phi$  along the axes of symmetry are made use of:



$$v \Big|_{\varphi = \frac{\varphi_0}{2}} = 0 \qquad \frac{\partial \phi}{\partial \varphi} \Big|_{\varphi = \frac{\varphi_0}{2}} = 0$$

and

$$u \Big|_{x = \frac{b}{2}} = 0 \qquad \frac{\partial \phi}{\partial x} \Big|_{x = \frac{b}{2}} = 0$$

Equation (2) is substituted into (20) which is then integrated from  $\varphi = 0$  to  $\varphi = \frac{\varphi_0}{2}$ . The resulting condition for  $v = 0$  at  $\varphi = 0$  is:

$$\begin{aligned} v \Big|_{\substack{\varphi=0 \\ x=x^*}} &= \int_0^{\varphi_0/2} \left[ \frac{a}{K_x K_\varphi - \nu^2 K^2} \left( K_x \frac{\partial^2 \phi}{\partial x^2} - \nu K \frac{\partial^2 \phi}{a^2 \partial \varphi^2} \right) + w \right]_{x=x^*} d\varphi \\ &= \int_0^{\varphi_0/2} \left[ \frac{1}{R} \frac{K_x}{K} \frac{\partial^2 \phi}{\partial x^2} + w \right]_{x=x^*} d\varphi + \frac{\nu}{a^2 R} \left( \frac{\partial \phi}{\partial \varphi} \right)_{\substack{x=x^* \\ \varphi=0}} = 0 \end{aligned} \quad (46)$$

Similar approach gives:

$$u \Big|_{\substack{x=0 \\ \varphi=\varphi^*}} = \int_0^{b/2} \left[ \frac{1}{a^2 R} \frac{K_\varphi}{K} \frac{\partial^2 \phi}{\partial \varphi^2} \right]_{\varphi=\varphi^*} dx + \frac{\nu}{R} \left( \frac{\partial \phi}{\partial x} \right)_{\substack{x=0 \\ \varphi=\varphi^*}} = 0 \quad (47)$$

The finite difference operators corresponding to equations (37) - (47) and the stations on which each operator will be applied are given below. The inertia terms are excluded.

$\frac{(\Delta x)^2}{D_x} M_x \Big|_{x=0} = 0$ . Apply to points along the edge  $x = 0$  (Sta. 1-6).

$$\begin{array}{ccc} & \nu \frac{D_x'}{D_x} \frac{1}{\lambda^2} & \\ & -2(1 + \nu \frac{D_x'}{D_x} \frac{1}{\lambda^2}) & \\ 1 & (w_i) & 1 \end{array} \quad (37')$$

$$\nu \frac{D_x'}{D_x} \frac{1}{\lambda^2}$$

$\frac{2(\Delta x)^3}{D_x} V_x \Big|_{x=0} = \frac{2(\Delta x)^3}{D_x} \frac{\bar{K}_x}{a\Delta\varphi} w$ . Apply to points along the edge  $x = 0$  (Sta. 1-6). Use  $\bar{K}_x = 0$  at points where there is no spring support.

$$\begin{array}{ccccccc}
 & -\frac{\alpha}{D_x \lambda^2} & & 0 & & \frac{\alpha}{D_x \lambda^2} & \\
 & & & & & & \\
 -1 & 2\left(1 + \frac{\alpha}{D_x \lambda^2}\right) & - & \frac{2\bar{K}_x (a\Delta\varphi)^2}{D_x \lambda^3} & -2\left(1 + \frac{\alpha}{D_x \lambda^2}\right) & 1 & (38') \\
 & & & (w_i) & & & \\
 & -\frac{\alpha}{D_x \lambda^2} & & 0 & & \frac{\alpha}{D_x \lambda^2} & 
 \end{array}$$

$(a\Delta\varphi)^2 N_x \Big|_{x=0} = 0$ . Apply to points along the edge  $x = 0$  without spring support.

$$\begin{array}{c}
 1 \\
 -2 \\
 (\phi_i) \\
 1
 \end{array}
 \quad (39')$$

$\frac{2K(a\Delta\varphi)\lambda R}{K_\varphi} u \Big|_{x=0} = 0$ . Apply to points along the edge  $x = 0$  with spring supports. This condition is not used for the panel under consideration because no spring support exists along the edge  $x = 0$ .

(axis of symmetry)

$$\begin{array}{ccccccc}
 1 & 2 & \dots & 2 & 2 & 1 & \\
 -2 & -4 & \dots & -4 & \left(-4 + \nu\lambda^2 \frac{2K}{K_\varphi}\right) & -2 & -\nu\lambda^2 \frac{K}{K_\varphi} \\
 1 & 2 & \dots & 2 & 2 & 1 & 
 \end{array}
 \quad (47')$$

$(4\Delta x a\Delta\varphi) S_x \Big|_{x=0} = 0$ . Apply to all points along the edge  $x = 0$  except the corner point. For points not on the axis of symmetry (Sta. 2-5)

$$\begin{array}{ccc} -1 & 0 & 1 \\ 0 & 0 & 0 \\ 1 & 0 & -1 \end{array} \quad (w_i)$$

$$\begin{array}{ccc} -\frac{D(1-\nu)}{a} & 0 & \frac{D(1-\nu)}{a} \\ 0 & 0 & 0 \\ \frac{D(1-\nu)}{a} & 0 & -\frac{D(1-\nu)}{a} \end{array} \quad (\phi_i)$$

(40'a)

For points on the axis of symmetry (Sta. 6)

$$\begin{array}{ccc} -1 & 0 & 1 \\ 1 & 0 & -1 \end{array} \quad (w_i)$$

$$\begin{array}{ccc} -\frac{D(1-\nu)}{a} & 0 & \frac{D(1-\nu)}{a} \\ \frac{D(1-\nu)}{a} & 0 & -\frac{D(1-\nu)}{a} \end{array} \quad (\phi_i)$$

(40'b)

$\frac{(a\Delta\varphi)^2}{D_\varphi} M_\varphi|_{\varphi=0} = 0$ . Apply to points along the edge  $\varphi = 0$  (Sta. 1, 7, 13, 19).

$$\begin{array}{ccc} 1 & & \\ \nu\lambda^2 \frac{D_\varphi'}{D_\varphi} & -2 \left[ 1 + \nu\lambda^2 \frac{D_\varphi'}{D_\varphi} \right] & \nu\lambda^2 \frac{D_\varphi'}{D_\varphi} \\ & (w_i) & \\ 1 & & \end{array}$$

(41')

$2 \frac{(a\Delta\varphi)^3}{D_\varphi} V_\varphi|_{\varphi=0} = 2 \frac{(a\Delta\varphi)^3}{D_\varphi} \frac{\bar{K}_\varphi}{\Delta x} w$ : Apply to points along the edge  $\varphi = 0$  (Sta. 1, 7, 13, 19).

Use  $\bar{K}_\varphi = 0$  for stations with no spring support.

$$\begin{array}{ccc} 1 & & \\ \frac{\beta\lambda^2}{D_\varphi} & -2 \left( 1 + \frac{\beta\lambda^2}{D_\varphi} \right) & \frac{\beta\lambda^2}{D_\varphi} \\ 0 & \frac{-2\lambda\bar{K}_\varphi (a\Delta\varphi)^2}{D_\varphi} & 0 \\ & (w_i) & \\ -\frac{\beta\lambda^2}{D_\varphi} & 2 \left( 1 + \frac{\beta\lambda^2}{D_\varphi} \right) & -\frac{\beta\lambda^2}{D_\varphi} \\ -1 & & \end{array}$$

(42')

$(\Delta x)^2 N_{\phi}|_{\phi=0} = 0$ . Apply to points along the edge  $\phi = 0$  without a spring support.

1	-2	1
$(\phi_i)$		

(43')

$2 \frac{K}{K_x} aR(\Delta x) v|_{\phi=0} = 0$ . Apply to points along the edge  $\phi = 0$  with spring supports.

$\frac{KR (\Delta x)^2}{K_x} (w_i)$ $2 \frac{KR}{K_x} (\Delta x)^2$ <p style="text-align: center;">.</p> <p style="text-align: center;">.</p> <p style="text-align: center;">.</p> $2 \frac{KR}{K_x} (\Delta x)^2$ $\frac{KR}{K_x} (\Delta x)^2$ <p style="text-align: center;">↑ (axis of symmetry)</p>		$-\frac{\nu K}{\lambda^2 K_x}$ <table style="width: 100%; text-align: center;"> <tr> <td style="padding: 5px;">1</td> <td style="padding: 5px;">-2</td> <td style="padding: 5px;">1</td> </tr> <tr> <td colspan="3" style="padding: 5px;"><math>(\phi_i)</math></td> </tr> <tr> <td style="padding: 5px;">2</td> <td style="padding: 5px;"><math>\left(\frac{\nu}{\lambda^2} \frac{K}{K_x} - 4\right)</math></td> <td style="padding: 5px;">2</td> </tr> <tr> <td style="padding: 5px;">2</td> <td style="padding: 5px;">-4</td> <td style="padding: 5px;">2</td> </tr> <tr> <td style="padding: 5px;">.</td> <td style="padding: 5px;">.</td> <td style="padding: 5px;">.</td> </tr> <tr> <td style="padding: 5px;">.</td> <td style="padding: 5px;">.</td> <td style="padding: 5px;">.</td> </tr> <tr> <td style="padding: 5px;">2</td> <td style="padding: 5px;">-4</td> <td style="padding: 5px;">2</td> </tr> <tr> <td style="padding: 5px;">-1</td> <td style="padding: 5px;">-2</td> <td style="padding: 5px;">-1</td> </tr> </table> <p style="text-align: center;">↑ (axis of symmetry)</p>	1	-2	1	$(\phi_i)$			2	$\left(\frac{\nu}{\lambda^2} \frac{K}{K_x} - 4\right)$	2	2	-4	2	.	.	.	.	.	.	2	-4	2	-1	-2	-1
1	-2	1																								
$(\phi_i)$																										
2	$\left(\frac{\nu}{\lambda^2} \frac{K}{K_x} - 4\right)$	2																								
2	-4	2																								
.	.	.																								
.	.	.																								
2	-4	2																								
-1	-2	-1																								

(46')

$(4\Delta x a \Delta \phi) N_{\phi_x}|_{\phi=0} = 0$ . Apply to points along the edge  $\phi = 0$  except the corner point (1). For point not on axis of symmetry (Sta. 7, 13)

-1	0	1
0	0	0
$(\phi_i)$		
1	0	-1

(44'a)

For point on axis of symmetry (Sta. 19)

$$\begin{array}{|c|c|}
 \hline
 -1 & 1 \\
 \hline
 0 & 0 \\
 (\phi_i) & \\
 \hline
 1 & -1 \\
 \hline
 \end{array} \quad (44'b)$$

$(4\Delta x a \Delta \varphi) 2M_{x\varphi} \Big|_{\substack{x=0 \\ \varphi=0}} = -(4\Delta x \Delta \varphi) \bar{K}_1 w$ . Apply to the corner point  $x = 0, \varphi = 0$  (Sta. 1)

$$\begin{array}{|c|c|c|}
 \hline
 -1 & 0 & 1 \\
 \hline
 0 & \frac{2\bar{K}_1 (a\Delta\varphi)^2}{D\lambda(1-\nu)} & 0 \\
 (w_i) & & \\
 \hline
 1 & 0 & -1 \\
 \hline
 \end{array} \quad (45')$$

Furthermore,  $\phi$  is an auxiliary stress function whose second derivatives give corresponding stress values. The value of  $\phi$  may be arbitrarily assigned at a certain station without influencing the stress distribution. The condition is represented as:

$$\phi_c = 0 \quad (48)$$

$$\begin{array}{|c|}
 \hline
 1 \\
 (\phi_i) \\
 \hline
 \end{array} \quad (48')$$

The above condition is applied to station (24).

#### 4. Finite Difference Expressions in Matrix Form.

Assembling of all of the equations described previously yields a set of homogeneous equations of the following form:

$$[A] \begin{Bmatrix} w_a \\ w_b \\ \phi \end{Bmatrix} - \Omega \begin{bmatrix} [M_1] & 0 & 0 \\ [M_2] & 0 & 0 \\ 0 & 0 & 0 \end{bmatrix} \begin{Bmatrix} w_a \\ w_b \\ \phi \end{Bmatrix} = 0 \quad (49)$$

where  $[A]$  = the coefficient matrix.

$w_a$  = the normal deflection matrix for the stations on the quarter panel.

$\{w_b\}$  = the normal deflection matrix for the exterior stations.

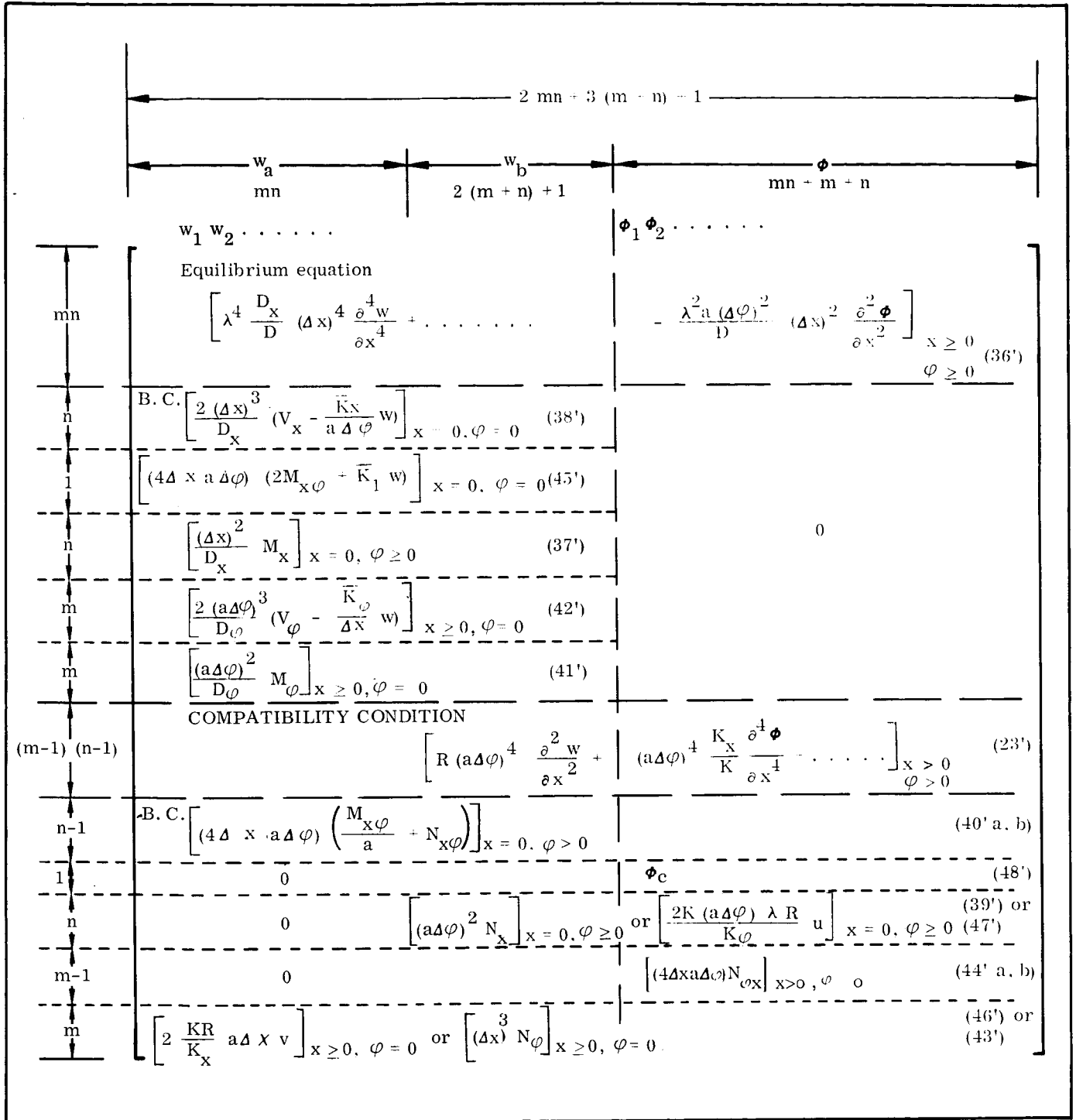
$\{M_1\}$  = the interior mass matrix representing the mass of the shell proper.

$\{M_2\}$  = the boundary mass matrix representing masses attached at the edges of the panel.

Consider a quarter panel covered by  $m$  equidistant grid points along the  $x$ -direction and  $n$  equidistant grid points along the  $\varphi$ -direction. A total of  $(m \times n)$  grid points are located on the panel. A stiffener with sectional area  $A_{sx}$  may be located along any grid line parallel to the  $x$ -axis. A stiffener with sectional area  $A_{s\varphi}$  may be located along the circular grid line parallel to the  $\varphi$ -direction. The number of stiffeners are not limited. Using the above structural panel, and referring to Equation (49), it may be shown that there are  $(mn)$  elements for  $\{w_a\}$ . There are  $(2m + 2n + 1)$  elements for  $\{w_b\}$  corresponding to the exterior stations extending two grid pitches from the panel edges (Figure 13). The auxiliary function  $\phi$  is accounted for extending one grid pitch from the edges. In this manner,  $[(m + 1)(n + 1) - 1]$  elements are established for  $\{\phi\}$ . Adding the numbers of the elements listed above, a total of  $[2mn + 3(m + n) + 1]$  is counted, which is the order of the square coefficient matrix  $[A]$ . The details of organizing the  $[A]$  matrix is illustrated in the diagram shown on the next page.

In the matrix shown on page 40, the finite difference equations used to generate the rows of the coefficient matrix are listed which are marked with the equation numbers described in the text. The following procedures may be used to generate the basic pattern. The pattern is used as the input to the computer program which computes the elements of  $A$ .

- a. Draw the grid pattern and the station numbers on a transparent paper.
- b. Simplify the finite difference operators by assigning an index number to each coefficient. Make a list of the index numbers and the corresponding coefficients. A typical index table used to generate the  $[A]$  matrix for the localized I. U. panel is shown in Table 2 at the end of Section B.
- c. Rewrite the finite difference operators by placing corresponding index numbers in position with the grid pattern as shown on the transparent paper.
- d. Place the transparent paper over a finite difference operator and keep the center coefficient coinciding with the station for which that operator will be read.
- e. Locate the row number as defined in the  $[A]$  matrix diagram shown on page 40. Read each station number on the transparent paper and the corresponding index number on the bottom sheet representing the differential operators of an equation. Enter the index number as an element of  $[A]$  whose column number corresponds to the station number read. Repeat the operation for  $w$  and  $\phi$ . If a station number appears more than once for a certain operator, assign a new index number to the sum of the coefficients referring to that station. Enter the new index number as an element of  $[A]$  which is also included in Table 2.



- f. Repeat procedures (d) and (e) until the matrix is filled with the index numbers. In the input format to the computer program, only non-zero elements are included where the index numbers are identified with proper row and column numbers.

INDEX NUMBERS	MATRIX ELEMENTS	INDEX NUMBERS	MATRIX ELEMENTS	INDEX NUMBERS	MATRIX ELEMENTS	INDEX NUMBERS	MATRIX ELEMENTS
(1)	1	(19)	$\nu \lambda^2 D_\phi / D_\phi$	(36)	-(12)	(57)	2x(22)
(2)	$-4\lambda^2(D_x \lambda^2 + H)/D$	(20)	$-2[1 + (\nu \lambda^2 D_\phi / D_\phi)]$	(37)	-(17)	(58)	2x(24)
(3)	$D_x \lambda^4 / D$	(21)	$-2\lambda^2(a\Delta\phi)^2 R$	(38)	-(29)	(59)	2x(25)
(4)	$-4(H\lambda^2 + D_\phi)/D$	(22)	$\lambda^2(a\Delta\phi)^2 R$	(39)	2x(2)	(60)	2x(26)
(5)	$2H\lambda^2/D$	(23)	$(6K_x \lambda^4/K) + 8P\lambda^2 + (6K_\phi/K)$	(40)	2x(3)	(61)	2x(27)
(6)	$D_\phi/D$	(24)	$-4\lambda^2[(K_x \lambda^2/K) + P]$	(41)	2x(4)	(62)	2x(28)
(7)	$2\lambda^2(a\Delta\phi)^2/aD$	(25)	$K_x \lambda^4/K$	(42)	2x(5)	(63)	2x(1)
(8)	$(6D_x \lambda^4 + 8H\lambda^2 + 6D_\phi)/D$	(26)	$-4[(K_\phi/K) + P\lambda^2]$	(43)	2x(6)	(64)	-(30)
(9)	$-\lambda^2(a\Delta\phi)^2/aD$	(27)	$2P\lambda^2$	(44)	(3)+(8)	(65)	2x(9)
(10)	$-2\bar{K}_x(a\Delta\phi)^2/D_x \lambda^3$	(28)	$K_\phi/K$	(45)	(6)+(8)	(66)	-(1)
(11)	$\alpha/\lambda^2 D_x$	(29)	-2	(46)	(3)+(6)+(8)	(67)	-(11)
(12)	$-2[1 + (\alpha/\lambda^2 D_x)]$	(30)	$D(1-\nu)/a$	(47)	2x(11)	(68)	$2\bar{K}_1(a\Delta\phi)^2/DA(1-\nu)$
(14)	$\nu D_x / \lambda^2 D_x$	(31)	$K(a\Delta\phi)^2 R/K_x \lambda^2$	(48)	-2x(11)	(69)	-(18)
(15)	$-2[1 + (\nu D_x / \lambda^2 D_x)]$	(32)	-4	(49)	4x(5)	(70)	2x(19)
(16)	$-2\lambda \bar{K}_\phi(a\Delta\phi)^2/D_\phi$	(33)	$-\nu K/\lambda^2 K_x$	(50)	4x(27)	(71)	-2x(29)
(17)	$\beta \lambda^2/D_\phi$	(34)	$-4 + (\nu K/\lambda^2 K_x)$	(51)	2x(14)		
(18)	$-2[1 + (\beta \lambda^2/D_\phi)]$	(35)	$2K(a\Delta\phi)^2 R/K_x \lambda^2$	(52)	(25)+(23)		
				(53)	(23)+(28)		
				(54)	2x(17)		
				(55)	-2x(17)		
				(56)	(23)+(25)+(28)		

TABLE 2. TYPICAL INDEX TABLE OF THE FINITE DIFFERENCE PROGRAM



The interior mass matrix  $[M_1]$  is a square matrix of order  $mn$ . The diagonal terms are generated as follows:

$$(M_1)_{ii} = 1 + \frac{W_i + \rho g \left[ A_{s\phi_i} a \Delta \phi + A_{sx_i} \Delta x \right]}{mg \Delta x a \Delta \phi} \text{ for interior points} \quad (50)$$

$$(M_1)_{ii} = 1 \quad \text{for points on the boundaries} \quad (51)$$

where the subscript  $i$  indicates the station number and  $W_i$  is the concentrated weight attached to that station.

The off diagonal terms of  $M_1$  matrix appear only for a component connected to the quarter panel through more than one attachment point. All values of  $(M_1)_{i,j}$ ,  $i \neq j$  must be precalculated as input to the computer program. Considering the loaded I. U. segment (Figure 14) as an example, the inertia forces acting on the attachment points A and B due to

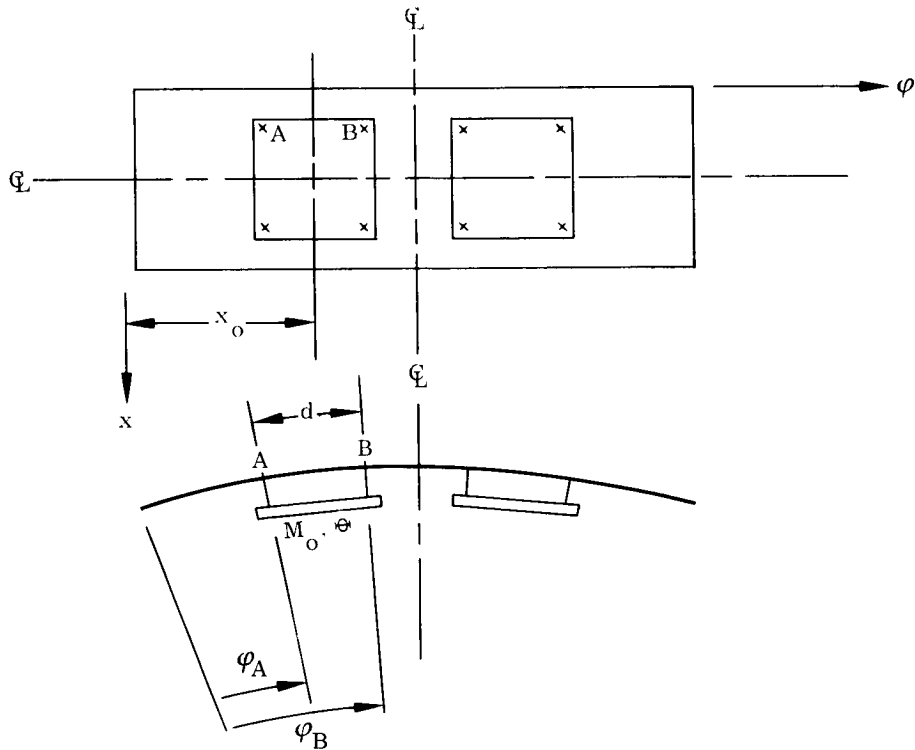


FIGURE 14. LOADED INSTRUMENT UNIT SEGMENT

a component with mass  $M_o$  and moment of inertia  $\phi$  with respect to the axis through centroid in  $x$ -direction may be expressed in the following forms.

$$F_A = \frac{M_o}{4} \omega^2 \left( \frac{w_A + w_B}{2} \right) - \frac{\phi}{2d} \omega^2 \frac{w_B - w_A}{d}$$

$$= \frac{\omega^2}{8} \left[ \left( M_o + \frac{4\phi}{d^2} \right) w_A + \left( M_o - \frac{4\phi}{d^2} \right) w_B \right] \quad (52)$$

$$F_B = \frac{\omega^2}{8} \left[ \left( M_o - \frac{4\phi}{d^2} \right) w_A + \left( M_o + \frac{4\phi}{d^2} \right) w_B \right] \quad (53)$$

where  $d$  is the distance between A and B in circumferential direction. If each attachment point coincides with a grid point, it follows that

$$\Delta(M_1)_{A,A} = \Delta(M_1)_{B,B} = \frac{1}{8\bar{m}\Delta x a \Delta\varphi} \left( M_0 + \frac{4\Theta}{d^2} \right) \quad (54)$$

$$(M_1)_{A,B} = (M_1)_{B,A} = \frac{1}{8\bar{m}\Delta x a \Delta\varphi} \left( M_0 - \frac{4\Theta}{d^2} \right) \quad (55)$$

The values computed from Equation (54) represent the concentrated weights attached to stations A and B. The values are to be added to the corresponding elements of Equation (50).

If the attachment points are off the grid points, similar approach can be used to evaluate the corresponding elements of  $M_1$ . Numerical example is given later in Appendix II.

The boundary mass matrix  $[M_2]$  is a rectangular matrix of size  $(2m + 2n + 1) \times (mn)$ . It is composed of the inertia terms for the masses attached to the edge of the panel which is reflected in the transverse shear equations. Referring to the grid pattern and number systems of Figure 13, the elements of  $M_2$  are defined as follows:

$$(M_2)_{i,j} = 0 \text{ except} \quad (56)$$

$$(M_2)_{i,i} = - \frac{2D \left[ (1/2)\rho g (A_{s\varphi_i} a \Delta\varphi + A_{sx_i} \Delta x) + W_{x_i} \right]}{D_{x_i} \lambda^3 (a \Delta\varphi)^2 \bar{m}g} \quad i = 2, 3, \dots, n \quad (57)$$

$$(M_2)_{n+1,1} = \frac{2 \left\{ \frac{\rho g}{4} [A_{s\varphi_1} a \Delta\varphi + A_{sx_1} \Delta x] + W_{x_1} \right\}}{\lambda (1-\nu) (a \Delta\varphi)^2 \bar{m}g} \quad (58)$$

$$(M_2)_{2n+1+i,j} = - \frac{2D \lambda \left\{ (1/2)\rho g (A_{sx_j} \Delta x + A_{s\varphi_j} a \Delta\varphi) + W_{\varphi_j} \right\}}{D_{\varphi_j} (a \Delta\varphi)^2 \bar{m}g} \quad \begin{matrix} i = 2, \dots, m \\ j = (i-1)n+1 \end{matrix} \quad (59)$$

### C. THE EIGENVALUE-EIGENVECTOR PROBLEM

In the previous section, a procedure is described to generate the index numbers representing the elements of  $[A]$ ,  $[M_1]$  and  $[M_2]$ . The index number pattern is used as the input to the subject computer program. Based on the input, the computer program generates the matrices according to the local shell and stiffener data which is also part of the input information. The final matrix equation generated by the computer is of the following form:

$$\begin{bmatrix} A_{11} & A_{12} & A_{13} \\ A_{21} & A_{22} & 0 \\ A_{31} & A_{32} & A_{33} \end{bmatrix} \begin{Bmatrix} w_a \\ w_b \\ \phi \end{Bmatrix} - \Omega \begin{Bmatrix} M_1 & w_a \\ M_2 & w_a \\ 0 \end{Bmatrix} = 0 \quad (60)$$

or

$$A_{11} w_a + A_{12} w_b + A_{13} \phi - \Omega M_1 w_a = 0 \quad (61)$$

$$A_{21} w_a + A_{22} w_b - \Omega M_2 w_a = 0 \quad (62)$$

$$A_{31} w_a + A_{32} w_b + A_{33} \phi = 0 \quad (63)$$

where

$A_{11}$  = square matrix (mn) x (mn)

$A_{12}$  = rectangular matrix (mn) x (2m + 2n + 1)

$A_{13}$  = rectangular matrix (mn) x (mn + m + n)

$A_{21}$  = rectangular matrix (2m + 2n + 1) x (mn)

$A_{22}$  = square matrix (2m + 2n + 1) x (2m + 2n + 1)

$A_{31}$  = rectangular matrix (mn + m + n) x (mn)

$A_{32}$  = rectangular matrix (mn + m + n) x (2m + 2n + 1)

$A_{33}$  = square matrix (mn + m + n) x (mn + m + n)

Equation (60) represents an eigenvalue-eigenvector problem.  $\Omega$  is the eigenvalue and  $w_a$  the corresponding eigenvectors. Specifically, matrix Equation (61) represents the dynamic equations of equilibrium for the interior and boundary points of the system. Equation (62) represents the boundary conditions involving bending moments and transverse shears. Equation (63) represents the compatibility equations and the boundary conditions involving the in-plane forces. The column of  $w_b$  pertains to the deflections at the exterior points of the system.  $\phi$  represents the stress function at all stations concerned.

Equations (62) and (63) are used to eliminate  $w_b$  and  $\phi$  from Equation (61). The end result is a matrix equation suitable for eigenvalue-eigenvector determination by the "MITERS" routine which forms part of the computer program.

From Equation (62),

$$w_b = A_{22}^{-1} (\Omega M_2 - A_{21}) w_a \quad (64)$$

From Equation (63),

$$\phi = -A_{33}^{-1} (A_{31} w_a + A_{32} w_b) \quad (65)$$

Substituting equation (64) into (65) yields:

$$\phi = -A_{33}^{-1} \left[ A_{31} + A_{32} A_{22}^{-1} (\Omega M_2 - A_{21}) \right] w_a \quad (66)$$

Substituting equations (64) and (66) into (61), the standard format suitable for the "MITERS" routine is obtained:

$$[\bar{A}] \{w_a\} - \frac{1}{\Omega} \{w_a\} = 0 \quad (67)$$

where

$$[\bar{A}] = \left[ \begin{array}{c} A_{11} - A_{12} A_{22}^{-1} A_{21} - A_{13} A_{33}^{-1} (A_{31} - A_{32} A_{22}^{-1} A_{21}) \\ M_1 - (A_{12} - A_{13} A_{33}^{-1} A_{32}) A_{22}^{-1} M_2 \end{array} \right]^{-1} \quad (68)$$

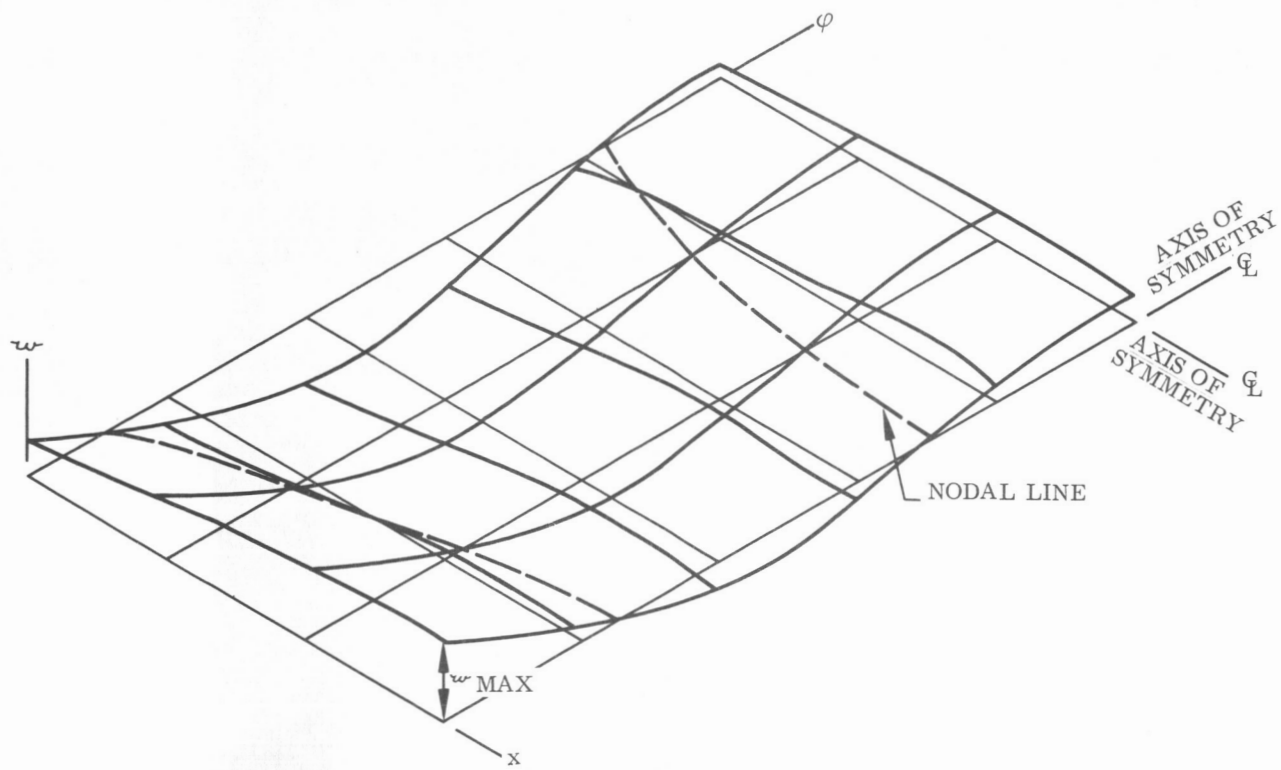
The computer program yields the eigenvalues and the eigenvectors starting with the largest value of  $\frac{1}{\Omega}$ . In other words, the lowest frequency and the corresponding modal data are generated first, to be followed by modal data corresponding to gradually increasing frequencies. After one eigenvalue and the corresponding mode shape are obtained, Equation (66) is used to compute the corresponding  $\phi$  values as partial output of the computer program.

The above writeup describes the general scheme used in mechanizing the computer program. Results obtained so far have been promising as compared to the test data. The complete program, the work instructions and the sample data are given in Appendix II.

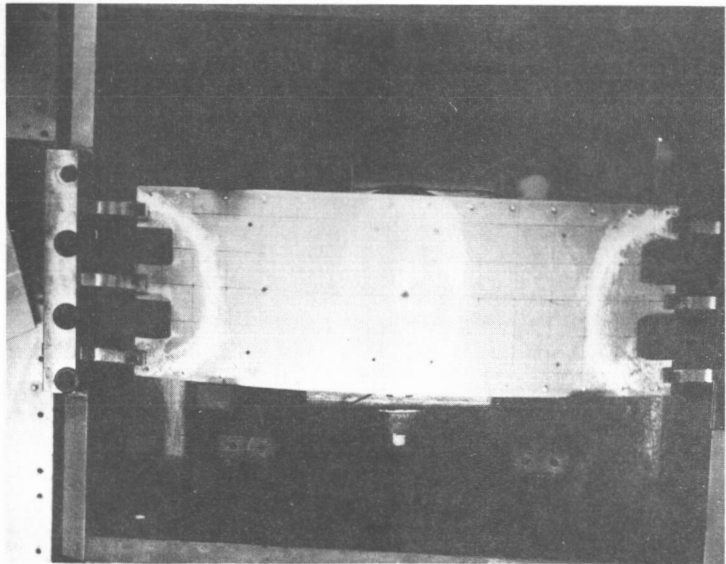
#### D. SEGMENTED INSTRUMENT UNIT DATA

Four (4) sets of analytical data were acquired from the finite difference computer program for the unloaded and loaded Instrument Unit segments. The computed natural frequencies and corresponding mode shapes of the first five modes of the unloaded Instrument Unit scale model segment supported by stiff springs (supporting configuration II) are plotted in Figure 4 and Figures 15 through 18. Figures 19 through 23 give the analytical data of the unloaded I. U. scale model supported by soft springs (supporting configuration I). Modal data of loaded scale model supported by stiff springs, and loaded full scale Instrument Unit segment supported by soft springs are shown in Figures 24-28 and Figures 29-33 respectively. Figures 34 and 35 are the plots of in-plane stress distribution and magnitude and direction of the top surface principal stresses corresponding to the mode given in Figure 4. The in-plane stress distribution corresponding to the first mode of loaded full scale I. U. segment is shown in Figure 36.

Testing was undertaken on the segmented I. U. scale model supported by stiff springs. Test data compared favorably with the analytical data for the first two modes in both unloaded and loaded configurations. The experimental frequencies and corresponding nodal lines are shown in the bottom portion of figures 4, 15, and 24. The comparison between the theoretical predicted frequencies and the test frequencies corresponding to the minimum impedance points for the loaded I. U. segment is given in Figure 7.



(a) ANALYTICAL DATA  $f = 289.7$  CPS



(b) EXPERIMENTAL DATA  $f = 264$  CPS

FIGURE 15. SECOND MODAL DATA OF SEGMENTED INSTRUMENT UNIT SCALE MODEL, SUPPORT CONFIGURATION (II)

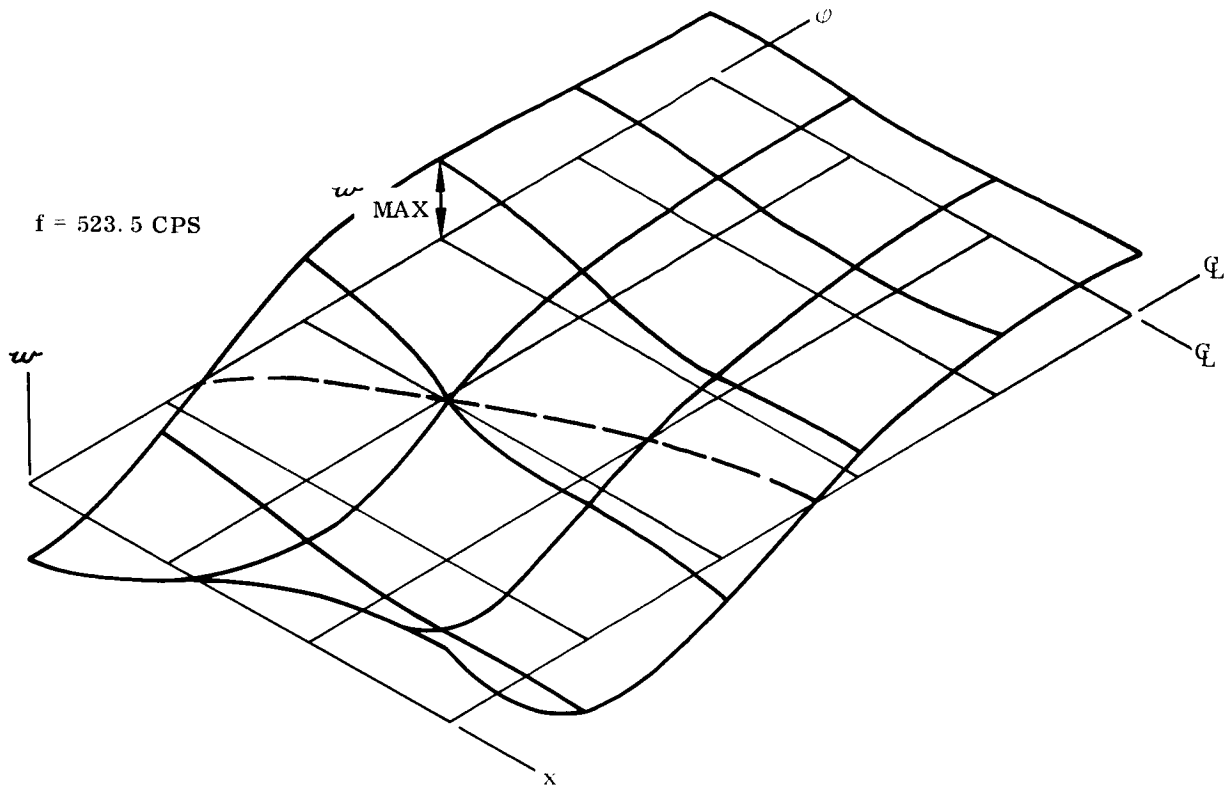


FIGURE 16. THIRD MODAL DATA OF SEGMENTED INSTRUMENT UNIT SCALE MODEL, SUPPORT CONFIGURATION (II)

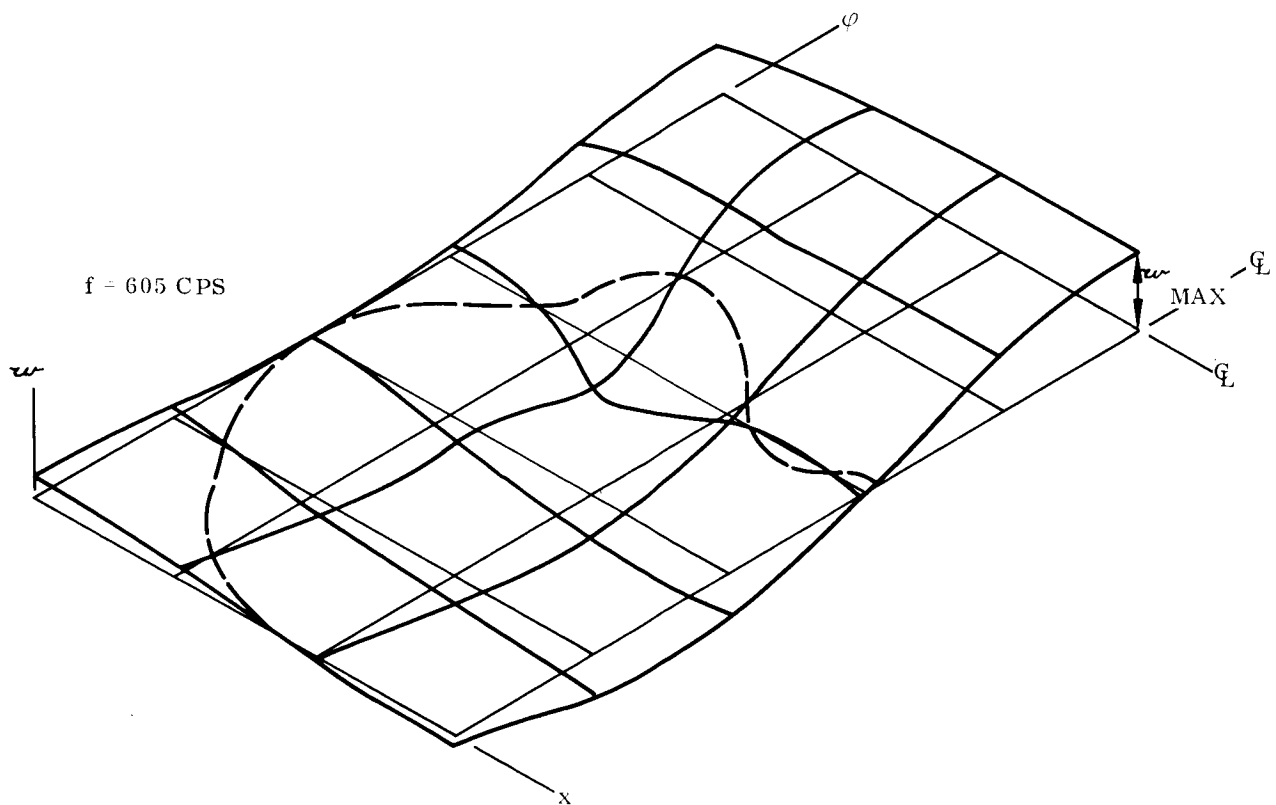


FIGURE 17. FOURTH MODAL DATA OF SEGMENTED INSTRUMENT UNIT SCALE MODEL, SUPPORT CONFIGURATION (II)

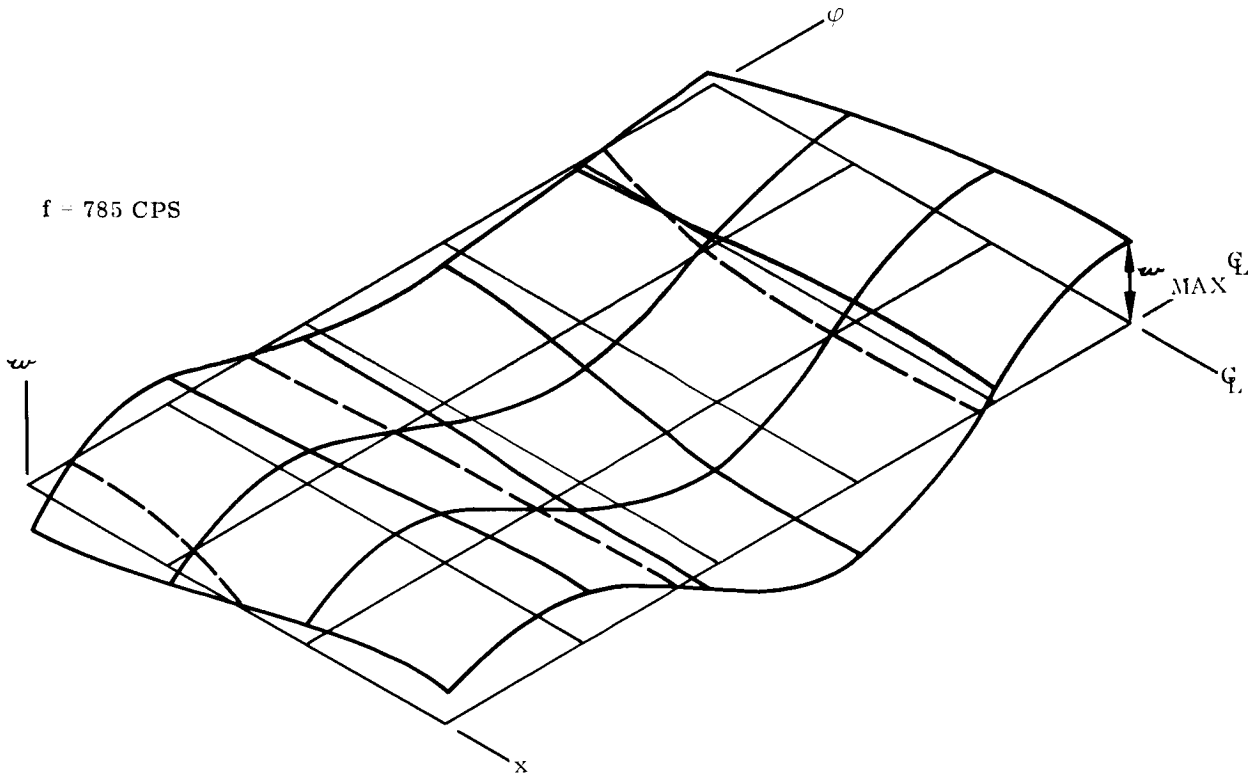


FIGURE 18. FIFTH MODAL DATA OF SEGMENTED INSTRUMENT UNIT SCALE MODEL, SUPPORT CONFIGURATION (II)

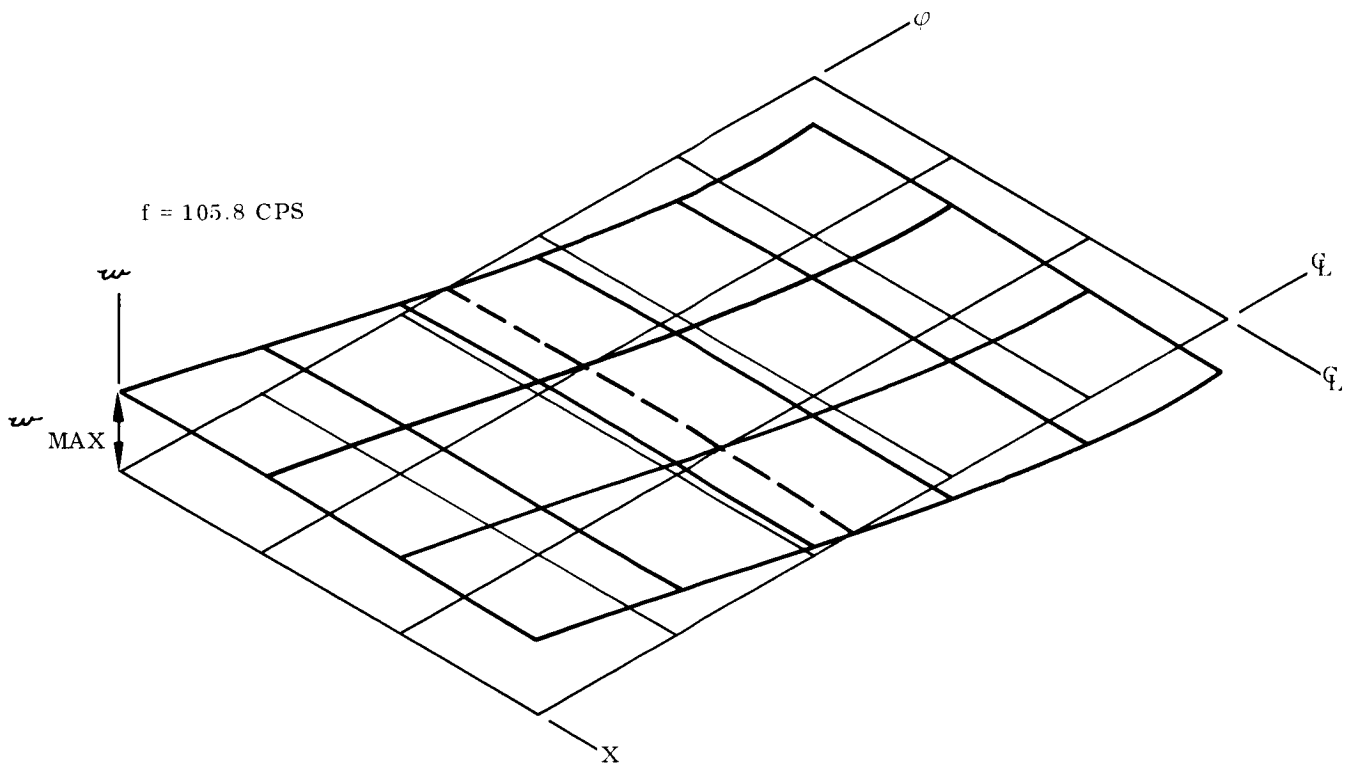


FIGURE 19. FIRST MODAL DATA OF SEGMENTED INSTRUMENT UNIT SCALE MODEL, SUPPORT CONFIGURATION (I)

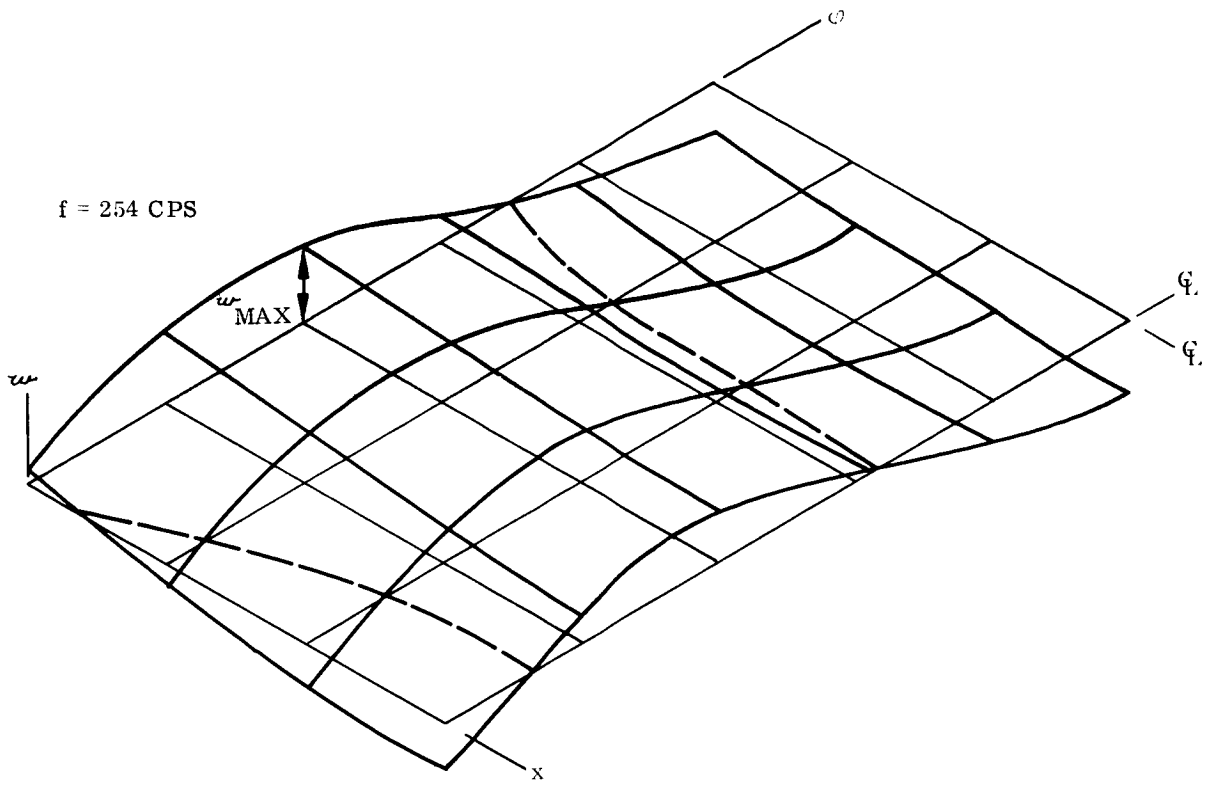


FIGURE 20. SECOND MODAL DATA OF SEGMENTED INSTRUMENT UNIT SCALE MODEL, SUPPORT CONFIGURATION (I)

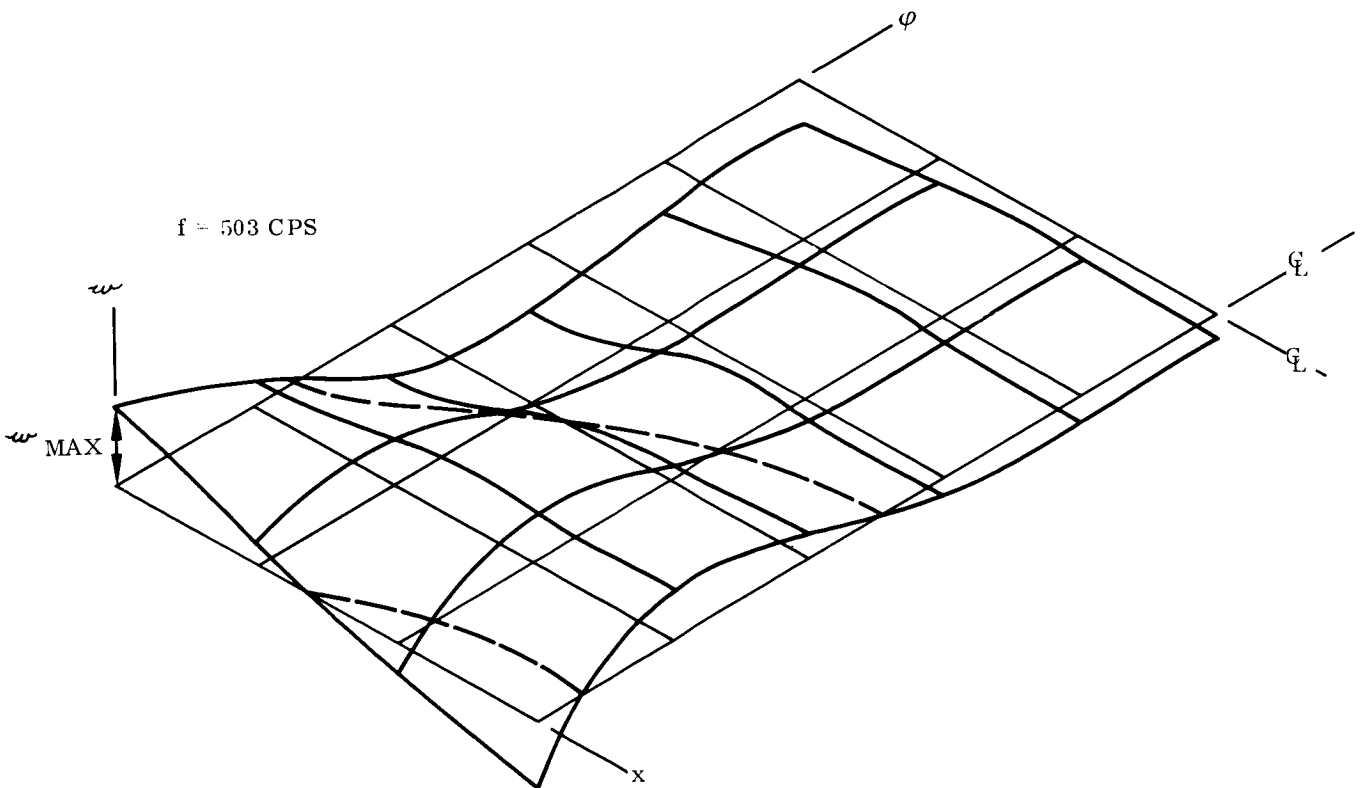


FIGURE 21. THIRD MODAL DATA OF SEGMENTED INSTRUMENT UNIT SCALE MODEL, SUPPORT CONFIGURATION (I)



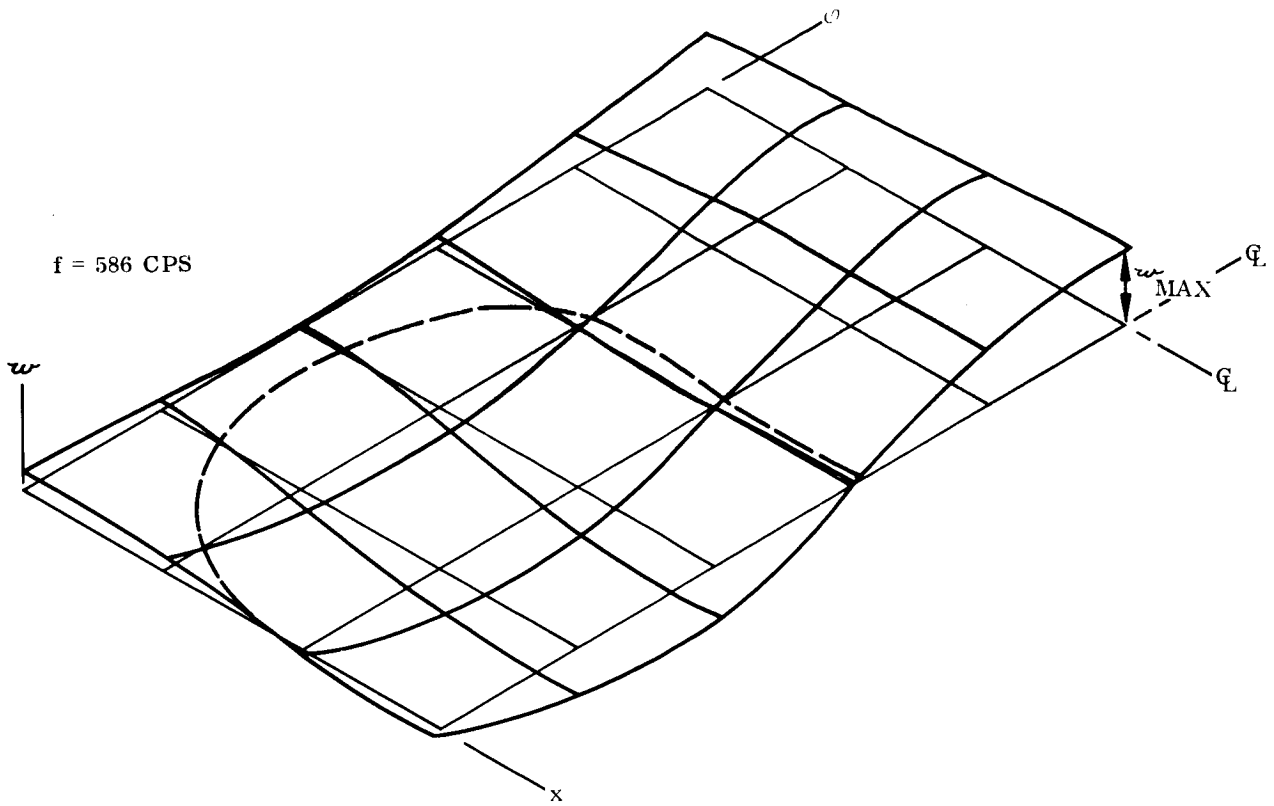


FIGURE 22. FOURTH MODAL DATA OF SEGMENTED INSTRUMENT UNIT SCALE MODEL, SUPPORT CONFIGURATION (I)

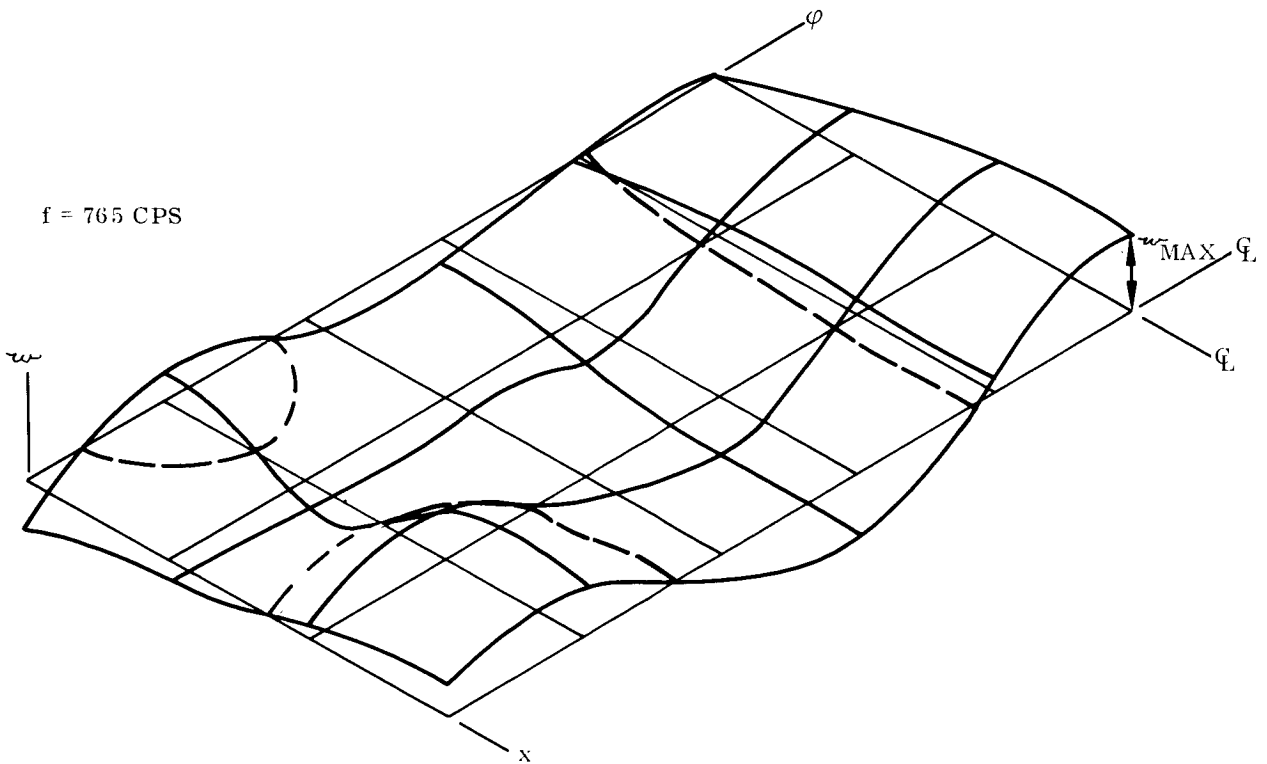
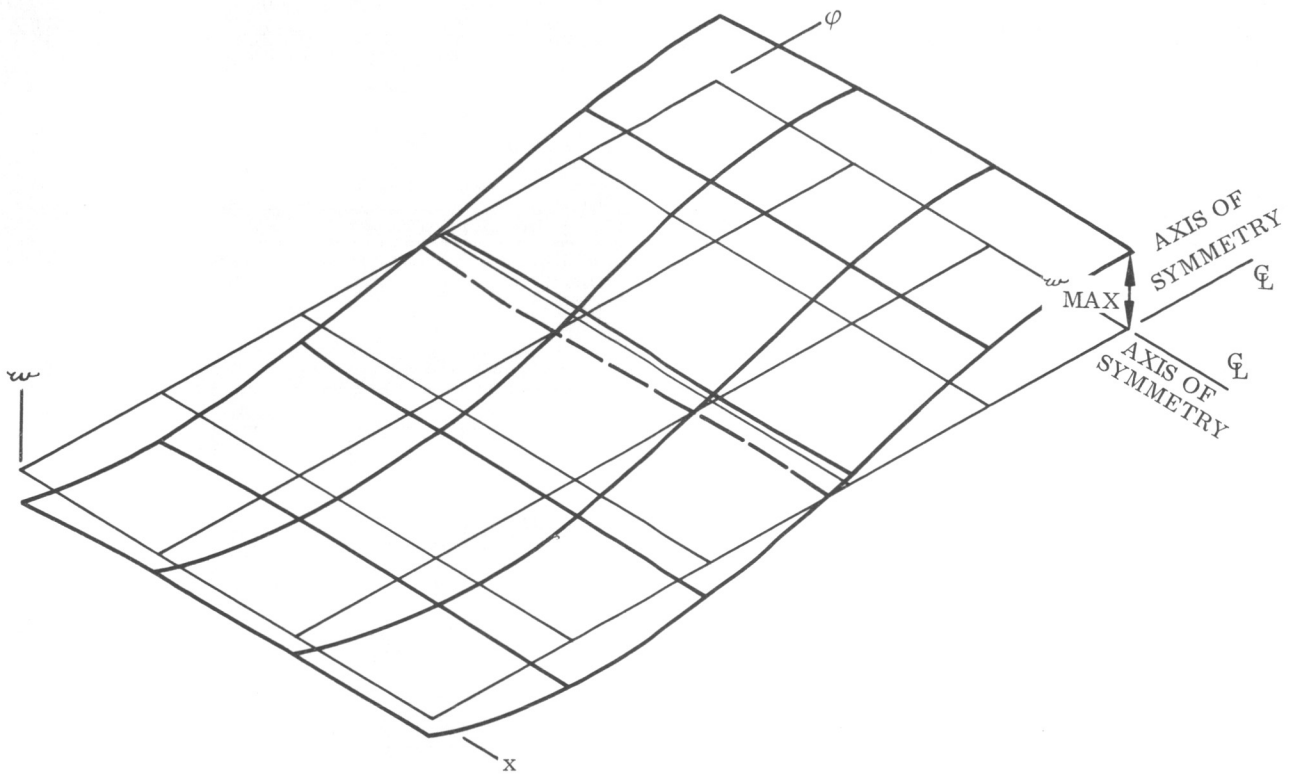
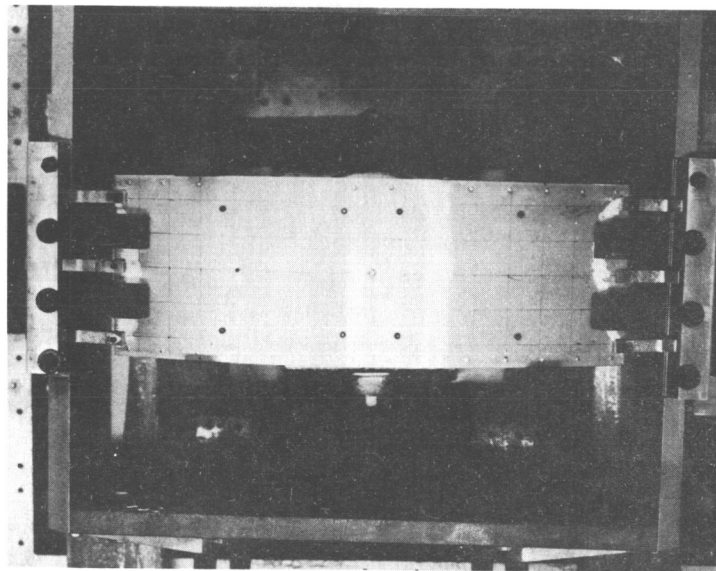


FIGURE 23. FIFTH MODAL DATA OF SEGMENTED INSTRUMENT UNIT SCALE MODEL, SUPPORT CONFIGURATION (I)



(a) ANALYTICAL DATA  $f = 172.8$  CPS



(b) EXPERIMENTAL DATA  $f = 172$  CPS

FIGURE 24. FIRST MODAL DATA OF SEGMENTED INSTRUMENT UNIT SCALE MODEL WITH SIMULATED COMPONENTS ATTACHED, SUPPORT CONFIGURATION (II)

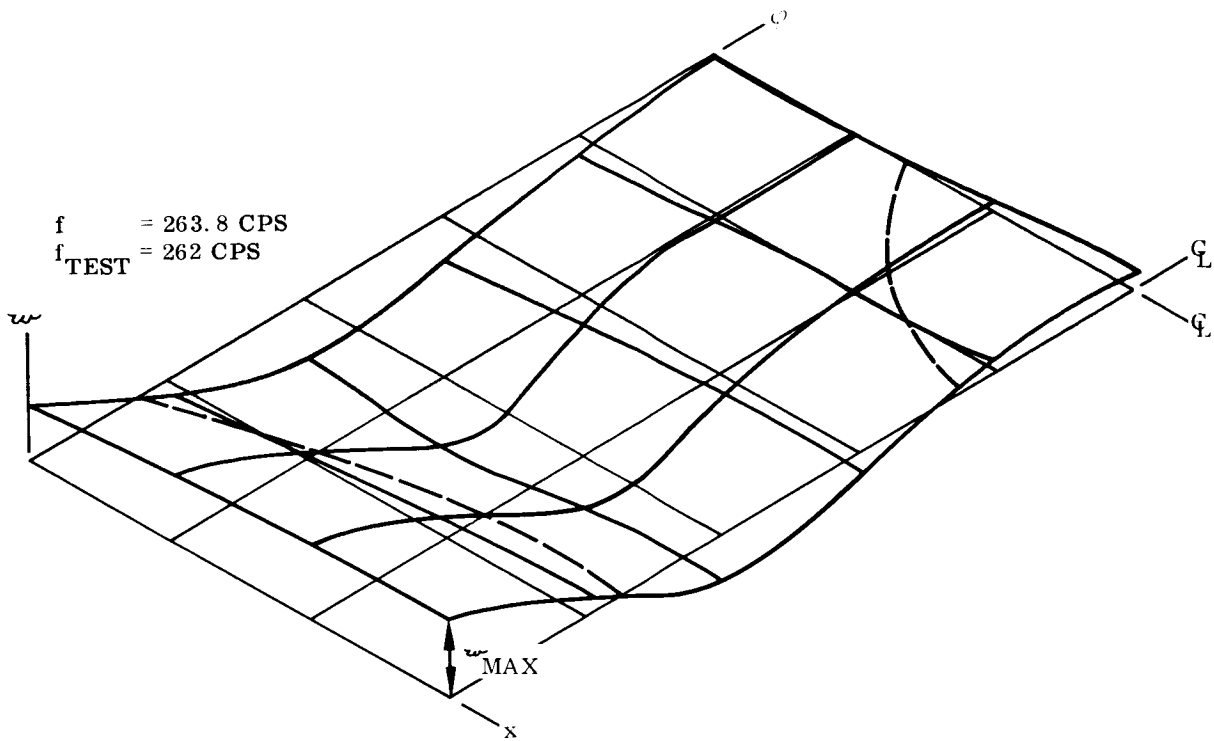


FIGURE 25. SECOND MODAL DATA OF SEGMENTED INSTRUMENT UNIT SCALE MODEL WITH SIMULATED COMPONENTS ATTACHED, SUPPORT CONFIGURATION (II)

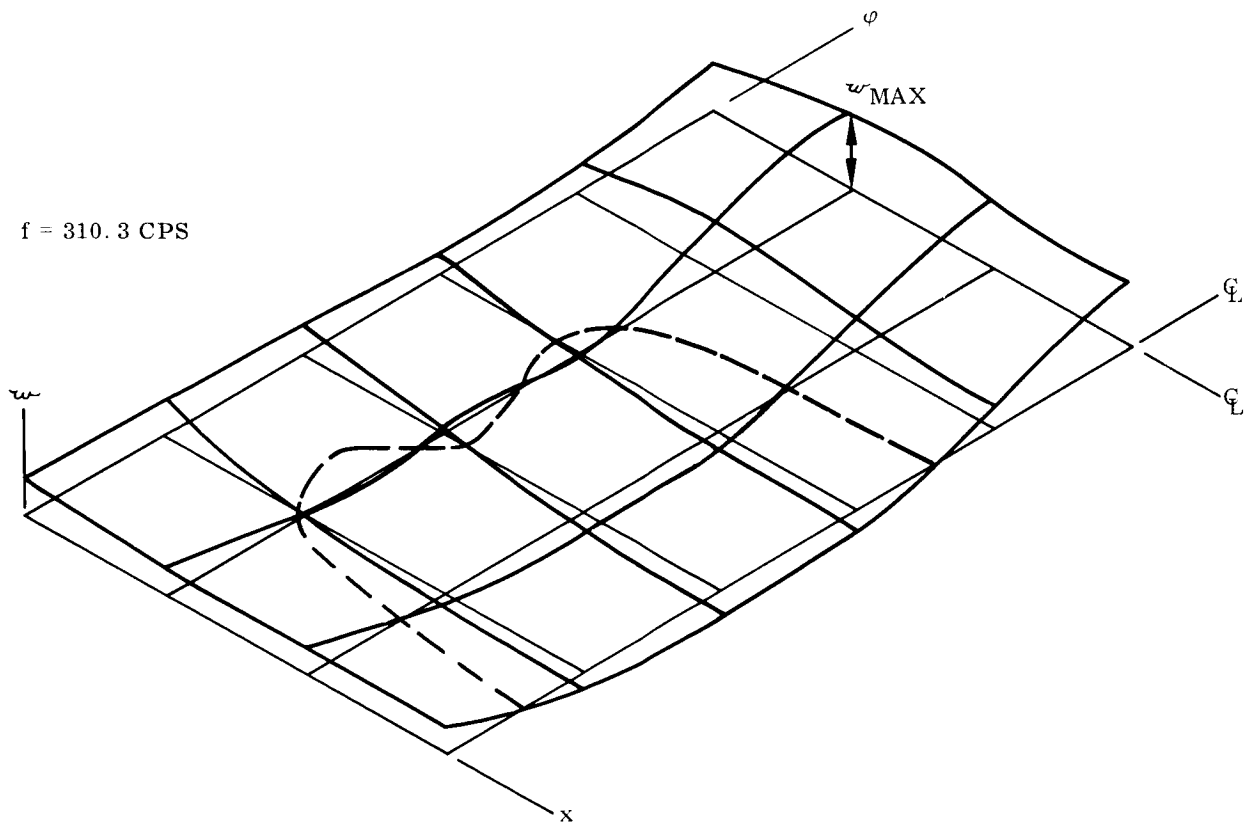


FIGURE 26. THIRD MODAL DATA OF SEGMENTED INSTRUMENT UNIT SCALE MODEL WITH SIMULATED COMPONENTS ATTACHED, SUPPORT CONFIGURATION (II)

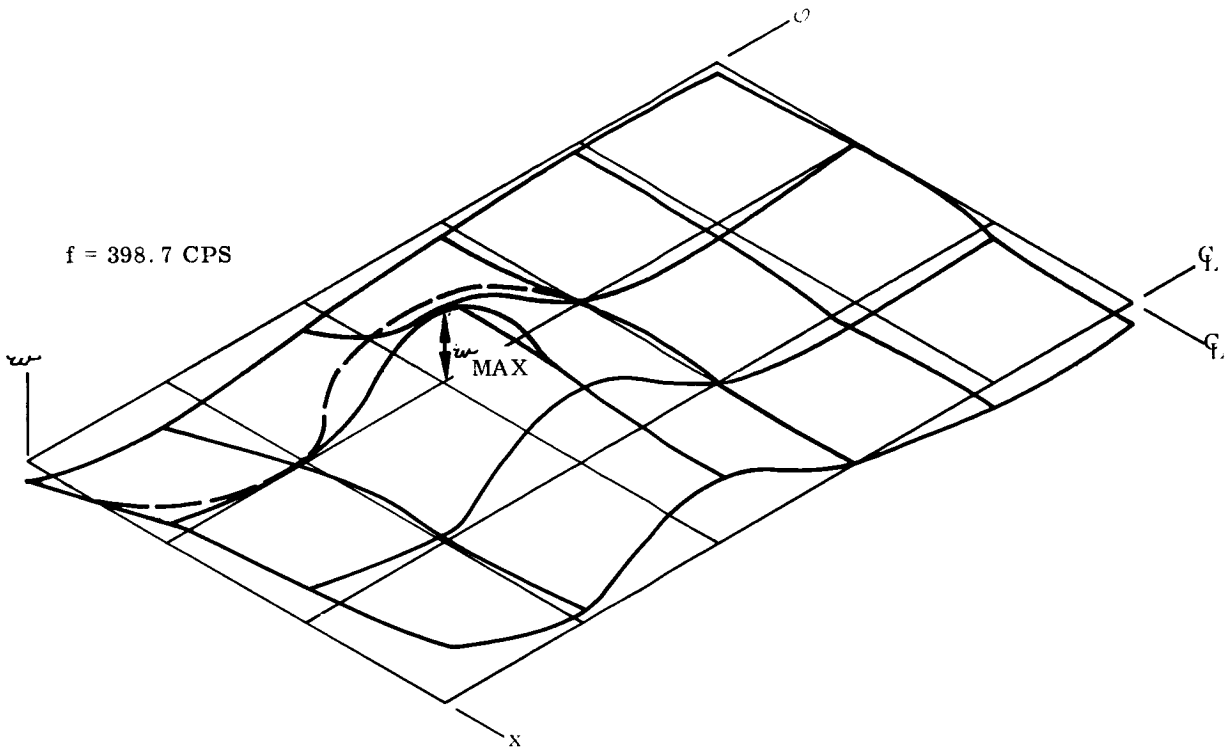


FIGURE 27. FOURTH MODAL DATA OF SEGMENTED INSTRUMENT UNIT SCALE MODEL WITH SIMULATED COMPONENTS ATTACHED, SUPPORT CONFIGURATION (II)

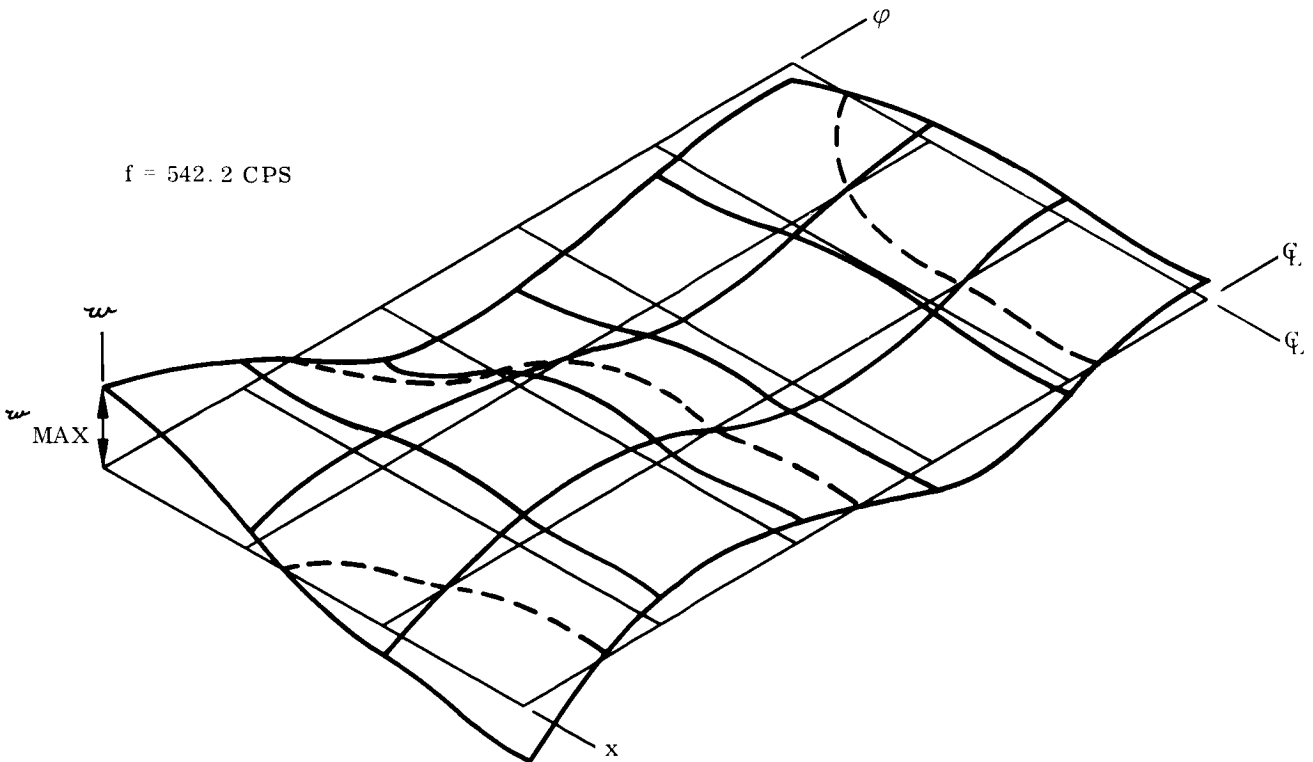


FIGURE 28. FIFTH MODAL DATA OF SEGMENTED INSTRUMENT UNIT SCALE MODEL WITH SIMULATED COMPONENTS ATTACHED, SUPPORT CONFIGURATION (II)

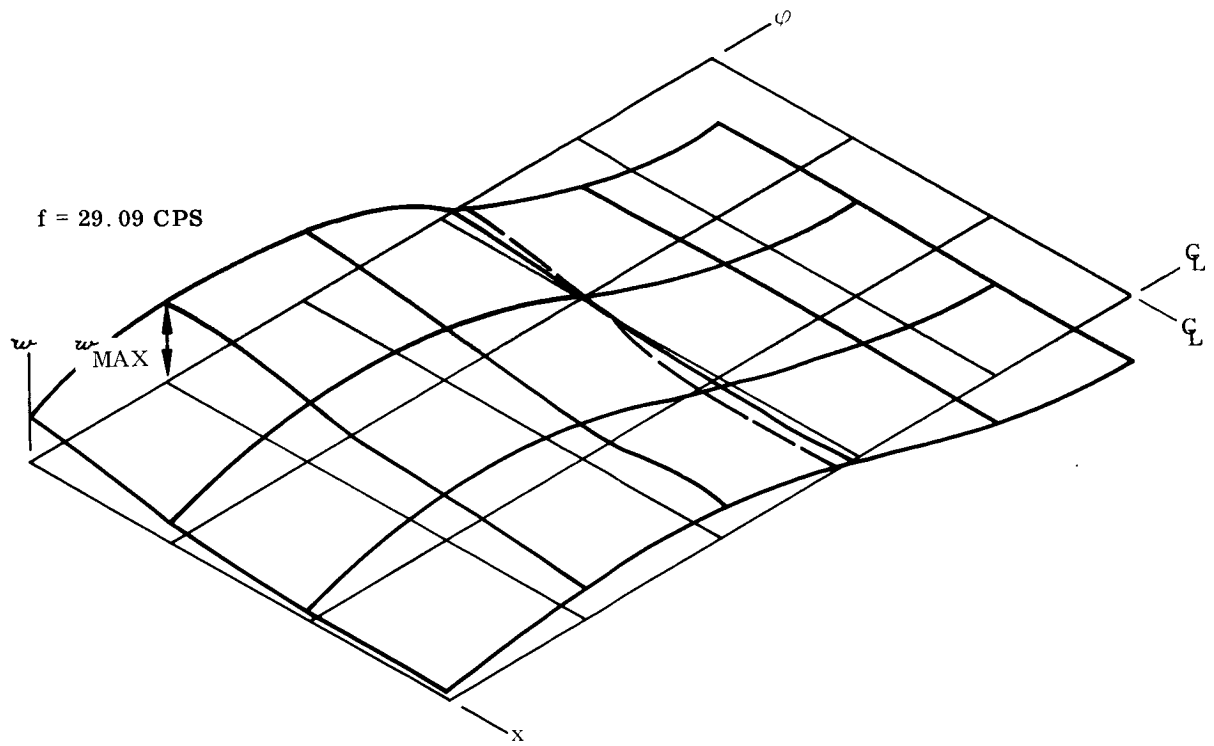


FIGURE 29. FIRST MODAL DATA OF INSTRUMENT UNIT SEGMENT WITH COMPONENTS ATTACHED, SUPPORT CONFIGURATION (I)

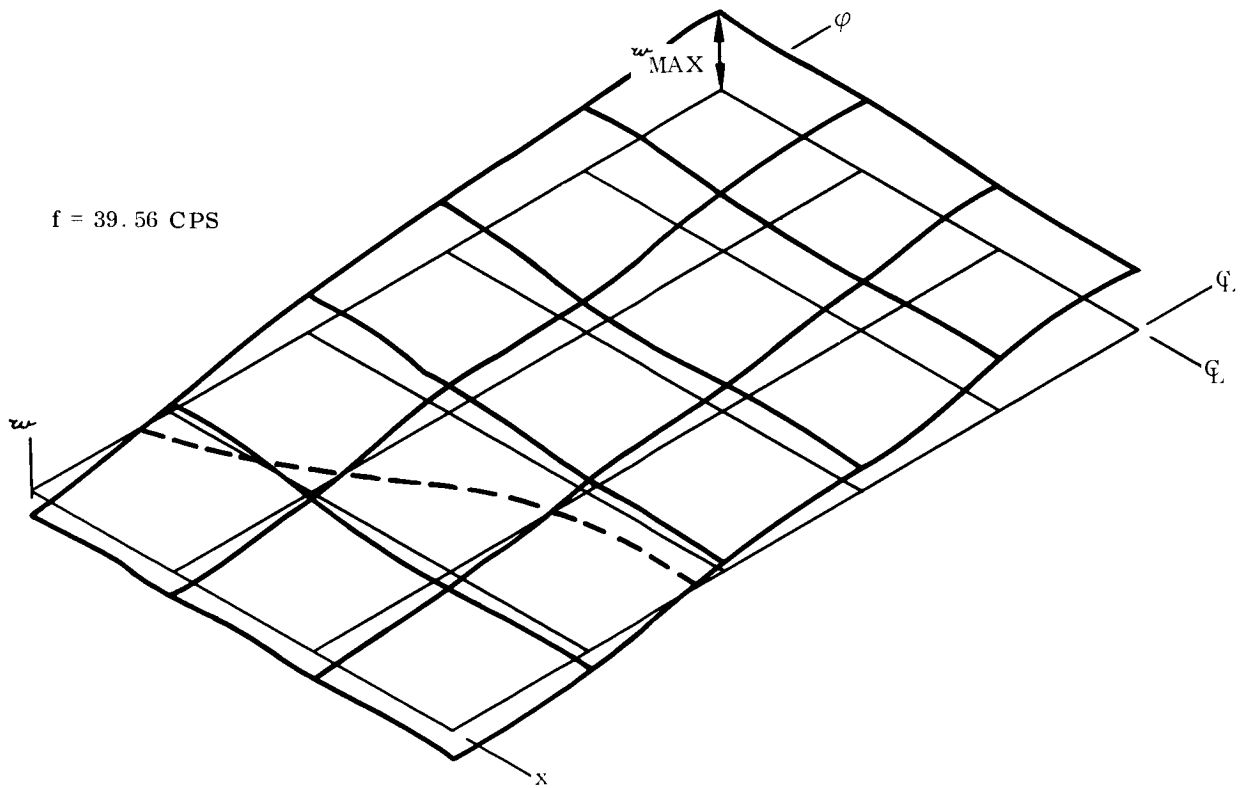


FIGURE 30. SECOND MODAL DATA OF INSTRUMENT UNIT SEGMENT WITH COMPONENTS ATTACHED, SUPPORT CONFIGURATION (I)

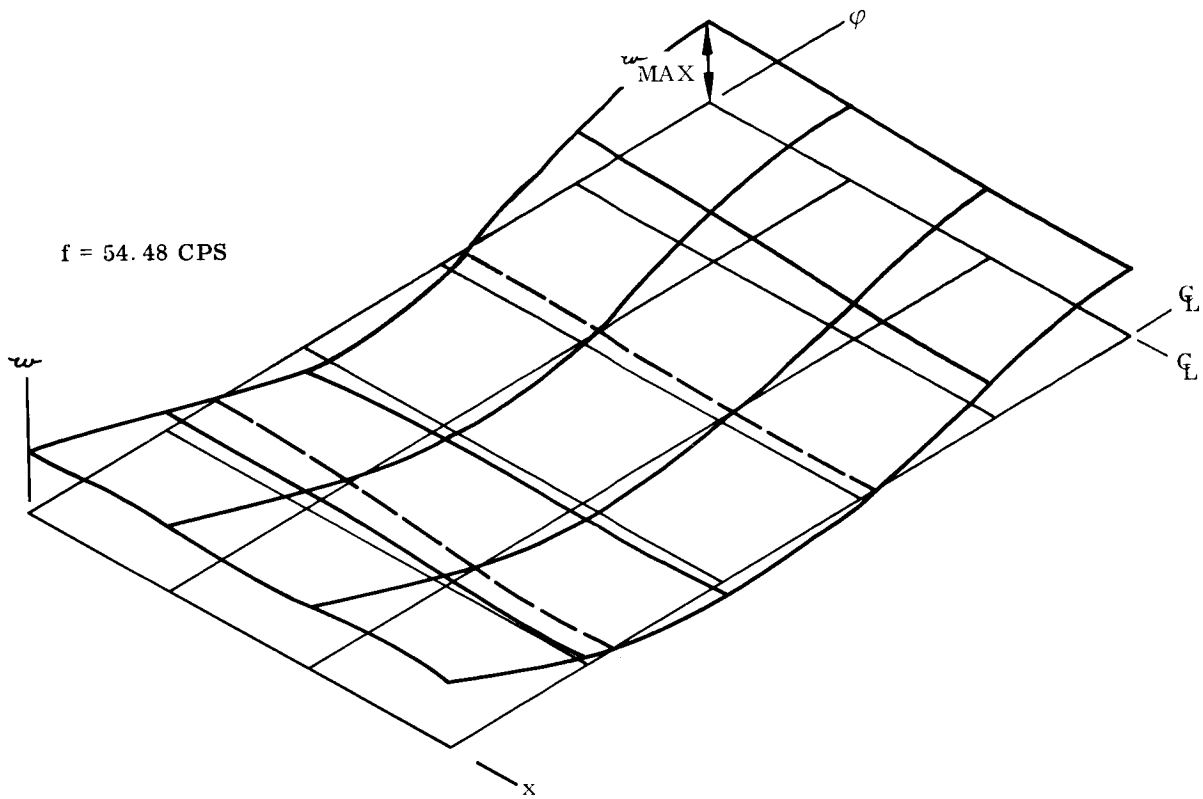


FIGURE 31. THIRD MODAL DATA OF INSTRUMENT UNIT SEGMENT WITH COMPONENTS ATTACHED. SUPPORT CONFIGURATION (I)

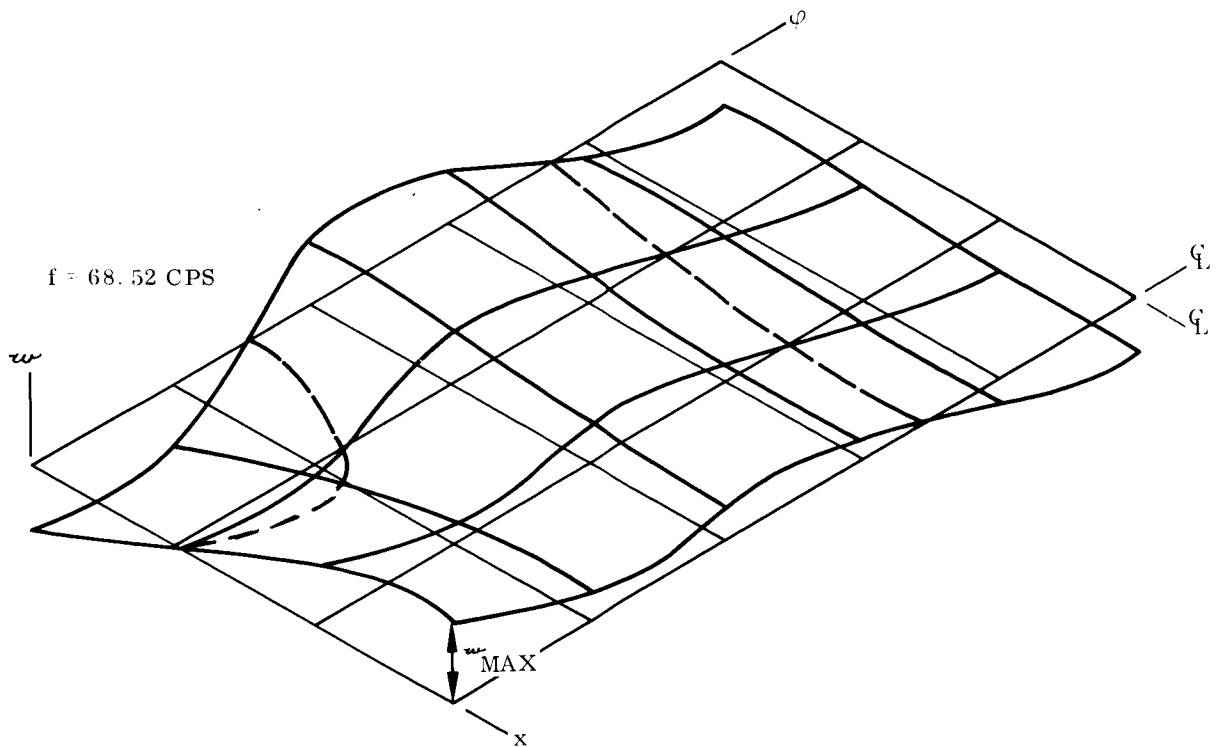


FIGURE 32. FOURTH MODAL DATA OF INSTRUMENT UNIT SEGMENT WITH COMPONENTS ATTACHED, SUPPORT CONFIGURATION (I)

f = 93.92 CPS

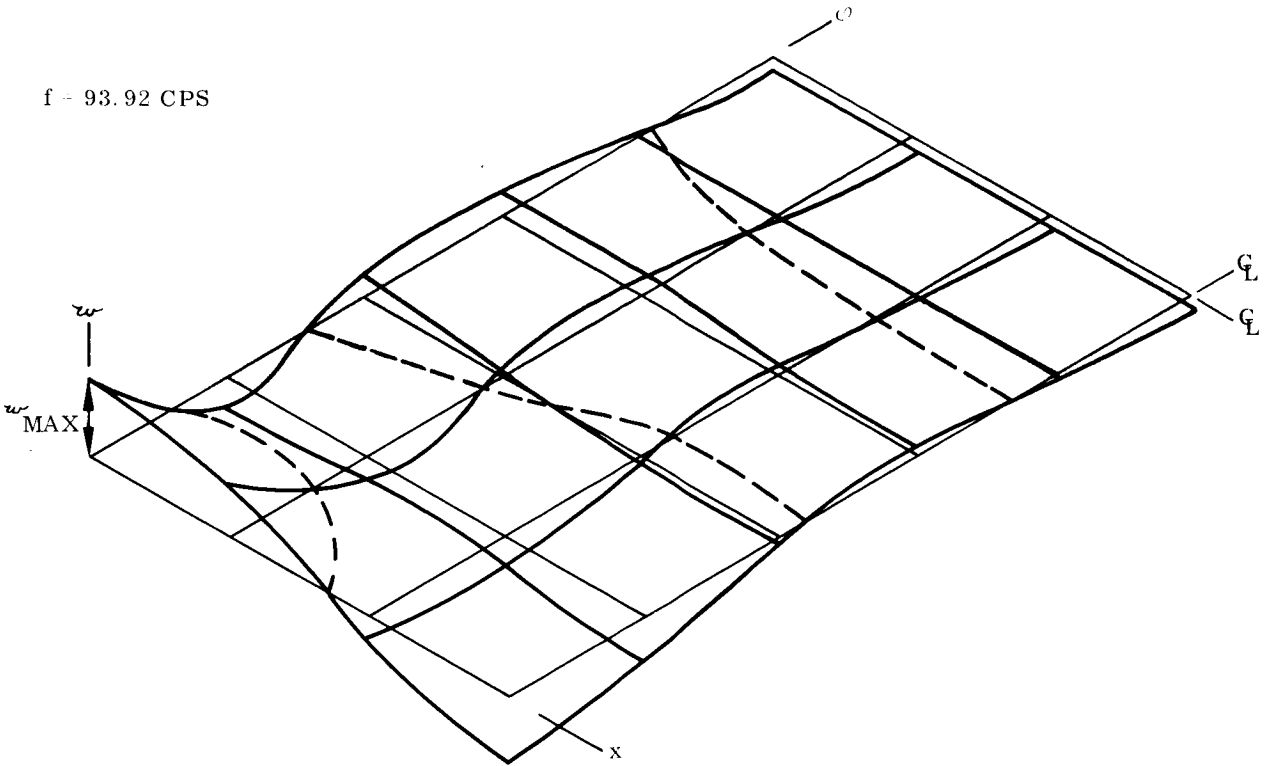


FIGURE 33. FIFTH MODAL DATA OF INSTRUMENT UNIT SEGMENT WITH COMPONENTS ATTACHED, SUPPORT CONFIGURATION (I)

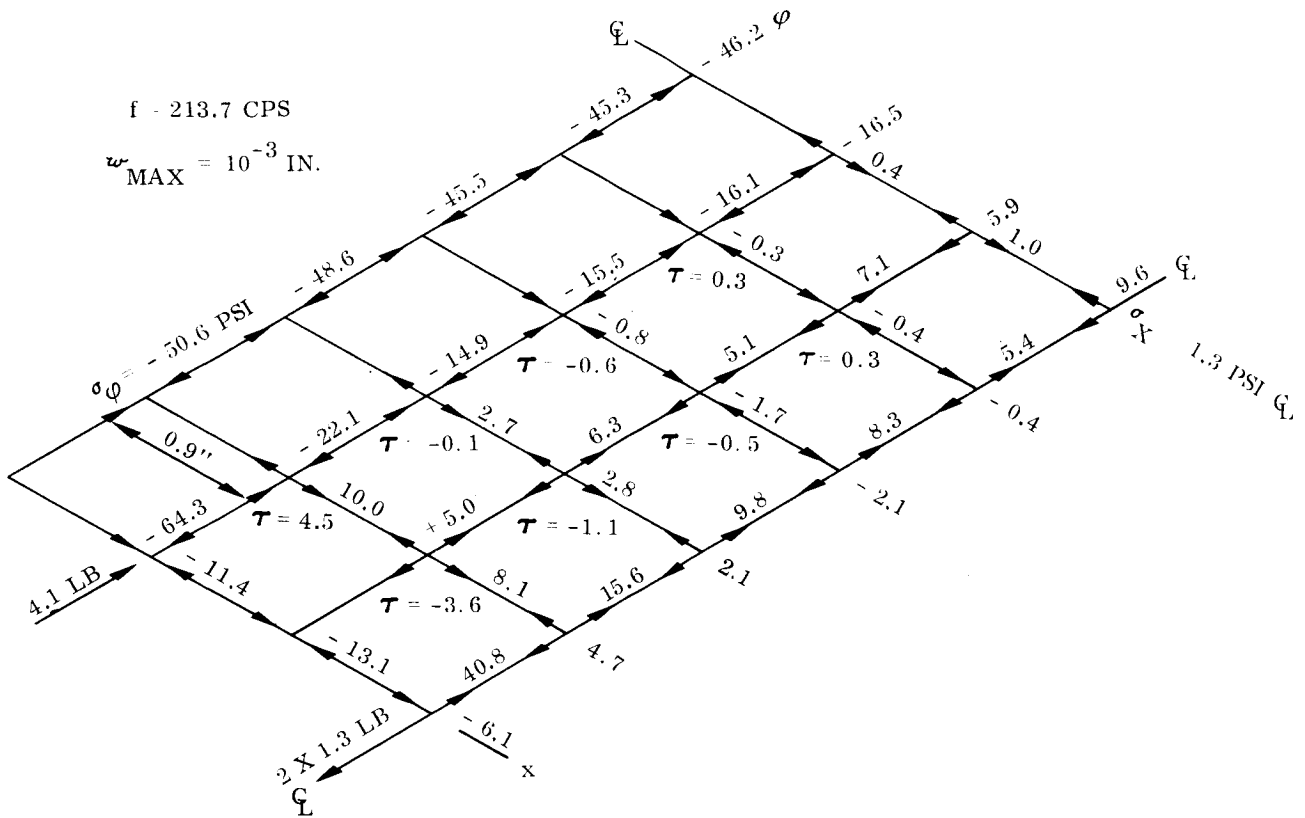


FIGURE 34. IN-PLANE STRESS DISTRIBUTION CORRESPONDING TO THE FIRST MODE OF SEGMENTED INSTRUMENT UNIT SCALE MODEL, SUPPORT CONFIGURATION (II)

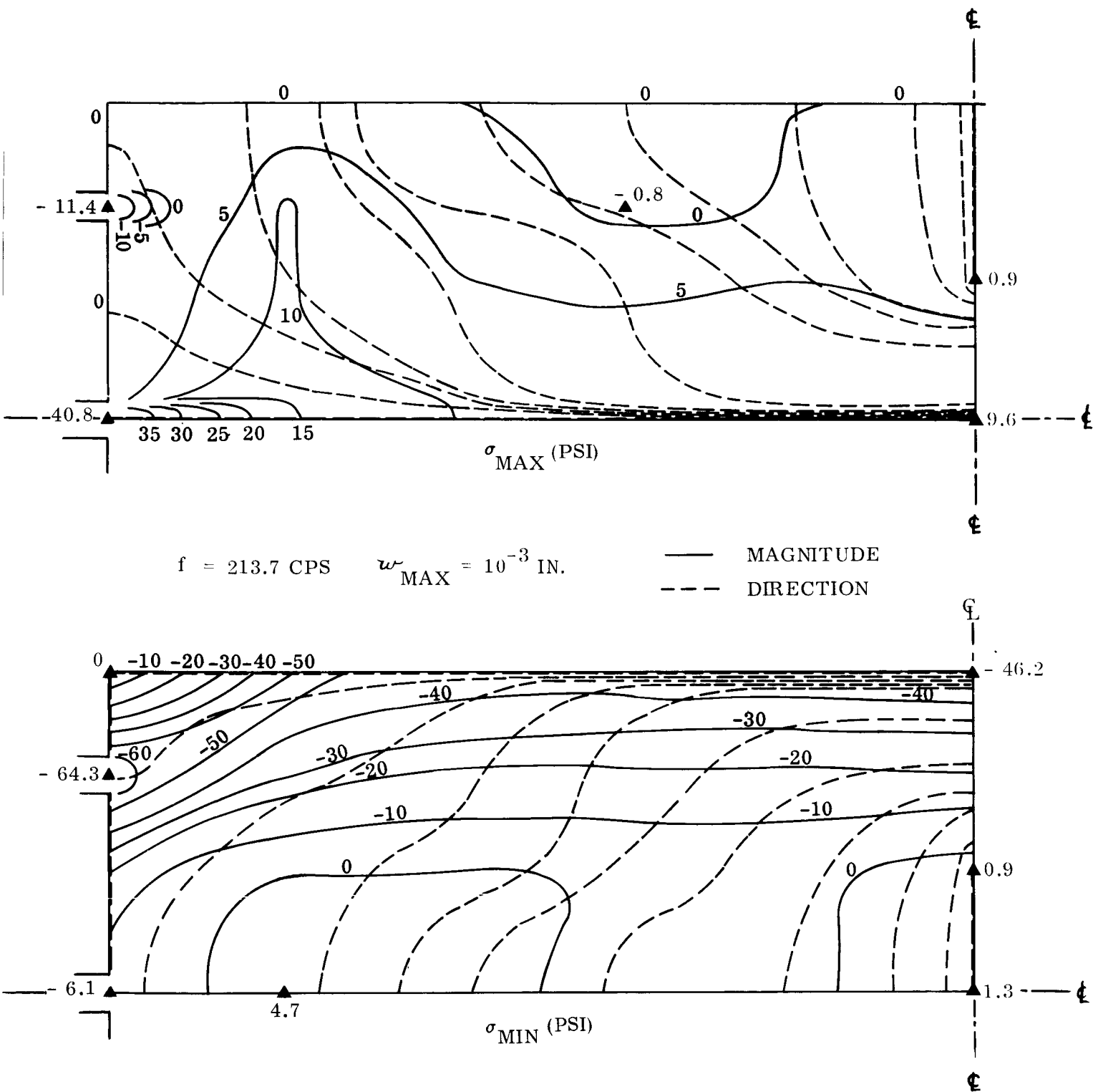


FIGURE 35. MAGNITUDE AND DIRECTION OF TOP SURFACE PRINCIPAL STRESSES CORRESPONDING TO THE FIRST MODE OF SEGMENTED INSTRUMENT UNIT SCALE MODEL. SUPPORT CONFIGURATION (II)



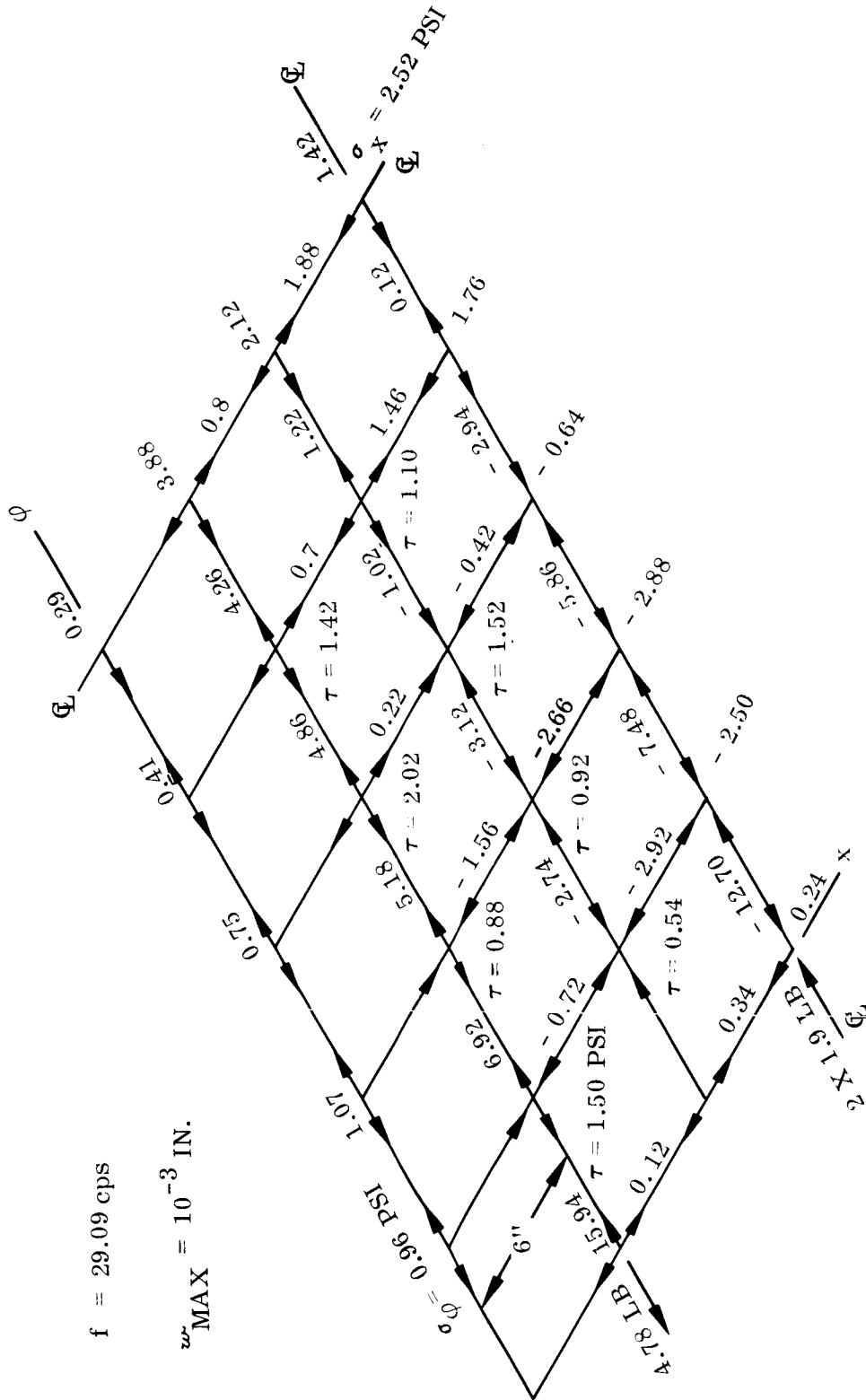


FIGURE 36. IN-PLANE STRESS DISTRIBUTION CORRESPONDING TO THE FIRST MODE OF INSTRUMENT UNIT SEGMENT WITH COMPONENTS ATTACHED. SUPPORT CONFIGURATION (I)

## APPENDIX II

### USER INFORMATION OF THE FINITE DIFFERENCE COMPUTER PROGRAM

The program gives the analytical predictions of natural vibration modes and frequencies of a curved panel or a curved sandwiched panel with arbitrary boundary and supporting conditions. Coefficient matrix derived from the finite difference expression for equilibrium equations, compatibility equations, and boundary conditions is organized and read into the program as input data.

The complete program consists of three CHAIN's. The first chain is used to compute all necessary data and set up a table called "E table" - that has all the elements needed in the coefficient matrix A. The second chain is mechanized to transfer these elements to proper locations in the matrix and to reduce the matrix into an eigenmatrix. The third chain is used to compute the eigenvalues and eigenvectors, and the associated stress function.

### INPUT DATA

The program is set up for the grid pattern shown in Figure 13.

#### 1. Solid Shell

Symbols used in Analysis	Fortran Coding	Definitions
a	A	Radius of curvature (in)
b	B	Length of a panel in x-direction (in)
$\phi_0$	PHIO	Circumferential angle in $\phi$ -direction (rad)
$\nu$	PNU	Poisson's ratio
E	E	Young's modulus of elasticity (lb/in <sup>2</sup> )
$\rho$	RHO	Mass density per unit volume (lb-sec <sup>2</sup> /in <sup>4</sup> )
h	H	Shell thickness (in)
n	N	Number of grid points in $\phi$ -direction = 6
m	M	Number of grid points in x-direction = 4
	NMODE	Number of modes sought
	IOPT	= 1 for solid shell

Symbols used in Analysis	Fortran Coding	Definitions
$z_{sx}, z_{s\phi}$	SZX, SZP	Distances from c. g. of the stiffeners to the middle surface of the panel in x-direction and $\phi$ -direction, (in.) numbers of values to be read in are controlled by an index, NEXT; their locations in the array by J1, J2 and K2.
$A_{sx}, A_{s\phi}$	SAX, SAP	Cross sectional area of stiffener* in x-direction and $\phi$ -direction (in <sup>2</sup> ). Controlling indices are J1, J2, K2 and NEXT.
$I_{sx}, I_{s\phi}$	SIX, SIP	Moment of inertial of stiffener, * in x-direction and $\phi$ -direction respectively, about its own centroidal axis (in <sup>4</sup> ). Controlling indices are J1, J2, K2 and NEXT.
$\bar{K}_x$	BARKX	Spring constant of a point-support along the axis $x = 0$ , (lbs/in) Controlling indices are I1, I2, and NXT1.
$\bar{K}_\phi$	BARKP	Same as above along the axis $\phi = 0$ , (lbs/in) Controlling indices are I3, I4, and NXT2.
$W_x$	WX	Weights along the boundary $x = 0$ , (lbs) Controlling indices are I5, I6, and NXT3.
$W_\phi$	WP	Weights along the boundary $\phi = 0$ , (lbs) Controlling indices are I5, I6, and NXT4.
$W$	WT	Weights at interior points of the panel (lbs) Controlling index is JWT
$i, j$	IM1, IM2	Row and column number, respectively, of off-diagonal term of mass (internal) matrix.
$(M_1)_{i, j, i \neq j}$	AM3	Actual element that corresponds to IM1 and IM2 in the (internal) mass matrix. Number of this off-diagonal terms is limited to six.
	IETBL, NEC	Row number and column number, respectively of the matrix element (E) table. The E table is generated according to the definition of Table 2 for all grid points. A particular element of the table is to be transferred to a particular location in the coefficient matrix A.
	NZRO	Number of non-zero elements in a row of coefficient matrix.
	NAC	Column number of the non-zero element in the coefficient matrix.

NOTE: For a stiffener which is placed along one boundary of the panel, double the values of the corresponding cross-sectional area and the moment of inertia as input.

## 2. Sandwiched Shell

The definitions made for the solid shell apply to the sandwiched shell with the following exceptions and additional definitions.

Symbols used in Analysis	Fortran Coding	Definition
h	H	Total thickness of outer and inner facings.
c	CORE	Thickness of core.
$\frac{c}{m}$	BARM	Mass per unit area of the panel.
	IOPT	= 2. For sandwiched panel.

KEY PUNCH FORM - GENERAL PURPOSE

FORM 20-708 (6-7-61)

JOB TITLE		ENGINEER										PAGE	OF	
DPA SERIAL NO.		PRE	DEC	JOB NO.	LASH	FOR ORGN. NO.					ANALYST	DATE		
INPUT DATA FORMAT														
A														
B														
PHIO														
PNU														
E														
RHO														
H														
CORE														
BARM														
N														
M														
*														
**														
*NMODE														
**IOPT														
J1	K2	JWT												
SZX (1)														
SZX (2)														
SZX (J1)														
SZX (J2)														
SZP (K2)														
SIX (2)														
SIX (J1)														
SIX (J2)														
SIP (K2)														
SIP (1)														
SIP (2)														
SIP (J1)														
SIP (J2)														
SAX (1)														
SAX (2)														
SAX (J1)														
SAX (J2)														
SAP (K2)														
SAP (1)														
SAP (2)														
SAP (J1)														
SAP (J2)														
J2	NEXT													
FOR NEXT 0. REPEAT THIS AND THE FOLLOWING CARDS														
FOR NEXT 1. THE FOLLOWING IS THE LAST READING FOR SAX, SAP, SIX, SIP, SZX, SZA, SZP														
SAX (J2)														
SIX (J2)														
SIP (J2)														
SZX (J2)														
SZA (J2)														
SZP (J2)														
I1	I2	NXT1												
FOR NXT1 0. REPEAT THIS AND THE FOLLOWING CARDS														
FOR NXT1 1. THE FOLLOWING IS THE LAST READING FOR BARKX														





## LISTINGS

The following listing contains the complete main program and its subroutines with the exception of a subroutine named "MITER"\*. The MITER subroutine is a standard eigenvalue, eigenvector routine. Its physical package of the subroutine follows CHAIN 3. DATA are also included here, and the technique for their arrangements is stated in the next section of the appendix.

---

\*The writeup and subroutine deck of "MITER" are available at IBM SHARE general program library.



```

$EXECUTE      FIB
* XEQ
* CHAIN (1,8)
C CHAIN 1 - SETUP FOR E TABLE
  DIMENSION SAX(24),SIX(24),SZX(24),SAP(24),SIP(24),SZP(24),WT(24),
  1 BARKX(24),BARKP(24),WP(24),CZX(24),FX(24),DBXP(24),DBX(24),
  2 CZP(24),FP(24),DPPR(24),DP(24),SKP(24),BIGP(24),BIGR(24),
  3 ALFA(24),BETA(24),AM1(24,24),AM2(21,24),EL(24,80),WX(24),
  4 SKX(24),BIGH(24),XEL(24,80),XAM1(24,24),XAM2(21,24),IM1(6),
  5 IM2(6),AM3(6)
  EQUIVALENCE (XEL,EL),(NMODE,NMODX),(E,XEX),(RHO,XRHO),(H,XH),
  1 (D,XD),(ADP2,XADP2),(AM1,XAM1),(AM2,XAM2)
  COMMON EL,NMODE,E,RHO,H,D,ADP2,AM1,AM2,BARM,IOPT
C
C FOR SINGLE SHELL IOPT =1
C FOR SANDWICHED SHELL WITH HONEYCOMB CORE IOPT =2
C
  10 FORMAT (6E12.8/3E12.8,4I3)
  30 FORMAT(3I3)
  31 FORMAT(2I3)
  32 FORMAT(6E12.8)
  50 FORMAT(I12,E12.8,I12,E12.8,I12)
  62 FORMAT(2E12.8)
  1 STARTS=1000.
  READ INPUT TAPE 5,10,A,B,PHIO,PNU,E,RHO,H,CORE,BARM,N,M,NMODE,IOPT
  M =M-1
  N =N-1
  NM=(N+1)*(M+1)
  DO 15 I=1,NM
    SAX(I)=0.
    SIX(I)=0.
    SZX(I)=0.
    SAP(I)=0.
    SIP(I)=0.
    SZP(I)=0.
    WT(I)=0.
    BARKP(I)=0.
  25 WP(I)=0.
  15 CONTINUE
  CN=N

```

```

MAIN
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CHN10072
CHN10073
CHN10074
CHN10075
CHN10076
CHN10077

CM=M
N1=N+1
M1=M+1
NEXT =0
DO 20 I=1,N1
  BARKX(I)=0.
  20 WX(I)=0.
  IF(IOPT-2) 336,337,338
  338 CALL EXIT
  336 WRITE OUTPUT TAPE 6,7
  7 FORMAT (1H1 //20X,62H ** FINITE DIFFERENCE METHOD FOR LOCALIZED SHCHN10048
    1ELL VIBRATION ** ///)
  GO TO 328
  337 WRITE OUTPUT TAPE 6,339,CORE,BARM
  339 FORMAT (1H1 //75H FINITE DIFFERENCE METHOD FOR SANDWICHED SHELL
    1WITH CORE THICKNESS =E14.5,22H AND RIGIDITY FACTOR =E14.5,/)
  328 WRITE OUTPUT TAPE 6,300
  300 FORMAT (1H 35X,18H ** INPUT DATA ** //)
  WRITE OUTPUT TAPE 6,310
  310 FORMAT(9X,1HA 13X,1HB 11X,1HC 11X,1HD 13X,1HE 11X,1HF 13X,1HG 13X,
    1 1H 13X,1HNMODE)
  WRITE OUTPUT TAPE 6,320,A,B,PHIO,PNU,E,RHO,H,NMODE
  320 FORMAT(7E15.5,8X,I2 //)
  READ INPUT TAPE 5,30,J1,K2,JWT
  READ INPUT TAPE 5,32,(SZX(I),I=1,J1),(SZP(I),I=1,K2)
  READ INPUT TAPE 5,32,(SIX(I),I=1,J1),(SIP(I),I=1,K2)
  READ INPUT TAPE 5,32,(SAX(I),I=1,J1),(SAP(I),I=1,K2)
  26 READ INPUT TAPE 5,31,J2,NEXT
  READ INPUT TAPE 5,32,SAX(J2),SAP(J2),SIX(J2),SIP(J2),SZX(J2),
    1 SZP(J2)
  IF(NEXT-1) 26,52,26
  52 READ INPUT TAPE 5,30,I1,I2,NXT1
  READ INPUT TAPE 5,62,BARKX(I1),BARKX(I2)
  IF(NXT1-1) 52,51,52
  51 READ INPUT TAPE 5,30,I3,I4,NXT2
  READ INPUT TAPE 5,62,BARKP(I3),BARKP(I4)
  IF(NXT2-1) 51,53,51
  53 READ INPUT TAPE 5,30,I5,I6,NXT3
  READ INPUT TAPE 5,62,WX(I5),WX(I6)
  IF(NXT3-1) 53,54,53

```

```

54 READ INPUT TAPE 5,30,I7,I8,NXT4
   READ INPUT TAPE 5,62,WP(I7),WP(I8)
   IF(NXT4-1) 54,55,54
55 READ INPUT TAPE 5,32,(WT(I),I=1,JWT)
   WRITE OUTPUT TAPE 6,330
330 FORMAT( 8H **AX** )
   WRITE OUTPUT TAPE 6,340,(SAX(I),I=1,NM)
   WRITE OUTPUT TAPE 6,350
350 FORMAT(// 9H **APHI** )
   WRITE OUTPUT TAPE 6,340,(SAP(I),I=1,NM)
   WRITE OUTPUT TAPE 6,360
360 FORMAT(// 9H **ZX** )
   WRITE OUTPUT TAPE 6,340,(SZX(I),I=1,NM)
   WRITE OUTPUT TAPE 6,370
370 FORMAT(// 9H **ZP** )
   WRITE OUTPUT TAPE 6,340,(SZP(I),I=1,NM)
   WRITE OUTPUT TAPE 6,380
380 FORMAT(// 9H **IX** )
   WRITE OUTPUT TAPE 6,340,(SIX(I),I=1,NM)
   WRITE OUTPUT TAPE 6,390
390 FORMAT(// 9H **IP** )
   WRITE OUTPUT TAPE 6,340,(SIP(I),I=1,NM)
   WRITE OUTPUT TAPE 6,400
400 FORMAT(// 9H **WX** )
   WRITE OUTPUT TAPE 6,340,(WX(I),I=1,N1)
   WRITE OUTPUT TAPE 6,410
410 FORMAT(// 9H **WP** )
   WRITE OUTPUT TAPE 6,340,(WP(I),I=1,NM)
   WRITE OUTPUT TAPE 6,420
420 FORMAT(// 9H **WT** )
   WRITE OUTPUT TAPE 6,340,(WT(I),I=1,NM)
   WRITE OUTPUT TAPE 6,430
430 FORMAT(// 9H **KX** )
   WRITE OUTPUT TAPE 6,340,(BARKX(I),I=1,N1)
   WRITE OUTPUT TAPE 6,440
440 FORMAT(// 9H **KP** )
   WRITE OUTPUT TAPE 6,340,(BARKP(I),I=1,NM)
340 FORMAT(6E18.5)
60 H2 =H*H
   H3 =H2*H

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```

PNU2 =PNU*PNU
IF(IOPT-2) 101,102,103
103 CALL EXIT
101 D =E*H3/(12.*(1.-PNU2))
  GO TO 104
102 D =(E*H*(H+2.*CORE)**2)/(16.*(1.-PNU2))
104 DELX =0.5*B/CM
  DELP =0.5*PHIO/CN
  BLAM =A*DELP/DELX
  SK =E*H/(1.-PNU2)
  ADP =A*DELP
  HADP =H*ADP
  HDELX =H*DELX
  EAP =E/ADP
  EDX =E/DELX
  DO 100 I=1,NM
    SKX(I) =SK+E*SAX(I)/ADP
    CZX(I) =SAX(I)*SZX(I)/(SAX(I)+HADP)
    FX(I) =EAP*(SIX(I)+SAX(I))*SZX(I)-CZX(I)**2)
    DBXP(I) =D+SK*CZX(I)*CZX(I)
    DBX(I) =FX(I)+DBXP(I)
    CZP(I) =SAP(I)*SZP(I)/(SAP(I)+HDELX)
    FPI(I) =EDX*(SIP(I)+SAP(I))*SZP(I)-CZP(I)**2)
    DPPR(I) =D+SK*CZP(I)*CZP(I)
    DPI(I) =FPI(I)+DPPR(I)
    SKPI(I) =SK+EDX*SAP(I)
  100 CONTINUE
  PNU5 =0.5*PNU
  DNU =D*(1.-PNU)
  PNUK =PNU2*SK*SK
  AK =A*SK
  PKN1 =(1.-PNU)*SK*SK
  PNUK1 =PNU*SK*SK
  DO 110 J=1,NM
    BIGH(J) =PNU5*(DBXP(J)+DPPR(J))+DNU
    SKXP =SKX(J)*SKP(J)
    BIGP(J) =(SKXP-PNUK1)/PKN1
    BIGR(J) =(SKXP-PNUK)/AK
    ALFA(J) =PNU*DBXP(J)+2.*DNU
    BETA(J) =PNU*DPPR(J)+2.*DNU
  
```

```

110 CONTINUE
   RG =RHO*386.064
   IF (IOPT-2) 112,113,114
114 CALL EXIT
112 RGH =RG*H
   GO TO 116
113 RGH =BARM*386.064
116 DXAP =RGH*DELX*ADP
C
C  FORMATION OF M1 AND M2 MATRIX
   DO 119 I =1,NM
   DO 119 J =1,NM
119 AM1(I,J) =0.
C
   DO 120 I=1,NM
   IF(I-N1) 130,130,125
125 AM1(I,I) =1.+(WT(I)+RG*(SAP(I)+ADP+SAX(I)*DELX))/DXAP
   GO TO 120
130 AM1(I,I) =1.
120 CONTINUE
   K =1
   DO 140 I=1,M1
   AM1(K,K) =1.
   K=K+N1
140 CONTINUE
   READ INPUT TAPE 5,403,(IM1(IJ),IM2(IJ),IJ=1,6)
   READ INPUT TAPE 5, 32,(AM3(K),K=1,6)
403 FORMAT (12I3)
   DO 414 K =1,6
   I=IM1(K)
   J=IM2(K)
414 AM1(I,J) =AM3(K)
C
C   M2=2*(N+M+2)+1
C
   DO 142 I=1,M2
   DO 142 J=1,NM
142 AM2(I,J) =0.
C

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```

BL3 =BLAM**3
ADP2 =ADP*ADP
APRG =ADP2*RGH
APRL =BLAM*(1.-PNU)*APRG
RG5 =0.5*RG
RG4 =0.5*RG5

DO 150 I=2,N1
150 AM2(I,I)=-2.*(D/DBX(I))*((RG5*(SAP(I)*ADP+SAX(I)*DELX)+WX(I))/(BL3CHN10206
1*APRG))
AM2(N+2,1) =2.*(RG4*(SAP(1)*ADP+SAX(1)*DELX)+WX(1))/APRL
DO 146 K=2,M1
I =N1*(K-1)+1
L =2*N+3+K
AM2(L,I) =-2.*(D/DP(I))*(BLAM*(RG5*(SAX(I)*DELX+SAP(I)*ADP)+WP(I))
1 )/(ADP2*RGH)
146 CONTINUE
WRITE OUTPUT TAPE 6,122
122 FORMAT (1H125HMASS 1 MATRIX - INTERNAL )
DO 121 I=1,NM
WRITE OUTPUT TAPE 6,470,I
121 WRITE OUTPUT TAPE 6,340,(AM1(I,J),J=1,NM)
WRITE OUTPUT TAPE 6,126
126 FORMAT(/26H MASS 2 MATRIX - BOUNDARY )
DO 152 I=1,M2
WRITE OUTPUT TAPE 6,470,I
152 WRITE OUTPUT TAPE 6,340, (AM2(I,J),J=1,NM)
C
C TO MAKE A STORAGE TABLE FOR ELEMENTS USED IN MATRICES
C
BL2 =BLAM*BLAM
BL4 =BL2*BL2
BLD4 =BL4/D
BLD2 =BL2/D
FAC14 =PNU/BL2
FAC19 =PNU*BL2
FAC21 =ADP2*BL2
ADBL2 =ADP2/BL2
JK=80

```

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C

```
DO 165 I=1,NM
DO 165 J=1,JK
165 EL(I,J) =0.
ADBL2 =ADP2/BL2
DO 170 J=1,NM
EL(J,1) =1.
EL(J,2) =-4.*(BLD4*DBX(J)+BLD2*BIGH(J))
EL(J,3) =BLD4*DBX(J)
EL(J,4) =-4.*(BLD2*BIGH(J)+DP(J)/D)
EL(J,5) =2.*BLD2*BIGH(J)
EL(J,6) =DP(J)/D
EL(J,7) =2.*FAC21/(A*D)
EL(J,8) =6.*BLD4*DBX(J)+8.*BLD2*BIGH(J)+6.*DP(J)/D
EL(J,9) =-0.5*EL(1,7)
EL(J,11) =ALFA(J)/(BL2*DRX(J))
EL(J,12) =-2.*(1.+EL(J,11))
EL(J,14) =FAC14*DBXP(J)/DBX(J)
EL(J,15) =-2.*(1.+EL(J,14))
EL(J,17) =BL2*BETA(J)/DP(J)
EL(J,18) =-2.*(1.+EL(J,17))
EL(J,19) =FAC19*DPPR(J)/DP(J)
EL(J,20) =-2.*(1.+EL(J,19))
EL(J,21) =-2.*FAC21*BIGR(J)
EL(J,22) =-0.5*EL(J,21)
EL(J,23) =6.*SKX(J)*BL4/SK+8.*RIGP(J)*BL2+6.*SKP(J)/SK
EL(J,24) =-4.*BL2*(SKX(J)*BL2/SK+BIGP(J))
EL(J,25) =SKX(J)*BL4/SK
EL(J,26) =-4.*(SKP(J)/SK+RIGP(J)*BL2)
EL(J,27) =2.*BIGP(J)*BL2
EL(J,28) =SKP(J)/SK
EL(J,29) =-2.
EL(J,30) =D*(1.-PNU)/A
EL(J,31) =SK*ADBL2*BIGR(J)/SKX(J)
EL(J,32) =-4.
EL(J,33) =-FAC14*SK/SKX(J)
EL(J,34) =-(4.+EL(J,33))
EL(J,35) =2.*EL(J,31)
EL(J,36) =-EL(J,12)
EL(J,37) =-EL(J,17)
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EL(J,38) =2.
EL(J,39) =2.*EL(J,2)
EL(J,40) =2.*EL(J,3)
EL(J,41) =2.*EL(J,4)
EL(J,42) =2.*EL(J,5)
EL(J,43) =2.*EL(J,6)
EL(J,44) =EL(J,3)+EL(J,8)
EL(J,45) =EL(J,6)+EL(J,8)
EL(J,46) =EL(J,3)+EL(J,6)+EL(J,8)
EL(J,47) =2.*EL(J,11)
EL(J,48) =-EL(J,47)
EL(J,49) =4.*EL(J,5)
EL(J,50) =4.*EL(J,27)
EL(J,51) =2.*EL(J,14)
EL(J,52) =EL(J,25)+EL(J,23)
EL(J,53) =EL(J,23)+EL(J,28)
EL(J,54) =EL(J,17)*2.
EL(J,55) =-EL(J,54)
EL(J,56) =EL(J,23)+EL(J,25)+EL(J,28)
EL(J,57) =2.*EL(J,22)
EL(J,58) =2.*EL(J,24)
EL(J,59) =2.*EL(J,25)
EL(J,60) =2.*EL(J,26)
EL(J,61) =2.*EL(J,27)
EL(J,62) =2.*EL(J,28)
EL(J,63) =2.
EL(J,64) =-EL(1,30)
EL(J,65) =2.*EL(1,9)
EL(J,66) =-1.
EL(J,67) =-EL(J,11)
EL(J,69) =-EL(J,18)
EL(J,70) =2.*EL(J,19)
EL(J,71) =4.
170 CONTINUE
EL(1,68) =+(BARKX(1)*ADP2)/(D*BLAM*(1.-PNU)*0.5)
C
DO 180 K=2,M1
I=(K-1)*(N+1)+1
180 EL(I,16) =-2.*BLAM*BARKP(I)*ADP2/DP(I)
DO 185 I=2,N1
  
```



```

185 EL(I,10) =-2.*BARKX(I)*ADP2/(DBX(I)*BL3)
WRITE OUTPUT TAPE 6,450
450 FORMAT(1H114H ** E TABLE **)
DO 460 I=1,NM
WRITE OUTPUT TAPE 6,470,I
WRITE OUTPUT TAPE 6,480,(EL(I,J),J=1,71)
460 CONTINUE
470 FORMAT(/9H ROW NO. I2)
480 FORMAT(8E15.4)
CALL CHAIN (2,8)
END
* CHAIN (2,8)
C CHAIN 2 - TRANSFER OF E TABLE TO PROPER LOCATION IN A MATRIX AND
C REDUCTION PERFORMED
C
DIMENSION EL(24,80),TEMP1(34,24),EXTRA(34,24),A11(24,24),
1 A12(24,21),A13(24,34),A22(21,21),A31(34,24),A32(34,21),
2 A33(34,34),AA1(24,79),AA2(21,45),AA3(34,79),B1(34,1),NAC(60),
3 NEC(60),AM1(24,24),AM2(21,24),A21(21,24),IETBL(34),XEL(24,80),
4 XAM1(24,24),XAM2(21,24)
EQUIVALENCE (AA1(1),A11(1)),(AA1(577),A12(1)),(AA1(1081),A13(1)),
1 (AA2(1),A21(1)),(AA2(505),A22(1)),(AA3(1),A31(1)),
2 (AA3(817),A32(1)),(AA3(1531),A33(1))
EQUIVALENCE (XEL,EL),(NMODE,NMODX),(E,XEX),(RHO,XRHO),(H,XH),
1 (D,XD),(ADP2,XADP2),(AM1,XAM1),(AM2,XAM2)
COMMON EL,NMODE,E,RHO,H,D,ADP2,AM1,AM2,BARM,IOPT,EXTRA,TEMP1
C GENERATING A21(21*24) AND A22(21*21) MATRICES
C
NR=21
NC=45
DO 501 I=1,NR
DO 501 J=1,NC
501 A22(I,J) =0.
575 FORMAT(36I2)
READ INPUT TAPE 5,575,(IETBL(J),J=1,NR)
DO 511 J=1,NR
L3 =IETBL(J)
552 READ INPUT TAPE 5,535,NZRO,(NAC(I),NEC(I),I=1,NZRO)
DO 510 K=1,NZRO

```

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CHN20028

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```

L1=NAC(K)
L2=NEC(K)
AA2(J,L1) =EL(L3,L2)
510 CONTINUE
511 CONTINUE
509 WRITE OUTPUT TAPE 6,550
550 FORMAT(1H1 16H** A21 MATRIX **//)
DO 520 I=1,NR
WRITE OUTPUT TAPE 6,560,I
520 WRITE OUTPUT TAPE 6,565,(A21(I,J),J=1,24)
WRITE OUTPUT TAPE 6,570
560 FORMAT( 8H ** ROW I2,2H**)
565 FORMAT(8E15.4)
570 FORMAT(1H1 16H** A22 MATRIX **//)
DO 521 I=1,NR
WRITE OUTPUT TAPE 6,560,I
521 WRITE OUTPUT TAPE 6,565,(A22(I,J),J=1,21)
C
C GENERATING A31(34*24),A32(34*21) AND A33(34*34) MATRICES
C
WRITE OUTPUT TAPE 6,48
48 FORMAT (1H116HCHECK M1 MATRIX //)
DO 45 I=1,24
WRITE OUTPUT TAPE 6,46,I
45 WRITE OUTPUT TAPE 6,47,(AM1(I,J),J=1,24)
46 FORMAT ( 8H ROW NO.I2)
47 FORMAT (8E15.4)
WRITE OUTPUT TAPE 6,49
49 FORMAT (1H116HCHECK M2 MATRIX //)
DO 52 I=1,21
WRITE OUTPUT TAPE 6,46,I
52 WRITE OUTPUT TAPE 6,47,(AM2(I,J),J=1,24)
NR=34
NC=79
DO 600 I=1,NR
DO 600 J=1,NC
600 AA3(I,J) =0.
C
C READ INPUT TAPE 5,575,(IETBL(J),J=1,NR)
C

```

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```

DO 610 J=1,NR
L3 =IETBL(J)
READ INPUT TAPE 5,535,NZRO,(NAC(I),NEC(I),I=1,NZRO)
DO 610 K=1,NZRO
L1=NAC(K)
L2=NEC(K)
AA3(J,L1)=EL(L3,L2)
610 CONTINUE

615 WRITE OUTPUT TAPE 6,630
630 FORMAT(1H1 16H** A31 MATRIX **//)
DO 640 I=1,34
WRITE OUTPUT TAPE 6,560,I
640 WRITE OUTPUT TAPE 6,565,(A31(I,J),J=1,24)
WRITE OUTPUT TAPE 6,650
650 FORMAT(1H1 16H** A32 MATRIX **//)
DO 660 I=1,34
WRITE OUTPUT TAPE 6,560,I
660 WRITE OUTPUT TAPE 6,565,(A32(I,J),J=1,21)
WRITE OUTPUT TAPE 6,670
670 FORMAT(1H1 16H** A33 MATRIX **//)
DO 680 I=1,34
WRITE OUTPUT TAPE 6,560,I
680 WRITE OUTPUT TAPE 6,565,(A33(I,J),J=1,34)
DO 522 I=1,21
522 B1(I,1)=1.
CALL MXIV1 (A22,21,B1,1,DETER)
WRITE OUTPUT TAPE 6,576,DETER
576 FORMAT(//10X,14H DETERMINANT =E15.6)
CALL MLTMX4 (34,21,A32,21,21,A22,EXTRA,A32)
535 FORMAT(24I3)
CALL MLTMX1 (34,21,A32,21,24,A21,EXTRA,TEMP1)
DO 690 I=1,34
690 B1(I,1)=1.
CALL MXIV2 (A33,34,B1,1,DETR)
WRITE OUTPUT TAPE 6,576,DETR
DO 700 I=1,34
DO 700 J=1,24
700 TEMP1(I,J)=A31(I,J)-TEMP1(I,J)
CALL MLTMX2 (34,34,A33,34,24,TEMP1,EXTRA,TEMP1)

```

```
CALL MLTMX3 (34,34,A33,34,21,A32,EXTRA,A32)
CALL MLTX11 (34,21,A32,21,24,AM2,EXTRA,A33)
```

C  
C  
C  
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C  
C

```
GENERATING A11(24*24),A12(24*21), AND A13(24*24) MATRIX
```

```
NR=24
NC=79
DO 720 I=1,NR
DO 720 J=1,NC
720 AA1(I,J) =0.
READ INPUT TAPE 5,575,(IETBL(J),J=1,NR)
DO 730 J=1,NR
L3 =IETBL(J)
```

C

```
READ INPUT TAPE 5,535,NZRO,(NAC(I),NEC(I),I=1,NZRO)
```

```
DO 730 K=1,NZRO
L1=NAC(K)
L2=NEC(K)
AA1(J,L1) =EL(L3,L2)
730 CONTINUE
WRITE OUTPUT TAPE 6,740
740 FORMAT(1H1 16H** A11 MATRIX **//)
DO 741 I=1,NR
WRITE OUTPUT TAPE 6,745,I
741 WRITE OUTPUT TAPE 6,750,(A11(I,J),J=1,24)
745 FORMAT(8H ** ROW I2,2H** )
750 FORMAT(8E15.4)
WRITE OUTPUT TAPE 6,755
755 FORMAT(1H1 16H** A12 MATRIX **//)
DO 751 I=1,NR
WRITE OUTPUT TAPE 6,745,I
751 WRITE OUTPUT TAPE 6,750,(A12(I,J),J=1,21)
WRITE OUTPUT TAPE 6,765
765 FORMAT(1H1 16H** A13 MATRIX **//)
DO 761 I=1,NR
WRITE OUTPUT TAPE 6,745,I
761 WRITE OUTPUT TAPE 6,750,(A13(I,J),J=1,34)
```

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CHN20147
CHN20148
```

```

CALL MLTMX5 (24,21,A12,21,21,A22,EXTRA,A12)
CALL MLTX12 (24,21,A12,21,24,A21,EXTRA,A31)
CALL MLTMX6 (24,34,A13,34,24,TEMP1,EXTRA,EL)
DO 772 I=1,24
DO 772 J=1,24
772 A11(I,J) =A11(I,J)-A31(I,J)-EL(I,J)
C
CALL MLTMX8 (24,21,A12,21,24,AM2,EXTRA,EL)
CALL MLTX10 (24,34,A13,34,24,A33,EXTRA,A13)
DO 773 I=1,24
DO 773 J=1,24
A31(I,J) =EL(I,J)-A13(I,J)
775 A13(I,J) =AM1(I,J)-A31(I,J)
773 CONTINUE
WRITE OUTPUT TAPE 6,54
54 FORMAT (1H1 20HINTERMEDIATE PRINTS )
DO 56 I =1,24
WRITE OUTPUT TAPE 6,46,I
56 WRITE OUTPUT TAPE 6,47,(A13(I,J),J=1,24)
DO 780 I=1,24
780 B1(I,1)=1.
CALL MXIV3 (A11,24,B1,1,DETR)
WRITE OUTPUT TAPE 6,576,DETR
WRITE OUTPUT TAPE 6,54
DO 62 I=1,24
WRITE OUTPUT TAPE 6,46,I
62 WRITE OUTPUT TAPE 6,47,(A11(I,J),J=1,24)
CALL MLTMX7 (24,24,A11,24,24,A13,EXTRA,EL)
WRITE OUTPUT TAPE 6,782
782 FORMAT(1H1 13HFINAL MATRIX /)
DO 785 I=1,24
WRITE OUTPUT TAPE 6,745,I
785 WRITE OUTPUT TAPE 6,750,(EL(I,J),J=1,24)
DO 796 I=1,34
DO 796 J=1,24
796 EXTRA(I,J) =A33(I,J)
CALL CHAIN (3,8)
END
SUBROUTINE MLTX12 (N1,M1,A,N2,M2,B,AB,ABF)

```

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CHN20149
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CHN20186

```

MLTM

```

C      **FOR MULTIPLICATION OF REAL MATRICES
C      AB = A * B
C      DIMENSION A(24,21),B(21,24),AB(34,24),ABF(34,24)
C      10
C      DO 1 I=1,N1
C      DO 1 J=1,M2
C      AB(I,J)=0.
C      DO 1 K=1,M1
C      PRODC=A(I,K)*B(K,J)
C      AB(I,J)=AB(I,J)+PRODC
C      1  DO 2 I=1,N1
C      DO 2 J=1,M2
C      ABF(I,J)=AB(I,J)
C      RETURN
C      END
C      SUBROUTINE MLTX11 (N1,M1,A,N2,M2,B,AB,ABF)
C      **FOR MULTIPLICATION OF REAL MATRICES
C      AB = A * B
C      DIMENSION A(34,21),B(21,24),AB(34,24),ABF(34,34)
C      10
C      DO 1 I=1,N1
C      DO 1 J=1,M2
C      AB(I,J)=0.
C      DO 1 K=1,M1
C      PRODC=A(I,K)*B(K,J)
C      AB(I,J)=AB(I,J)+PRODC
C      1  DO 2 I=1,N1
C      DO 2 J=1,M2
C      ABF(I,J)=AB(I,J)
C      RETURN
C      END
C      SUBROUTINE MLTX10 (N1,M1,A,N2,M2,B,AB,ABF)
C      **FOR MULTIPLICATION OF REAL MATRICES
C      AB = A * B
C      DIMENSION A(24,34),B(34,34),AB(34,24),ABF(24,34)
C      10
C      DO 1 I=1,N1
C      DO 1 J=1,M2

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```
AB(I,J)=0.  
DO 1 K=1,M1  
  PRODC=A(I,K)*B(K,J)  
  AB(I,J)=AB(I,J)+PRODC  
DO 2 I=1,N1  
DO 2 J=1,M2  
  ABF(I,J)=AB(I,J)  
RETURN  
END
```

MLTM  
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MLTM

```
C  
C  
C  
C  
SUBROUTINE MLTMX8 (N1,M1,A,N2,M2,B,AB,ABF)  
**FOR MULTIPLICATION OF REAL MATRICES  
  AB = A * B  
  DIMENSION A(24,21),B(21,24),AB(34,24),ABF(24,80)
```

MLTM  
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```
DO 1 I=1,N1  
DO 1 J=1,M2  
  AB(I,J)=0.  
DO 1 K=1,M1  
  PRODC=A(I,K)*B(K,J)  
  AB(I,J)=AB(I,J)+PRODC  
DO 2 I=1,N1  
DO 2 J=1,M2  
  ABF(I,J)=AB(I,J)  
RETURN  
END
```

MLTM  
MLTM

```
C  
C  
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C  
SUBROUTINE MLTMX7 (N1,M1,A,N2,M2,B,AB,ABF)  
**FOR MULTIPLICATION OF REAL MATRICES  
  DIMENSION A(24,24),B(24,34),AB(34,24),ABF(24,80)  
  AB = A * B
```

MLTM  
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MLTM  
MLTX  
MLTX

```
DO 1 I=1,N1  
DO 1 J=1,M2  
  AB(I,J)=0.  
DO 1 K=1,M1  
  PRODC=A(I,K)*B(K,J)  
  AB(I,J)=AB(I,J)+PRODC  
DO 2 I=1,N1  
DO 2 J=1,M2
```

```

2 ABF(I,J)=AB(I,J)
  RETURN
  END
C SUBROUTINE MLTMX6 (N1,M1,A,N2,M2,B,AB,ABF)
C **FOR MULTIPLICATION OF REAL MATRICES
C AB = A * B
C
10 DIMENSION A(24,34),B(34,24),AB(34,24),ABF(24,80)
  DO 1 I=1,N1
  DO 1 J=1,M2
  AB(I,J)=0.
  DO 1 K=1,M1
  PRODC=A(I,K)*B(K,J)
  AB(I,J)=AB(I,J)+PRODC
  DO 2 I=1,N1
  DO 2 J=1,M2
  ABF(I,J)=AB(I,J)
  RETURN
  END
C SUBROUTINE MLTMX5 (N1,M1,A,N2,M2,B,AB,ABF)
C **FOR MULTIPLICATION OF REAL MATRICES
C AB = A * B
C
10 DIMENSION A(24,21),B(21,21),AB(34,24),ABF(24,21)
  DO 1 I=1,N1
  DO 1 J=1,M2
  AB(I,J)=0.
  DO 1 K=1,M1
  PRODC=A(I,K)*B(K,J)
  AB(I,J)=AB(I,J)+PRODC
  DO 2 I=1,N1
  DO 2 J=1,M2
  ABF(I,J)=AB(I,J)
  RETURN
  END
C SUBROUTINE MLTMX4 (N1,M1,A,N2,M2,B,AB,ABF)
C **FOR MULTIPLICATION OF REAL MATRICES

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```

C      AB = A * B
C
      DIMENSION A(34,21),B(21,21),AB(34,24),ABF(34,21)
      DO 1 I=1,N1
      DO 1 J=1,M2
      AB(I,J)=0.
      DO 1 K=1,M1
      PRODUCT=A(I,K)*B(K,J)
      AB(I,J)=AB(I,J)+PRODUCT
      DO 2 I=1,N1
      DO 2 J=1,M2
      ABF(I,J)=AB(I,J)
      RETURN
      END
      SUBROUTINE MLTMX3 (N1,M1,A,N2,M2,B,AB,ABF)

```

MLTM  
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MLTX  
MLTM

```

C      **FOR MULTIPLICATION OF REAL MATRICES
C      AB = A * B
C
      DIMENSION A(34,34),B(34,21),AB(34,24),ABF(34,21)
      DO 1 I=1,N1
      DO 1 J=1,M2
      AB(I,J)=0.
      DO 1 K=1,M1
      PRODUCT=A(I,K)*B(K,J)
      AB(I,J)=AB(I,J)+PRODUCT
      DO 2 I=1,N1
      DO 2 J=1,M2
      ABF(I,J)=AB(I,J)
      RETURN
      END
      SUBROUTINE MLTMX2 (N1,M1,A,N2,M2,B,AB,ABF)

```

MLTM  
MLTM  
MLTM  
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MLTM  
MLTM  
MLTM

```

C      **FOR MULTIPLICATION OF REAL MATRICES
C      AB = A * B
C
      DIMENSION A(34,34),B(34,24),AB(34,24),ABF(34,24)
      DO 1 I=1,N1
      DO 1 J=1,M2
      AB(I,J)=0.

```

MLTM  
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MLTM  
MLTX  
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MLTX  
MLTX  
MLTM

```
1 DO 1 K=1,M1  
  PRODUCT=A(I,K)*B(K,J)  
  AB(I,J)=AB(I,J)+PRODUCT  
2 DO 2 I=1,N1  
  DO 2 J=1,M2  
  ABF(I,J)=AB(I,J)  
  RETURN  
  END  
C SUBROUTINE MLTMX1 (N1,M1,A,N2,M2,B,AB,ABF)  
C **FOR MULTIPLICATION OF REAL MATRICES  
C AB = A * B  
C DIMENSION A(34,21),B(21,24),AB(34,24),ABF(34,24)  
10 DO 1 I=1,N1  
  DO 1 J=1,M2  
  AB(I,J)=0.  
  DO 1 K=1,M1  
  PRODUCT=A(I,K)*B(K,J)  
  AB(I,J)=AB(I,J)+PRODUCT  
1 DO 2 I=1,N1  
  DO 2 J=1,M2  
  ABF(I,J)=AB(I,J)  
  RETURN  
  END
```

```

C      SUBROUTINE MXIV1 (A,N,B,M,DETERM)
C      MATRIX INVERSION WITH ACCOMPANYING SOLUTION OF LINEAR EQUATIONS
C      DIMENSION IPIVOT(21),A(21,21),B(21,1),INDEX(21,2),PIVOT(21)
C      EQUIVALENCE (IROW,JROW), (ICOLUMN,JCOLUMN), (AMAX, T, SWAP)
C
C      INITIALIZATION
C      10 DETERM=1.0
C      15 DO 20 J=1,N
C      20 IPIVOT(J)=0
C      30 DO 550 I=1,N
C
C      SEARCH FOR PIVOT ELEMENT
C      40 AMAX=0.0
C      45 DO 105 J=1,N
C      50 IF (IPIVOT(J)-1) 60, 105, 60
C      60 DO 100 K=1,N
C      70 IF (IPIVOT(K)-1) 80, 100, 740
C      80 IF (ABSF(AMAX)-ABSF(A(J,K))) 85, 100, 100
C      85 IROW=J
C      90 ICOLUMN=K
C      95 AMAX=A(J,K)
C      100 CONTINUE
C      105 CONTINUE
C      110 IPIVOT(ICOLUMN)=IPIVOT(ICOLUMN)+1
C
C      INTERCHANGE ROWS TO PUT PIVOT ELEMENT ON DIAGONAL
C      130 IF (IROW-ICOLUMN) 140, 260, 140
C      140 DETERM=-DETERM
C      150 DO 200 L=1,N
C      160 SWAP=A(IROW,L)
C      170 A(IROW,L)=A(ICOLUMN,L)
C      200 A(ICOLUMN,L)=SWAP
C      205 IF(M) 260, 260, 210
C      210 DO 250 L=1, M
C      220 SWAP=B(IROW,L)
C      230 B(IROW,L)=B(ICOLUMN,L)
C      250 B(ICOLUMN,L)=SWAP

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ANF40201

F4020007

F4020008

F4020009

F4020010

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F4020012

F4020013

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F4020038

F4020039

F4020040

F4020041

F4020042

F4020043

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260 INDEX(I,1)=IROW
270 INDEX(I,2)=ICOLUMN
310 PIVOT(I)=A(ICOLUMN,ICOLUMN)
320 DETERM=DETERM*PIVOT(I)
C
C
C
    DIVIDE PIVOT ROW BY PIVOT ELEMENT
330 A(ICOLUMN,ICOLUMN)=1.0
340 DO 350 L=1,N
350 A(ICOLUMN,L)=A(ICOLUMN,L)/PIVOT(I)
355 IF(M) 380, 380, 360
360 DO 370 L=1,M
370 B(ICOLUMN,L)=B(ICOLUMN,L)/PIVOT(I)
C
C
C
    REDUCE NON-PIVOT ROWS
380 DO 550 L1=1,N
390 IF(L1-ICOLUMN) 400, 550, 400
400 T=A(L1,ICOLUMN)
420 A(L1,ICOLUMN)=0.0
430 DO 450 L=1,N
450 A(L1,L)=A(L1,L)-A(ICOLUMN,L)*T
455 IF(M) 550, 550, 460
460 DO 500 L=1,M
500 B(L1,L)=B(L1,L)-B(ICOLUMN,L)*T
550 CONTINUE
C
C
C
    INTERCHANGE COLUMNS
600 DO 710 I=1,N
610 L=N+1-I
620 IF (INDEX(L,1)-INDEX(L,2)) 630, 710, 630
630 JROW=INDEX(L,1)
640 JCOLUMN=INDEX(L,2)
650 DO 705 K=1,N
660 SWAP=A(K,JROW)
670 A(K,JROW)=A(K,JCOLUMN)
700 A(K,JCOLUMN)=SWAP
705 CONTINUE
710 CONTINUE

```

```

740 RETURN
750 END (2,2,2,2,0)
SUBROUTINE MXIV2 (A,N,B,M,DETERM)
DIMENSION IPIVOT(34),A(34,34),B(34,1),INDEX(34,2),PIVOT(34)
MATRIX INVERSION WITH ACCOMPANYING SOLUTION OF LINEAR EQUATIONS
EQUIVALENCE (IROW,JROW), (ICOLUMN,JCOLUMN), (AMAX, T, SWAP)
C
C   INITIALIZATION
C
C   10 DETERM=1.0
C   15 DO 20 J=1,N
C   20 IPIVOT(J)=0
C   30 DO 550 I=1,N
C
C   SEARCH FOR PIVOT ELEMENT
C
C   40 AMAX=0.0
C   45 DO 105 J=1,N
C   50 IF (IPIVOT(J)-1) 60, 105, 60
C   60 DO 100 K=1,N
C   70 IF (IPIVOT(K)-1) 80, 100, 740
C   80 IF (ABSF(AMAX)-ABSF(A(J,K))) 85, 100, 100
C   85 IROW=J
C   90 ICOLUMN=K
C   95 AMAX=A(J,K)
C  100 CONTINUE
C  105 CONTINUE
C  110 IPIVOT(ICOLUMN)=IPIVOT(ICOLUMN)+1
C
C   INTERCHANGE ROWS TO PUT PIVOT ELEMENT ON DIAGONAL
C
C   130 IF (IROW=ICOLUMN) 140, 260, 140
C   140 DETERM=-DETERM
C   150 DO 200 L=1,N
C   160 SWAP=A(IROW,L)
C   170 A(IROW,L)=A(ICOLUMN,L)
C   200 A(ICOLUMN,L)=SWAP
C   205 IF(M) 260, 260, 210
C   210 DO 250 L=1, M
C   220 SWAP=B(IROW,L)

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F4020084
F4020085

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ANF40201
F4020007
F4020008
F4020009
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F4020011
F4020012
F4020013
F4020014
F4020015
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F4020017
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F4020020
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F4020036
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F4020038
F4020039
F4020040
F4020041

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230 B(IROW,L)=B(ICOLUMN,L)
250 B(ICOLUMN,L)=SWAP
260 INDEX(I,1)=IROW
270 INDEX(I,2)=ICOLUMN
310 PIVOT(I)=A(ICOLUMN,ICOLUMN)
320 DETERM=DETERM*PIVOT(I)
C
C   DIVIDE PIVOT ROW BY PIVOT ELEMENT
C
330 A(ICOLUMN,ICOLUMN)=1.0
340 DO 350 L=1,N
350 A(ICOLUMN,L)=A(ICOLUMN,L)/PIVOT(I)
355 IF(M) 380, 380, 360
360 DO 370 L=1,M
370 B(ICOLUMN,L)=B(ICOLUMN,L)/PIVOT(I)
C
C   REDUCE NON-PIVOT ROWS
C
380 DO 550 L1=1,N
390 IF(L1-ICOLUMN) 400, 550, 400
400 T=A(L1,ICOLUMN)
420 A(L1,ICOLUMN)=0.0
430 DO 450 L=1,N
450 A(L1,L)=A(L1,L)-A(ICOLUMN,L)*T
455 IF(M) 550, 550, 460
460 DO 500 L=1,M
500 B(L1,L)=B(L1,L)-B(ICOLUMN,L)*T
550 CONTINUE
C
C   INTERCHANGE COLUMNS
C
600 DO 710 I=1,N
610 L=N+1-I
620 IF (INDEX(L,1)-INDEX(L,2)) 630, 710, 630
630 JROW=INDEX(L,1)
640 JCOLUMN=INDEX(L,2)
650 DO 705 K=1,N
660 SWAP=A(K,JROW)
670 A(K,JROW)=A(K,JCOLUMN)
700 A(K,JCOLUMN)=SWAP
F4020042
F4020043
F4020044
F4020045
F4020046
F4020047
F4020048
F4020049
F4020050
F4020051
F4020052
F4020053
F4020054
F4020055
F4020056
F4020057
F4020058
F4020059
F4020060
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F4020062
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F4020064
F4020065
F4020066
F4020067
F4020068
F4020069
F4020070
F4020071
F4020072
F4020073
F4020074
F4020075
F4020076
F4020077
F4020078
F4020079
F4020080
F4020081

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705 CONTINUE
710 CONTINUE
740 RETURN
750 END (2,2,2,2,0)
SUBROUTINE MXIV3 (A,N,B,M,DETERM)
MATRIX INVERSION WITH ACCOMPANYING SOLUTION OF LINEAR EQUATIONS
DIMENSION IPIVOT(24),A(24,24),B(24,1),INDEX(24,2),PIVOT(24)
EQUIVALENCE (IROW,JROW), (ICOLUMN,JCOLUMN), (AMAX, T, SWAP)
C
C INITIALIZATION
C
10 DETERM=1.0
15 DO 20 J=1,N
20 IPIVOT(J)=0
30 DO 550 I=1,N
C SEARCH FOR PIVOT ELEMENT
C
40 AMAX=0.0
45 DO 105 J=1,N
50 IF (IPIVOT(J)-1) 60, 105, 60
60 DO 100 K=1,N
70 IF (IPIVOT(K)-1) 80, 100, 740
80 IF (ABSF(AMAX)-ABSF(A(J,K))) 85, 100, 100
85 IROW=J
90 ICOLUMN=K
95 AMAX=A(J,K)
100 CONTINUE
105 CONTINUE
110 IPIVOT(ICOLUMN)=IPIVOT(ICOLUMN)+1
C INTERCHANGE ROWS TO PUT PIVOT ELEMENT ON DIAGONAL
C
130 IF (IROW-ICOLUMN) 140, 260, 140
140 DETERM=-DETERM
150 DO 200 L=1,N
160 SWAP=A(IROW,L)
170 A(IROW,L)=A(ICOLUMN,L)
200 A(ICOLUMN,L)=SWAP
205 IF(M) 260, 260, 210

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F4020082
F4020083
F4020084
F4020085
ANF40201
F4020007
F4020008
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F4020010
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F4020014
F4020015
F4020016
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F4020037
F4020038
F4020039

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210 DO 250 L=1, M
220 SWAP=B(IROW,L)
230 B(IROW,L)=B(ICOLUMN,L)
250 B(ICOLUMN,L)=SWAP
260 INDEX(I,1)=IROW
270 INDEX(I,2)=ICOLUMN
310 PIVOT(I)=A(ICOLUMN,ICOLUMN)
320 DETERM=DETERM*PIVOT(I)
C
C   DIVIDE PIVOT ROW BY PIVOT ELEMENT
C
330 A(ICOLUMN,ICOLUMN)=1.0
340 DO 350 L=1,N
350 A(ICOLUMN,L)=A(ICOLUMN,L)/PIVOT(I)
355 IF(M) 380, 380, 350
360 DO 370 L=1,M
370 B(ICOLUMN,L)=B(ICOLUMN,L)/PIVOT(I)
C
C   REDUCE NON-PIVOT ROWS
C
380 DO 550 L1=1,N
390 IF(L1-ICOLUMN) 400, 550, 400
400 T=A(L1,ICOLUMN)
420 A(L1,ICOLUMN)=0.0
430 DO 450 L=1,N
450 A(L1,L)=A(L1,L)-A(ICOLUMN,L)*T
455 IF(M) 550, 550, 460
460 DO 500 L=1,M
500 B(L1,L)=B(L1,L)-B(ICOLUMN,L)*T
550 CONTINUE
C
C   INTERCHANGE COLUMNS
C
600 DO 710 I=1,N
610 L=N+1-I
620 IF (INDEX(L,1)-INDEX(L,2)) 630, 710, 630
630 JROW=INDEX(L,1)
640 JCOLUMN=INDEX(L,2)
650 DO 705 K=1,N
660 SWAP=A(K,JROW)

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F4020040
F4020041
F4020042
F4020043
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F4020077
F4020078
F4020079

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```

670 A(K,JROW)=A(K,JCOLUMN)
700 A(K,JCOLUMN)=SWAP
705 CONTINUE
710 CONTINUE
740 RETURN
750 END (2,2,2,2,0)
* C
      CHAIN (3,8)
      DIMENSION GUESS(24,2),VECTOR(24,20),EIGVAL(20),NITER(10),
1     US(24,28),HH(24,50),NAKSR(10),NAKDR(10),FREQ(10),EL(24,80),
2     AM1(24,24),AM2(21,24),TEMP1(34,24),A33(34,24),STRESS(34),
3     XEL(24,80),XAM2(21,24),XAM1(24,24),AMT(24,96),TEMP2(34,24)
      COMMON EL,NMODE,E,RHO,H,D,ADP2,AM1,AM2,BARM,IOPT,A33,TEMP1
      EQUIVALENCE (EL(1),XEL(1)),(NMODE,NMODX),(E,XEX),(RHO,XRHO),
1     (H,XH),(D,XD),(ADP2,XADP2),(AM1,XAM1),(AM2,XAM2),(BARM,XBAM),
2     (IOPT,IXPT)
      DO 12 I=1,24
      DO 12 J=2,48,2
12  AMT(I,J) =0.
      DO 14 I=1,24
      K=1
      DO 14 J=1,47,2
      AMT(I,J) =EL(I,K)
14  K=K+1
      NM=24
      NC=2
      AITKEN=0.0
      NITRDP=350
      NITRSP=50
      EPSP=0.
      EPDP=0.
      MAXR=24
      NTAPE=4
      WRITE OUTPUT TAPE 6,57
57  FORMAT (1H1)
      CALL MITERS (AMT,NTAPE,NM,GUESS,0,NMODE,VECTOR,FIGVAL,NITER,
1     NITRSP,NITRDP,EPSP,FPDP,IR,US,HH,MAXR,NC,AITKEN,NAKSR,
2     NAKDR,6)
* C

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F4020080
F4020081
F4020082
F4020083
F4020084
F4020085
CHN3
CHN30001
CHN30002
CHN30003
CHN30004
CHN30005
CHN30006
CHN30007
CHN30008
CHN30009
CHN30010
CHN30011
CHN30012
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 CHN30064  
 CHN30065  
 CHN30066  
 MX15  
 MLTM  
 MLTM  
 MLTM  
 MX15  
 MLTM  
 MX15

```

PI=3.1415927
SMAS=RHO*H
IF(IOPT-2) 32,33,34
34 CALL EXIT
32 CONV=D/(ADP2*ADP2*SMAS)
GO TO 35
33 CONV=D/(ADP2*ADP2*BARM)
35 IF(NC-2) 1261,1265,1261
1265 NMODE =NMODE*2
1261 DO 1234 I=1,NMODE
EIGVAL(I) =1./EIGVAL(I)
FREQ(I)=SQRTF(ABSF(EIGVAL(I))*CONV)/(2.*PI)
1234 CONTINUE
WRITE OUTPUT TAPE 6,1235
1235 FORMAT(1H0,21H**FREQUENCY IN CPS** /)
WRITE OUTPUT TAPE 6,1236,(FREQ(I),I=1,NMODE)
1236 FORMAT(5X,5E18.6)
C
1246 WRITE OUTPUT TAPE 6,1246
FORMAT (///32H VALUES OF STRESS FUNCTIONS ARE //)
DO 1242 K=1,NMODE
IF(EIGVAL(K)-0.1E-20) 1242,1242,1240
1240 WRITE OUTPUT TAPE 6,1245,K,EIGVAL(K)
DO 1241 I=1,34
DO 1241 J=1,24
1241 TEMP2(I,J) =-(TEMP1(I,J)+EIGVAL(K)*A33(I,J))
CALL MLTX15 (34,24,TEMP2,24,1,VECTOR,K,STRESS)
WRITE OUTPUT TAPE 6,1243,(STRESS(IS),IS=1,34)
1242 CONTINUE
1245 FORMAT (9H FOR NO. 12,1H,3X,14HEIGENVALUE OF E15.6)
1243 FORMAT(6E18.6)
CALL EXIT
END
SUBROUTINE MLTX15 (N1,M1,A, N2,M2,B,KL,ABF)
C
C **FOR MULTIPLICATION OF REAL MATRICES
C AB = A * B
C DIMENSION A(34,24),B(24,20),AB(34,2),ABF(34)
C
C J=KL
  
```

```

10 DO 1 I=1,N1
    AB(I,1)=0.
    DO 1 K=1,M1
        PRODC=A(I,K)*B(K,J)
    1 AB(I,1)=AB(I,1)+PRODC
    DO 2 I=1,N1
    2 ABF(I)=AB(I,1)
    RETURN
    END
* DATA
+13 +03+36 +02+7854 -00+3 -00+103 +08+2588 -03 0001
+5 -01+9 -00+1712 -04 6 4 5 2 -00+103 +00+1286 +00+1286 +00 0002
1 6 12 +00+0 +00+0 +00+0 +00+0 +00+0 +00+0 +00 0003
+0 +00+0 +00+0 +00+0 +00+0 +00+0 +00+0 +00 0004
+0 +00 +00 +00 +00 +00 +00 +00 +00 0005
+1286 +00+1286 +00+1286 +00+1286 +00+1286 +00+1286 +00 0006
+1286 +00 +00 +00 +00 +00 +00 +00 +00 0007
+906 +00+906 +00+906 +00+906 +00+906 +00+906 +00 0008
+906 +00 +00 +00 +00 +00 +00 +00 +00 0009
+906 +00+0 +00+1286 +00+0 +00+0 +00+0 +00 0010
13 0 +00+0 +00+1286 +00+0 +00+0 +00+0 +00 0011
+906 +00+0 +00+1286 +00+0 +00+0 +00+0 +00 0012
19 1 +00+0 +00+1286 +00+0 +00+0 +00+0 +00 0013
+906 +00+0 +00+1286 +00+0 +00+0 +00+0 +00 0014
1 1 1 -00+00 -00 +04 +02 +02 +02 +02 18A
+00 +00+00 -00 +04 +02 +02 +02 +02 18B
7 19 1 +04+627 +04 +04 +04 +04 +04 19A
+627 2 3 1 +02+176 +02 +02 +02 +02 +02 19B
+176 7 19 1 +02+235 +02 +02 +02 +02 +02 20A
+235 +00+0 +00+106 +02+0 +02+0 +02+0 +02 20B
+0 +00+0 +00+106 +02+0 +02+0 +02+0 +02 21A
+0 +00+0 +00+106 +02+0 +02+0 +02+0 +02 21B
+84 +01+168 +02+84 +01+77 +01+77 +01+77 +01+77
1 2 3 4 5 6 1 1 2 3 4 5 6 1 71319 1 71319
8 7 36 8 67 13 66 25 1 31 11 32 12 33 11 41 67
9 2 10 7 67 8 36 9 67 14 66 26 1 32 11 33 12 34 11

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MLTM
MX15
MLTM
MLTM
MX15
MLTX
MX15
MLTM

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9	3	10	8	67	9	36	10	67	15	66	27	1	33	11	34	12	35	11	A2003A								
9	4	10	9	67	10	36	11	67	16	66	28	1	34	11	35	12	36	11	A2004A								
9	5	10	10	67	11	36	12	67	17	66	29	1	35	11	36	12	37	11	A2005A								
7	6	10	11	48	12	36	18	66	30	1	36	47	37	12					A2006A								
5	1	13	8	1	31	1	33	66	41	66									A2007A								
5	1	15	2	14	7	1	32	1	39	14									A2008A								
5	1	14	2	15	3	14	8	1	33	1									A2009A								
5	2	14	3	15	4	14	9	1	34	1									A2010A								
5	3	14	4	15	5	14	10	1	35	1									A2011A								
5	4	14	5	15	6	14	11	1	36	1									A2012A								
4	5	51	6	15	12	1	37	1											A2013A								
8	2	69	3	66	8	37	31	17	33	37	38	1	39	18	41	17			A2014A								
9	2	37	7	16	8	69	9	66	14	37	39	17	40	1	41	18	43	17	A2016A								
9	8	37	13	16	14	69	15	66	20	37	41	17	42	1	43	18	45	17	A2018A								
7	14	55	19	16	20	60	21	66	43	54	44	1	45	18					A2020A								
5	1	20	2	1	1	19	32	19	39	1									A2015A								
5	1	19	7	20	8	1	13	19	41	1									A2017A								
5	7	19	13	20	14	1	19	19	43	1									A2019A								
4	13	70	19	20	20	1	45	1											A2021A								
8	9	101	112	141	151	161	171	182	201	222	324	2	3	4	5	624	1	2	3	4	5	6	71319	1	71319	AA30001A	
16	2	22	8	21	14	22	46	27	47	24	48	27	52	26	53	23	54	26	55	28	58	27	59			A3001A	
24	6	27	65	25	71	25	77	28																			A3001B
16	3	22	9	21	15	22	47	27	48	24	49	27	52	28	53	26	54	23	55	26	56	28	59			A3002A	
27	6	24	61	27	66	25	72	25																			A3002B
16	4	22	10	21	16	22	48	27	49	24	50	27	53	28	54	26	55	23	56	26	57	28	60			A3003A	
27	6	24	62	27	67	25	73	25																			A3003B
15	5	22	11	21	17	22	49	27	50	24	51	27	54	28	55	26	56	53	57	26	61	27	62			A3004A	
24	63	27	68	25	74	25																					A3004B
12	6	22	12	21	18	22	50	61	51	24	55	62	56	60	57	23	62	61	63	24	69	25	75			A3005A	
25																											A3005B
15	8	22	14	21	20	22	47	25	52	27	53	24	54	27	58	26	59	52	60	26	61	28	64			A3006A	
27	65	24	66	27	78	28																					A3006B
15	9	22	15	21	21	22	48	25	53	27	54	24	55	27	58	28	59	26	60	52	61	26	62			A3007A	
28	65	27	66	24	67	27																					A3007B
15	10	22	16	21	22	22	49	25	54	27	55	24	56	27	59	28	60	26	61	52	62	26	63			A3008A	
28	66	27	67	24	68	27																					A3008B
14	11	22	17	21	23	22	50	25	55	27	56	24	57	27	60	28	61	26	62	56	63	26	67			A3009A	
27	68	24	69	27																							A3009B
11	12	22	18	21	24	22	51	25	56	61	57	24	61	62	62	60	63	52	68	61	69	24				A3010A	
11	14	57	20	21	53	59	58	61	59	58	60	61	64	26	65	23	66	26	67	28	79	28				A3011A	

11	15	57	21	21	54	59	59	61	60	58	61	61	64	28	65	26	66	23	67	26	68	28	A3012A	
11	16	57	22	21	55	59	60	61	61	58	62	61	65	28	66	26	67	23	68	26	69	28	A3013A	
10	17	57	23	21	56	59	61	61	62	58	63	61	66	28	67	26	68	53	69	26			A3014A	
8	18	57	24	21	57	59	62	50	63	58	67	62	68	60	69	23							A3015A	
8	7	64	9	30	32	30	34	64	52	66	54	1	70	1	72	66							A3023A	
8	8	64	10	30	33	30	35	64	53	66	55	1	71	1	73	66							A3024A	
8	9	64	11	30	34	30	36	64	54	66	56	1	72	1	74	66							A3025A	
8	10	64	12	30	35	30	37	64	55	66	57	1	73	1	75	66							A3026A	
8	11	64	12	30	36	30	37	64	56	66	57	1	74	1	75	66							A3027A	
1	69	1																					A3034A	
3	46	29	47	1	76	1																	A3028A	
3	46	1	47	29	48	1																	A3029A	
3	47	1	48	29	49	1																	A3030A	
3	48	1	49	29	50	1																	A3031A	
3	49	1	50	29	51	1																	A3032A	
2	50	38	51	29																			A3033A	
4	47	66	59	1	76	1	78	66															A3016A	
4	53	66	65	1	77	1	79	66															A3017A	
4	59	66	65	1	78	1	79	66															A3018A	
3	46	29	52	1	70	1																	A3019A	
25	7	31	8	35	9	35	10	35	11	35	12	31	46	1	47	63	48	63	49	63	50	63	51	A3021A
1	52	29	53	34	54	32	55	32	56	32	57	29	58	1	59	63	60	63	61	63	62	63	63	A3021B
1	77	33																						A3021C
3	52	1	58	29	64	1																		A3020A
19	19	31	20	35	21	35	22	35	23	35	24	31	58	38	59	71	60	71	61	71	62	71	63	A3022A
38	64	29	65	34	66	32	67	32	68	32	69	29	79	33										A3022B
1	2	3	4	5	6	7	8	9	10	11	12	13	14	15	16	17	18	19	20	21	22	23	24	AA10003A
16	1	8	2	4	3	6	7	2	8	5	13	3	25	3	31	5	32	2	33	5	38	6	39	A1001A
4	41	5	46	7	52	9	70	9																A1001B
16	1	4	2	8	3	4	4	6	7	5	8	2	9	5	14	3	26	3	32	5	33	2	34	A1002A
5	39	6	47	7	53	9	71	9																A1002B
16	1	6	2	4	3	8	4	4	5	6	8	5	9	2	10	5	15	3	27	3	33	5	34	A1003A
2	35	5	48	7	54	9	72	9																A1003B
16	2	6	3	4	4	4	8	5	4	6	9	5	10	2	11	5	16	3	28	3	34	5	35	A1004A
2	36	5	49	7	55	9	73	9																A1004B
15	3	6	4	4	5	45	6	4	10	5	11	2	12	5	17	3	29	3	35	5	36	2	37	A1005A
5	50	7	56	9	74	9																		A1005B
12	4	43	5	41	6	8	11	42	12	2	18	3	30	3	36	42	37	2	51	7	57	9	75	A1006A
9																								A1006B
16	1	2	2	5	7	8	8	4	9	6	13	2	14	5	19	3	32	3	39	5	40	6	41	A1007A

4 43	5 46	9 52	7 58	9	8	9	8	4 10	6 13	5 14	2 15	5 20	3 33	A1007B
16 1	5 2	2 3	5 7	4 8	8	9	4 10	4 10	6 13	5 14	2 15	5 20	3 33	A1008A
3 41	6 47	9 53	7 59	9	8	4 9	8 10	4 11	6 14	6 14	5 15	2 16	5 21	A1008B
16 2	5 3	2 4	5 7	6 8	4 9	4 10	8 10	4 11	6 14	6 14	5 15	2 16	5 21	A1009A
3 34	3 48	9 54	7 60	9	4 10	4 10	8 11	4 12	6 15	6 15	5 16	2 17	5 22	A1009B
16 3	5 4	2 5	5 8	6 9	4 10	8 11	4 12	4 12	6 15	6 15	5 16	2 17	5 22	A1010A
3 35	3 49	9 55	7 61	9	4 11	4 11	4 12	4 16	5 17	5 17	2 18	5 23	3 36	A1010B
15 4	5 5	2 6	5 9	6 10	4 11	4 11	4 12	4 16	5 17	5 17	2 18	5 23	3 36	A1011A
3 50	9 56	7 62	9	4 11	4 12	8 17	4 18	2 24	3 37	3 37	3 51	9 57	7 63	A1011B
12 5	4 2	10 4	3 11	4 12	8 17	4 17	4 18	2 24	3 37	3 37	3 51	9 57	7 63	A1012A
9	6	2	10 4	3 11	4 12	8 17	4 18	2 24	3 37	3 37	3 51	9 57	7 63	A1012B
15 1	3 7	2 8	5 13	4 14	4 15	6 19	2 20	5 41	5 41	5 41	5 42	6 43	4 45	A1013A
5 52	9 58	7 64	9	4 14	4 15	6 19	2 20	5 41	5 41	5 41	5 42	6 43	4 45	A1013B
15 2	3 7	5 8	2 9	5 13	4 14	4 14	4 15	4 16	6 19	6 19	5 20	2 21	5 43	A1014A
6 53	9 59	7 65	9	4 14	4 14	4 15	4 16	4 16	6 19	6 19	5 20	2 21	5 43	A1014B
15 3	3 8	5 9	2 10	5 13	6 14	4 15	4 16	4 16	4 17	4 17	6 20	5 21	2 22	A1015A
5 54	9 60	7 66	9	4 15	6 15	4 16	4 17	4 17	4 18	4 18	6 21	5 22	2 23	A1015B
15 4	3 9	5 10	2 11	5 14	6 15	4 16	4 17	4 17	4 18	4 18	6 21	5 22	2 23	A1016A
5 55	9 61	7 67	9	4 16	6 16	4 17	4 18	4 18	4 22	4 22	5 23	2 24	5 56	A1016B
14 5	3 10	5 11	2 12	5 15	6 16	4 17	4 18	4 18	4 22	4 22	5 23	2 24	5 56	A1017A
9 62	7 68	9	2 13	5 16	6 17	4 18	4 19	4 19	4 23	4 23	5 24	2 25	6 57	A1017B
11 6	3 11	4 12	2 14	5 17	6 18	4 19	4 20	4 20	4 24	4 24	6 25	7 69	9	A1018A
11 7	4 0	13 39	14 42	19 8	20 4	21 6	21 6	4 22	4 45	4 45	4 58	6 5	6 4	A1019A
11 8	4 0	13 42	14 39	15 42	19 4	20 8	21 4	22 4	4 45	4 45	6 59	6 5	6 7	A1020A
11 9	4 0	14 42	15 39	16 42	19 6	20 6	21 4	22 8	4 23	4 23	6 60	6 5	6 7	A1021A
11 10	4 0	15 42	16 39	17 42	20 6	21 6	22 4	23 8	4 24	4 24	6 61	6 5	6 7	A1022A
10 11	4 0	16 42	17 39	18 42	21 6	22 6	23 4	24 4	4 62	4 62	6 62	6 5	6 7	A1023A
8 12	4 0	17 49	18 39	22 43	23 4	24 8	25 8	26 6	4 62	4 62	6 65	6 5	6 7	A1024A

## SAMPLE RUN

Some of the preliminary computations for the arrangement of input data are shown here, together with the definitions illustrated in figures. Appendix I supplements further information needed for the use of the programs.

The sample run provided here is the segment of the full scale Instrument Unit with sandwiched panel. Its idealized configuration is shown in Figure 37. The unit mass of the panel is

$$\begin{aligned} \text{BARM} &= 0.2588 \times 10^{-3} \times (0.02 + 0.03) + \frac{3.1}{12 \times 144} \times 0.9 \times \frac{1}{386} \\ &= 0.1712 \times 10^{-4} \text{ (lb-sec}^2/\text{in}^2) \end{aligned}$$

A, B, PHIO, PNU, E, RHO, N, M, NMODE, CORE, and IOPT are defined in the list of definitions and also in Figure 37. Referring to Figure 38, the distance from c.g.'s of the stiffener to the center line of the panel is assumed to be small.

$$z_{S\phi} = \text{SZP} \approx 0$$

Along the edge parallel to the x-direction, similar stiffening effect is assumed due to the filler material used at the boundary to replace the honeycomb material.

The cross-sectional area and moment of inertia of the stiffener are calculated as follows:

$$\begin{aligned} A_{S\phi} &= (2)(1.0)(0.125) + (1.25)(0.1625) = 0.453 \text{ in.}^2 \\ I_{S\phi} &= 2 \left[ \left( \frac{1}{12} \right) (1)(0.125)^3 + (0.125)(1)(0.3875^2) \right] + \\ &\quad \left( \frac{1}{12} \right) (0.1625)(1.25)^3 = 0.0643 \text{ in.}^4 \end{aligned}$$

Here, symmetry about the center line of panel is assumed. Since the stiffeners are placed along the boundaries, the values calculated above ( $A_s$  and  $I_s$ ) are doubled for use in the computer program:

$$\begin{aligned} A_{Sx} &= A_{S\phi} = 0.906 \text{ in.}^2 \\ I_{Sx} &= I_{S\phi} = 0.1286 \text{ in.}^4 \\ z_{Sx} &\approx 0 \end{aligned}$$

The Instrument Unit segment is supported by six cantilever beam type springs as shown in Figure 5. For a quarter panel, two springs are assumed acting on stations 7 and 19 (see

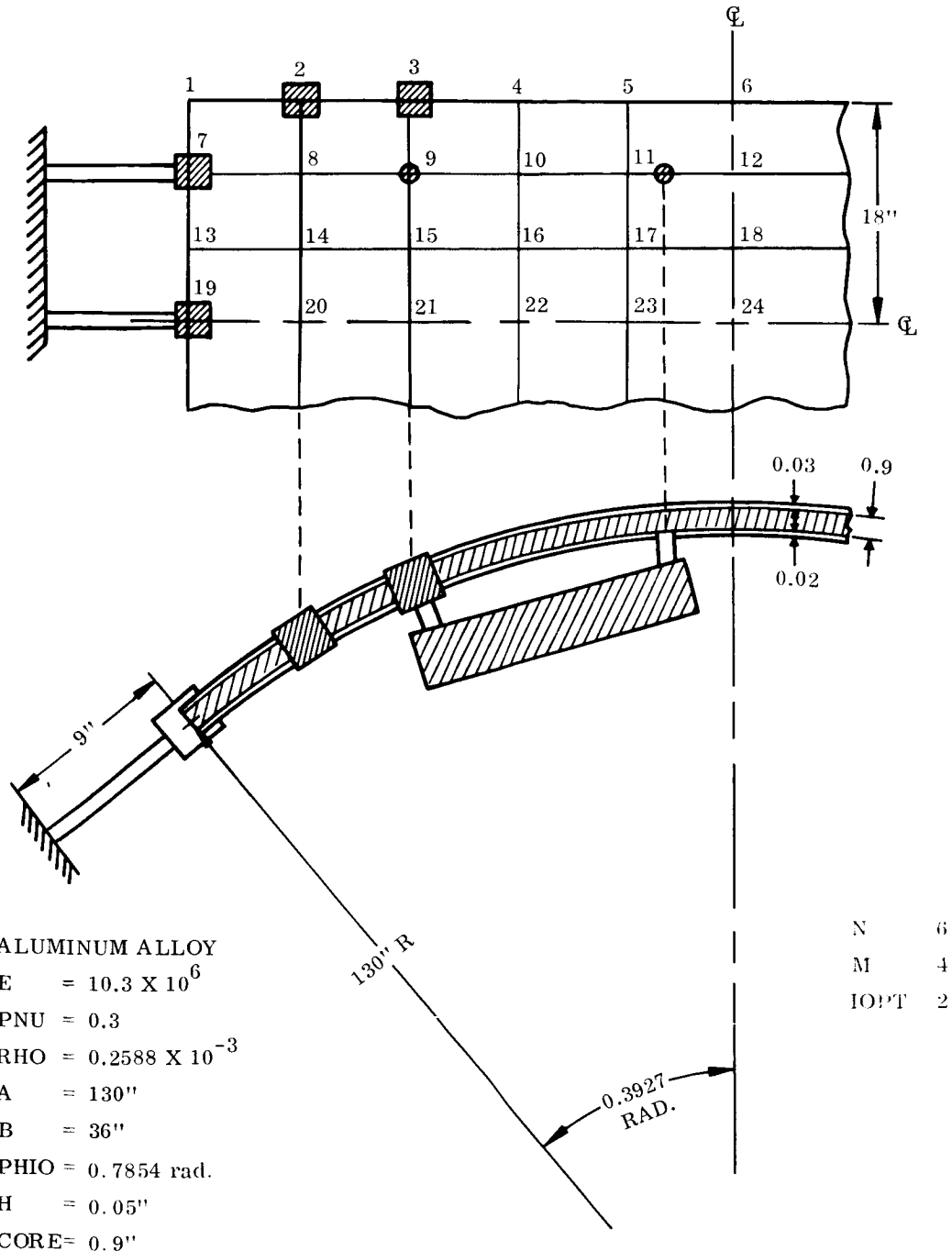


FIGURE 37. FULL SCALE INSTRUMENT UNIT LOCALIZED SHELL



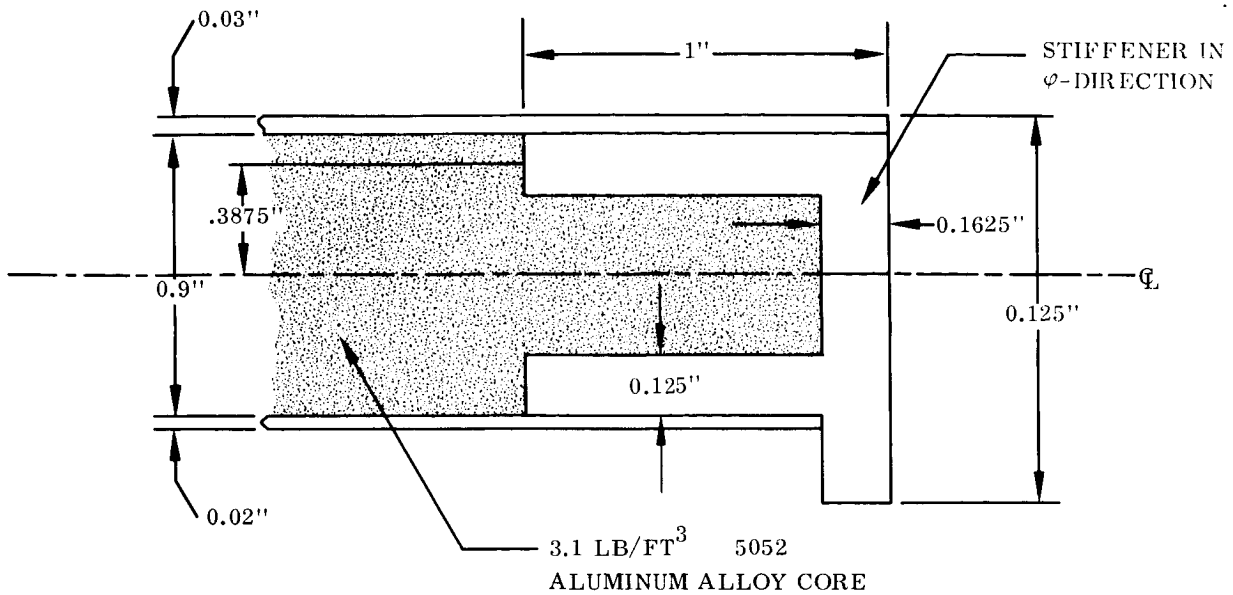


FIGURE 38. EDGE STIFFENER CONFIGURATION

Figure 37). The length of the cantilever supports is 9 inches. The corresponding spring constant,  $\bar{K}_\phi$ , is

$$\bar{K}_\phi(7) = \bar{K}_\phi(19) = \frac{3EI}{l^3} = \frac{3 \times 30 \times 10^6 \times 2.5 \times 0.625^3}{12 \times 9^3} = 6,270 \text{ lb./in.}$$

$$\bar{K}_x = 0.$$

For convenience in computation, the weights attached near the cantilever supports (see Figure 5) are assumed evenly distributed over the springs (Figure 37).

$$W_\phi(7) = W_\phi(19) = \frac{1}{3} \times 70.5 = 23.5 \text{ lb.}$$

Along the circumferential edges, boundary weights are attached at stations 2 and 3:

$$W_x(2) = W_x(3) = \frac{1}{2} \times 35.2 = 17.6 \text{ lb.}$$

The component-weight is assumed to be symmetrical with respect to both center lines of the I. U. segment, as shown in Figure 39. Considering the mass and mass moment of inertia of the component, if the attachment points A, B are both coinciding with the grid points, Eqs. (54) (55) may be used to compute its mass matrix elements. In the present case, point A falls on grid point 9. Point B is located between grid points 11, 12. The off-diagonal terms of  $M_1$  matrix, and the equivalent concentrated weights used at the points of attachment, are calculated as follows (see Figure 39).

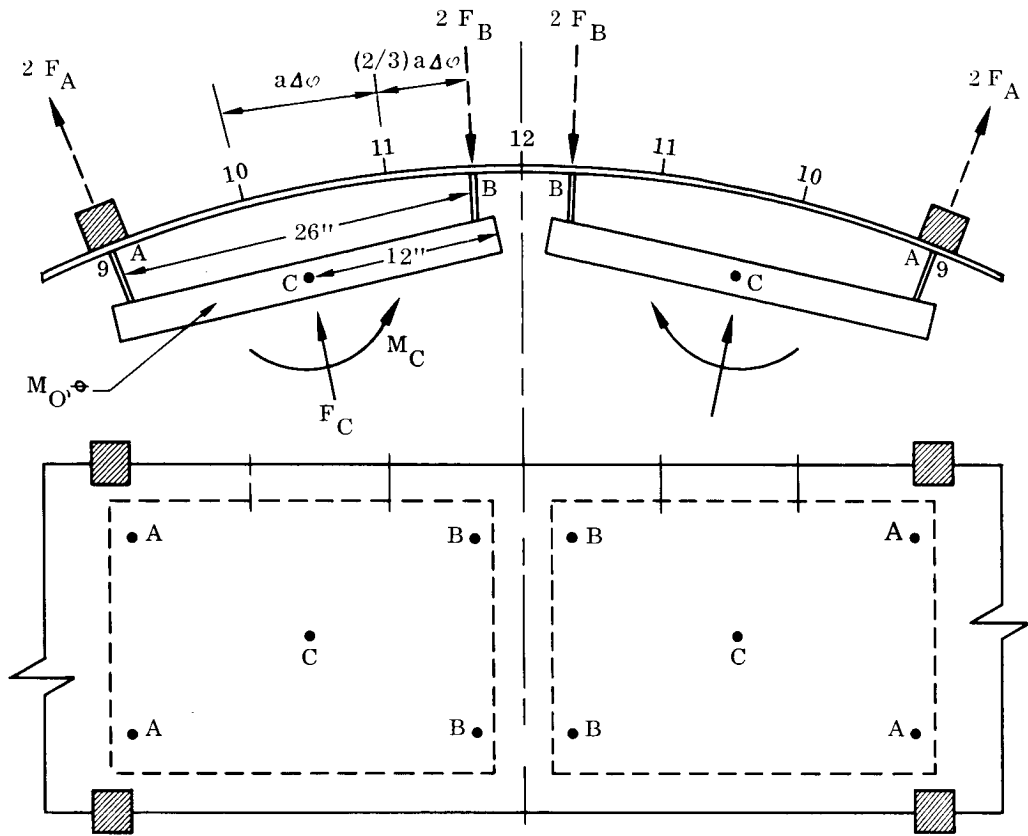


FIGURE 39. INERTIA FORCE DISTRIBUTION DUE TO A RIGID COMPONENT

$$M_o = \text{mass of the component} = \frac{90}{g} \text{ (lb. -sec.}^2\text{/in.)}$$

$$\ominus = \text{moment of inertia of the component about the axis through centroid in x-direction}$$

$$\approx 3.5 \text{ (in. -lb. -sec.}^2\text{)}$$

$$w_A = \text{deflection at station 9} = w_9$$

$$w_B = \frac{1}{3}(w_{11} + 2w_{12}) \text{ by assuming that } w_B \text{ is linearly proportional with } w_{11} \text{ and } w_{12}$$

$$w_C = \text{normal deflection at the centroid of the component}$$

$$= \frac{1}{26} (12w_A + 14w_B)$$

$$= \frac{1}{26} \left( 12w_9 + \frac{14}{3}w_{11} + \frac{28}{3}w_{12} \right)$$

$$F_A = \frac{1}{2} \left( \frac{12}{26} M_o \omega^2 w_C - \frac{1}{26} \ominus \omega^2 \frac{dw_C}{d\phi} \right)$$

$$= \text{inertia force acting at A due to } M_o \text{ and } \ominus$$

$$F_B = \frac{1}{2} \left( \frac{14}{26} M_o \omega^2 w_c + \frac{1}{26} \ominus \omega^2 \frac{dw_c}{ad\phi} \right)$$

= inertia force acting at B due to  $M_o$  and  $\ominus$

$$F_C = M_o \omega^2 w_c$$

$$M_C = \ominus \omega^2 \frac{dw_c}{ad\phi}$$

The inertia forces applied on the panel at points 9, 11 and 12 are

$$F_9 = F_A = \omega^2 (27.5 w_9 + 8.8 w_{11} + 17.6 w_{12}) \times 10^{-3} \text{ lb.}$$

$$F_{11} = \frac{1}{3} F_B = \omega^2 (8.8 w_9 + 4.05 w_{11} + 8.1 w_{12}) \times 10^{-3} \text{ lb.}$$

$$F_{12} = (2) \left( \frac{2}{3} \right) F_B = \omega^2 (35.2 w_9 + 16.2 w_{11} + 32.4 w_{12}) \text{ lb.}$$

Since the inertia force of the panel, as shown in the diagonal of  $M_1$  matrix, is unity per unit area, the above inertia forces must be normalized accordingly:

$$(M_1)_{9, 11} = \frac{\omega^2 \times 10^{-3}}{\bar{m} (\Delta x) (a\Delta\phi)\omega^2} \times 8.8 = 8.4$$

$$(M_1)_{9, 12} = \frac{\omega^2 \times 10^{-3}}{\bar{m} (\Delta x)(a\Delta\phi)\omega^2} \times 17.6 = 16.8$$

The equivalent concentrated weight attached at station 9 is

$$WT(9) = 10^{-3} \times 27.5 \times g = 10.6 \text{ lb.}$$

$$(M_1)_{11, 9} = \frac{\omega^2 \times 10^{-3}}{\bar{m} (\Delta x)(a\Delta\phi)\omega^2} \times 8.8 = 8.4$$

$$(M_1)_{11, 12} = \frac{\omega^2 \times 10^{-3}}{\bar{m} (\Delta x)(a\Delta\phi)\omega^2} \times 8.1 = 7.7$$

$$WT(11) = 10^{-3} \times 4.05 \times g = 1.56 \text{ lb}$$

$$(M_1)_{12, 9} = \frac{\omega^2 \times 10^{-3}}{\bar{m} (\Delta x)(a\Delta\phi)\omega^2} \times 35.2 = 33.6$$

$$(M_1)_{12, 11} = \frac{\omega^2 \times 10^{-3}}{\bar{m} (\Delta x)(a\Delta\phi)\omega^2} \times 16.2 = 15.4$$

$$WT(12) = 10^{-3} \times 32.4 \times g = 12.5 \text{ lb}$$

The non-zero elements of the  $M_1$  matrix are computed as follows:

	1	2	.....	9	10	11	12	13	.....
1	1				⋮				
		1			⋮				
			⋮		⋮				
					⋮				
				1					
9	.....	.....	.....	.....	27.2	8.4	16.8		
10						1			
11					8.4	4.85	7.7		
12					33.6	15.4	31.9		
13								1	
									⋮
									1

1 The computer printout of the sample run is given in the following pages.

FINITE DIFFERENCE METHOD FOR SANDWICHED SHELL, CORE THICKNESS = 0.90000E 00 MBAR = 0.17120E-04

\*\* INPUT DATA \*\*

** INPUT DATA **									
A	B	PHI0	NU	E	RHO	H	NMODE		
0.13000E 03	0.36000E 02	0.78540E 00	0.30000E-00	0.10300E 08	0.25880E-03	0.50000E-01	5		

\*\*AX\*\*  
 0.90600E 00  
 0.90600E 00  
 0.50600E 00  
 0.50600E 00

\*\*APHI\*\*  
 0.90600E 00  
 0.90600E 00  
 0.90600E 00  
 0.90600E 00

\*\*ZX\*\*  
 0.  
 0.  
 0.  
 0.

\*\*ZP\*\*  
 0.  
 0.  
 0.  
 0.

\*\*IX\*\*  
 0.12860E-00  
 0.12860E-00  
 0.12860E-00  
 0.12860E-00

\*\*IP\*\*  
 0.12860E-00  
 0.  
 0.  
 0.

FINITE DIFFERENCE METHOD FOR SANDWICHED SHELL, CORE THICKNESS = 0.90000E 00 MBAR = 0.17120E-04

\*\* INPUT DATA \*\*

```

** WX **
0. 0.17600E 02 0.17600E 02 0. 0. 0.

** WP **
0. 0. 0.17600E 02 0. 0. 0.
0.23500E 02 0. 0. 0. 0. 0.
0. 0. 0. 0. 0. 0.
0.23500E 02 0. 0. 0. 0. 0.

** WT **
0. 0. 0.10600E 02 0. 0. 0.12500E 02
0. 0. 0.15600E 01 0. 0. 0.
0. 0. 0. 0. 0. 0.
0. 0. 0. 0. 0. 0.

** KX **
0. 0. 0. 0. 0.

** KP **
0. 0. 0. 0. 0.
0.62700E 04 0. 0. 0. 0.
0. 0. 0. 0. 0.
0.62700E 04 0. 0. 0. 0.

```

FINITE DIFFERENCE METHOD FOR SANDWICHED SHELL, CORE THICKNESS = 0.90000E 00 MBAR = 0.17120E-04

MASS 1 MATRIX - INTERNAL      \*\* INPUT DATA \*\*

ROW NO. 1	0.	0.	0.	0.	0.	0.	0.	0.	0.
	0.10000E 01	0.	0.	0.	0.	0.	0.	0.	0.
	0.	0.	0.	0.	0.	0.	0.	0.	0.
	0.	0.	0.	0.	0.	0.	0.	0.	0.
	0.	0.	0.	0.	0.	0.	0.	0.	0.
ROW NO. 2	0.	0.	0.	0.	0.	0.	0.	0.	0.
	0.10000E 01	0.	0.	0.	0.	0.	0.	0.	0.
	0.	0.	0.	0.	0.	0.	0.	0.	0.
	0.	0.	0.	0.	0.	0.	0.	0.	0.
	0.	0.	0.	0.	0.	0.	0.	0.	0.
ROW NO. 3	0.	0.	0.	0.	0.	0.	0.	0.	0.
	0.10000E 01	0.	0.	0.	0.	0.	0.	0.	0.
	0.	0.	0.	0.	0.	0.	0.	0.	0.
	0.	0.	0.	0.	0.	0.	0.	0.	0.
	0.	0.	0.	0.	0.	0.	0.	0.	0.
ROW NO. 4	0.	0.	0.	0.	0.	0.	0.	0.	0.
	0.10000E 01	0.	0.	0.	0.	0.	0.	0.	0.
	0.	0.	0.	0.	0.	0.	0.	0.	0.
	0.	0.	0.	0.	0.	0.	0.	0.	0.
	0.	0.	0.	0.	0.	0.	0.	0.	0.
ROW NO. 5	0.	0.	0.	0.	0.	0.	0.	0.	0.
	0.10000E 01	0.	0.	0.	0.	0.	0.	0.	0.
	0.	0.	0.	0.	0.	0.	0.	0.	0.
	0.	0.	0.	0.	0.	0.	0.	0.	0.
	0.	0.	0.	0.	0.	0.	0.	0.	0.
ROW NO. 6	0.	0.	0.	0.	0.	0.	0.	0.	0.
	0.10000E 01	0.	0.	0.	0.	0.	0.	0.	0.
	0.	0.	0.	0.	0.	0.	0.	0.	0.
	0.	0.	0.	0.	0.	0.	0.	0.	0.
	0.	0.	0.	0.	0.	0.	0.	0.	0.
ROW NO. 7	0.	0.	0.	0.	0.	0.	0.	0.	0.
	0.10000E 01	0.	0.	0.	0.	0.	0.	0.	0.
	0.	0.	0.	0.	0.	0.	0.	0.	0.
	0.	0.	0.	0.	0.	0.	0.	0.	0.
	0.	0.	0.	0.	0.	0.	0.	0.	0.
ROW NO. 8	0.	0.	0.	0.	0.	0.	0.	0.	0.
	0.10000E 01	0.	0.	0.	0.	0.	0.	0.	0.
	0.	0.	0.	0.	0.	0.	0.	0.	0.
	0.	0.	0.	0.	0.	0.	0.	0.	0.
	0.	0.	0.	0.	0.	0.	0.	0.	0.

FINITE DIFFERENCE METHOD FOR SANDWICHED SHELL, CORE THICKNESS = 0.90000E 00 MBAR = 0.17120E-04

\*\* INPUT DATA \*\*

ROW NO.	9							
	0.	0.	0.	0.	0.	0.	0.	0.
	0.	0.	0.	0.	0.	0.	0.	0.
	0.	0.	0.	0.	0.	0.	0.	0.
	0.	0.	0.	0.	0.	0.	0.	0.
ROW NO.	10							
	0.	0.	0.	0.	0.	0.	0.	0.
	0.	0.	0.	0.	0.	0.	0.	0.
	0.	0.	0.	0.	0.	0.	0.	0.
	0.	0.	0.	0.	0.	0.	0.	0.
ROW NO.	11							
	0.	0.	0.	0.	0.	0.	0.	0.
	0.	0.	0.	0.	0.	0.	0.	0.
	0.	0.	0.	0.	0.	0.	0.	0.
	0.	0.	0.	0.	0.	0.	0.	0.
ROW NO.	12							
	0.	0.	0.	0.	0.	0.	0.	0.
	0.	0.	0.	0.	0.	0.	0.	0.
	0.	0.	0.	0.	0.	0.	0.	0.
	0.	0.	0.	0.	0.	0.	0.	0.
ROW NO.	13							
	0.	0.	0.	0.	0.	0.	0.	0.
	0.	0.	0.	0.	0.	0.	0.	0.
	0.	0.	0.	0.	0.	0.	0.	0.
	0.	0.	0.	0.	0.	0.	0.	0.
ROW NO.	14							
	0.	0.	0.	0.	0.	0.	0.	0.
	0.	0.	0.	0.	0.	0.	0.	0.
	0.	0.	0.	0.	0.	0.	0.	0.
	0.	0.	0.	0.	0.	0.	0.	0.
ROW NO.	15							
	0.	0.	0.	0.	0.	0.	0.	0.
	0.	0.	0.	0.	0.	0.	0.	0.
	0.	0.	0.	0.	0.	0.	0.	0.
	0.	0.	0.	0.	0.	0.	0.	0.
ROW NO.	16							
	0.	0.	0.	0.	0.	0.	0.	0.
	0.	0.	0.	0.	0.	0.	0.	0.
	0.	0.	0.	0.	0.	0.	0.	0.
	0.	0.	0.	0.	0.	0.	0.	0.





FINITE DIFFERENCE METHOD FOR SANDWICHED SHELL, CORE THICKNESS = 0.90000E 00 MBAR = 0.17120E-04

\*\* INPUT DATA \*\*

MASS 2 MATRIX - BOUNDARY									
ROW NO. 1	0.	0.	0.	0.	0.	0.	0.	0.	0.
	0.	0.	0.	0.	0.	0.	0.	0.	0.
	0.	0.	0.	0.	0.	0.	0.	0.	0.
	0.	0.	0.	0.	0.	0.	0.	0.	0.
ROW NO. 2	-0.10639E 02	0.	0.	0.	0.	0.	0.	0.	0.
	0.	0.	0.	0.	0.	0.	0.	0.	0.
	0.	0.	0.	0.	0.	0.	0.	0.	0.
	0.	0.	0.	0.	0.	0.	0.	0.	0.
	0.	0.	0.	0.	0.	0.	0.	0.	0.
ROW NO. 3	-0.10639E 02	0.	0.	0.	0.	0.	0.	0.	0.
	0.	0.	0.	0.	0.	0.	0.	0.	0.
	0.	0.	0.	0.	0.	0.	0.	0.	0.
	0.	0.	0.	0.	0.	0.	0.	0.	0.
	0.	0.	0.	0.	0.	0.	0.	0.	0.
ROW NO. 4	0.	0.	-0.27221E-00	0.	0.	0.	0.	0.	0.
	0.	0.	0.	0.	0.	0.	0.	0.	0.
	0.	0.	0.	0.	0.	0.	0.	0.	0.
	0.	0.	0.	0.	0.	0.	0.	0.	0.
	0.	0.	0.	0.	0.	0.	0.	0.	0.
ROW NO. 5	0.	0.	0.	0.	-0.27221E-00	0.	0.	0.	0.
	0.	0.	0.	0.	0.	0.	0.	0.	0.
	0.	0.	0.	0.	0.	0.	0.	0.	0.
	0.	0.	0.	0.	0.	0.	0.	0.	0.
	0.	0.	0.	0.	0.	0.	0.	0.	0.
ROW NO. 6	0.	0.	0.	0.	0.	0.	0.	-0.27221E-00	0.
	0.	0.	0.	0.	0.	0.	0.	0.	0.
	0.	0.	0.	0.	0.	0.	0.	0.	0.
	0.	0.	0.	0.	0.	0.	0.	0.	0.
	0.	0.	0.	0.	0.	0.	0.	0.	0.
ROW NO. 7	0.89392E 00	0.	0.	0.	0.	0.	0.	0.	0.
	0.	0.	0.	0.	0.	0.	0.	0.	0.
	0.	0.	0.	0.	0.	0.	0.	0.	0.
	0.	0.	0.	0.	0.	0.	0.	0.	0.
	0.	0.	0.	0.	0.	0.	0.	0.	0.
ROW NO. 8	0.	0.	0.	0.	0.	0.	0.	0.	0.
	0.	0.	0.	0.	0.	0.	0.	0.	0.
	0.	0.	0.	0.	0.	0.	0.	0.	0.
	0.	0.	0.	0.	0.	0.	0.	0.	0.
	0.	0.	0.	0.	0.	0.	0.	0.	0.
ROW NO. 9	0.	0.	0.	0.	0.	0.	0.	0.	0.
	0.	0.	0.	0.	0.	0.	0.	0.	0.
	0.	0.	0.	0.	0.	0.	0.	0.	0.
	0.	0.	0.	0.	0.	0.	0.	0.	0.
	0.	0.	0.	0.	0.	0.	0.	0.	0.

\*\* INPUT DATA \*\*

ROW NU. 10	0.	0.	0.	0.	0.	0.	0.	0.	0.	0.	0.	0.	0.	0.	0.	0.	0.	0.	0.	0.
	0.	0.	0.	0.	0.	0.	0.	0.	0.	0.	0.	0.	0.	0.	0.	0.	0.	0.	0.	0.
	0.	0.	0.	0.	0.	0.	0.	0.	0.	0.	0.	0.	0.	0.	0.	0.	0.	0.	0.	0.
	0.	0.	0.	0.	0.	0.	0.	0.	0.	0.	0.	0.	0.	0.	0.	0.	0.	0.	0.	0.
ROW NU. 11	0.	0.	0.	0.	0.	0.	0.	0.	0.	0.	0.	0.	0.	0.	0.	0.	0.	0.	0.	0.
	0.	0.	0.	0.	0.	0.	0.	0.	0.	0.	0.	0.	0.	0.	0.	0.	0.	0.	0.	0.
	0.	0.	0.	0.	0.	0.	0.	0.	0.	0.	0.	0.	0.	0.	0.	0.	0.	0.	0.	0.
	0.	0.	0.	0.	0.	0.	0.	0.	0.	0.	0.	0.	0.	0.	0.	0.	0.	0.	0.	0.
ROW NU. 12	0.	0.	0.	0.	0.	0.	0.	0.	0.	0.	0.	0.	0.	0.	0.	0.	0.	0.	0.	0.
	0.	0.	0.	0.	0.	0.	0.	0.	0.	0.	0.	0.	0.	0.	0.	0.	0.	0.	0.	0.
	0.	0.	0.	0.	0.	0.	0.	0.	0.	0.	0.	0.	0.	0.	0.	0.	0.	0.	0.	0.
	0.	0.	0.	0.	0.	0.	0.	0.	0.	0.	0.	0.	0.	0.	0.	0.	0.	0.	0.	0.
ROW NU. 13	0.	0.	0.	0.	0.	0.	0.	0.	0.	0.	0.	0.	0.	0.	0.	0.	0.	0.	0.	0.
	0.	0.	0.	0.	0.	0.	0.	0.	0.	0.	0.	0.	0.	0.	0.	0.	0.	0.	0.	0.
	0.	0.	0.	0.	0.	0.	0.	0.	0.	0.	0.	0.	0.	0.	0.	0.	0.	0.	0.	0.
	0.	0.	0.	0.	0.	0.	0.	0.	0.	0.	0.	0.	0.	0.	0.	0.	0.	0.	0.	0.
ROW NU. 14	0.	0.	0.	0.	0.	0.	0.	0.	0.	0.	0.	0.	0.	0.	0.	0.	0.	0.	0.	0.
	0.	0.	0.	0.	0.	0.	0.	0.	0.	0.	0.	0.	0.	0.	0.	0.	0.	0.	0.	0.
	0.	0.	0.	0.	0.	0.	0.	0.	0.	0.	0.	0.	0.	0.	0.	0.	0.	0.	0.	0.
	0.	0.	0.	0.	0.	0.	0.	0.	0.	0.	0.	0.	0.	0.	0.	0.	0.	0.	0.	0.
ROW NU. 15	0.	0.	0.	0.	0.	0.	0.	0.	0.	0.	0.	0.	0.	0.	0.	0.	0.	0.	0.	0.
	-0.11742E C3	0.	0.	0.	0.	0.	0.	0.	0.	0.	0.	0.	0.	0.	0.	0.	0.	0.	0.	0.
	0.	0.	0.	0.	0.	0.	0.	0.	0.	0.	0.	0.	0.	0.	0.	0.	0.	0.	0.	0.
	0.	0.	0.	0.	0.	0.	0.	0.	0.	0.	0.	0.	0.	0.	0.	0.	0.	0.	0.	0.
ROW NU. 16	0.	0.	0.	0.	0.	0.	0.	0.	0.	0.	0.	0.	0.	0.	0.	0.	0.	0.	0.	0.
	0.	0.	0.	0.	0.	0.	0.	0.	0.	0.	0.	0.	0.	0.	0.	0.	0.	0.	0.	0.
	0.	0.	0.	0.	0.	0.	0.	0.	0.	0.	0.	0.	0.	0.	0.	0.	0.	0.	0.	0.
	-0.13414E 01	0.	0.	0.	0.	0.	0.	0.	0.	0.	0.	0.	0.	0.	0.	0.	0.	0.	0.	0.
	0.	0.	0.	0.	0.	0.	0.	0.	0.	0.	0.	0.	0.	0.	0.	0.	0.	0.	0.	0.
ROW NU. 17	0.	0.	0.	0.	0.	0.	0.	0.	0.	0.	0.	0.	0.	0.	0.	0.	0.	0.	0.	0.
	0.	0.	0.	0.	0.	0.	0.	0.	0.	0.	0.	0.	0.	0.	0.	0.	0.	0.	0.	0.
	0.	0.	0.	0.	0.	0.	0.	0.	0.	0.	0.	0.	0.	0.	0.	0.	0.	0.	0.	0.
	-0.11742E 03	0.	0.	0.	0.	0.	0.	0.	0.	0.	0.	0.	0.	0.	0.	0.	0.	0.	0.	0.

\*\* INPUT DATA \*\*

ROW NU. 18	0.	0.	0.	0.	0.	0.	0.	0.
	0.	0.	0.	0.	0.	0.	0.	0.
	0.	0.	0.	0.	0.	0.	0.	0.
	0.	0.	0.	0.	0.	0.	0.	0.
ROW NU. 19	0.	0.	0.	0.	0.	0.	0.	0.
	0.	0.	0.	0.	0.	0.	0.	0.
	0.	0.	0.	0.	0.	0.	0.	0.
	0.	0.	0.	0.	0.	0.	0.	0.
ROW NU. 20	0.	0.	0.	0.	0.	0.	0.	0.
	0.	0.	0.	0.	0.	0.	0.	0.
	0.	0.	0.	0.	0.	0.	0.	0.
	0.	0.	0.	0.	0.	0.	0.	0.
ROW NU. 21	0.	0.	0.	0.	0.	0.	0.	0.
	0.	0.	0.	0.	0.	0.	0.	0.
	0.	0.	0.	0.	0.	0.	0.	0.
	0.	0.	0.	0.	0.	0.	0.	0.

FINITE DIFFERENCE METHOD FOR SANDWICHED SHELL, CORE THICKNESS = 0.90000E 00 MBAR = 0.17120E-04

\*\* INPUT DATA \*\*

** E TABLE **														
ROW NO. 1														
0.1000E 01	-0.8107E 02	0.1737E 02	-0.2288E 02	0.5792E 01	0.2824E 01	0.3836E-04	0.1443E 03							
-0.1918E-04	0.	0.2834E-00	-0.2567E 01	0.	0.5001E-01	-0.2100E 01	0.							
0.1743E 01	-0.5487E 01	0.3077E-00	-0.2615E 01	-0.2553E 08	0.1276E 08	0.4685E 03	-0.2449E 03							
0.2193E 02	-0.1722E 03	0.7861E 02	0.3748E 01	-0.2000E 01	0.6518E 03	0.5820E 06	-0.4000E 01							
-0.3962E-01	-0.3960E 01	0.1164E 07	0.2567E 01	-0.1743E 01	0.2000E 01	-0.1621E 03	0.3474E 02							
-0.4576E 02	0.1158E 02	0.5647E 01	0.1617E 03	0.1472E 03	0.1645E 03	0.5668E 00	-0.5668E 00							
0.2317E 02	0.3144E 03	0.1000E-00	0.4904E 03	0.4723E 03	0.3487E 01	-0.3487E 01	0.4942E 03							
0.2553E 08	-0.4899E 03	0.4386E 02	-0.3444E 03	0.1572E 03	0.7496E 01	0.2000E 01	-0.6518E 03							
-0.3836E-04	-0.1000E 01	-0.2834E-00	0.	0.5487E 01	0.6153E 00	0.4000E 01								
ROW NO. 2														
0.1000E 01	-0.4513E 02	0.8386E 01	-0.2288E 02	0.5792E 01	0.2824E 01	0.3836E-04	0.9042E 02							
-0.1918E-04	0.	0.5871E 00	-0.3174E 01	0.	0.1036E-00	-0.2207E 01	0.							
0.1743E 01	-0.5487E 01	0.3077E-00	-0.2615E 01	-0.9615E 07	0.4808E 07	0.1869E 03	-0.9060E 02							
0.8386E 01	-0.7205E 02	0.2853E 02	0.3748E 01	-0.2000E 01	0.6518E 03	0.5733E 06	-0.4000E 01							
-0.1036E-00	-0.3896E 01	0.1147E 07	0.3174E 01	-0.1743E 01	0.2000E 01	-0.9025E 02	0.1677E 02							
-0.4576E 02	0.1158E 02	0.5647E 01	0.9881E 02	0.9325E 02	0.1016E 03	0.1174E 01	-0.1174E 01							
0.2317E 02	0.1141E 03	0.2072E-00	0.1953E 03	0.1907E 03	0.3487E 01	-0.3487E 01	0.1991E 03							
0.9615E 07	-0.1812E 03	0.1677E 02	-0.1441E 03	0.5706E 02	0.7496E 01	0.2000E 01	-0.6518E 03							
-0.3836E-04	-0.1000E 01	-0.5871E 00	0.	0.5487E 01	0.6153E 00	0.4000E 01								
ROW NO. 3														
0.1000E 01	-0.4513E 02	0.8386E 01	-0.2288E 02	0.5792E 01	0.2824E 01	0.3836E-04	0.9042E 02							
-0.1918E-04	0.	0.5871E 00	-0.3174E 01	0.	0.1036E-00	-0.2207E 01	0.							
0.1743E 01	-0.5487E 01	0.3077E-00	-0.2615E 01	-0.9615E 07	0.4808E 07	0.1869E 03	-0.9060E 02							
0.8386E 01	-0.7205E 02	0.2853E 02	0.3748E 01	-0.2000E 01	0.6518E 03	0.5733E 06	-0.4000E 01							
-0.1036E-00	-0.3896E 01	0.1147E 07	0.3174E 01	-0.1743E 01	0.2000E 01	-0.9025E 02	0.1677E 02							
-0.4576E 02	0.1158E 02	0.5647E 01	0.9881E 02	0.9325E 02	0.1016E 03	0.1174E 01	-0.1174E 01							
0.2317E 02	0.1141E 03	0.2072E-00	0.1953E 03	0.1907E 03	0.3487E 01	-0.3487E 01	0.1991E 03							
0.9615E 07	-0.1812E 03	0.1677E 02	-0.1441E 03	0.5706E 02	0.7496E 01	0.2000E 01	-0.6518E 03							
-0.3836E-04	-0.1000E 01	-0.5871E 00	0.	0.5487E 01	0.6153E 00	0.4000E 01								
ROW NO. 4														
0.1000E 01	-0.4513E 02	0.8386E 01	-0.2288E 02	0.5792E 01	0.2824E 01	0.3836E-04	0.9042E 02							
-0.1918E-04	0.	0.5871E 00	-0.3174E 01	0.	0.1036E-00	-0.2207E 01	0.							
0.1743E 01	-0.5487E 01	0.3077E-00	-0.2615E 01	-0.9615E 07	0.4808E 07	0.1869E 03	-0.9060E 02							
0.8386E 01	-0.7205E 02	0.2853E 02	0.3748E 01	-0.2000E 01	0.6518E 03	0.5733E 06	-0.4000E 01							
-0.1036E-00	-0.3896E 01	0.1147E 07	0.3174E 01	-0.1743E 01	0.2000E 01	-0.9025E 02	0.1677E 02							
-0.4576E 02	0.1158E 02	0.5647E 01	0.9881E 02	0.9325E 02	0.1016E 03	0.1174E 01	-0.1174E 01							
0.2317E 02	0.1141E 03	0.2072E-00	0.1953E 03	0.1907E 03	0.3487E 01	-0.3487E 01	0.1991E 03							
0.9615E 07	-0.1812E 03	0.1677E 02	-0.1441E 03	0.5706E 02	0.7496E 01	0.2000E 01	-0.6518E 03							
-0.3836E-04	-0.1000E 01	-0.5871E 00	0.	0.5487E 01	0.6153E 00	0.4000E 01								

FINITE DIFFERENCE METHOD FOR SANDWICHED SHELL, CORE THICKNESS = 0.90000E 00 MBAR = 0.17120E-04

\*\* INPUT DATA \*\*

0.1743E 01	0.3077E-00	-0.2615E 01	0.1036E-00	-0.2207E 01	0.9060E 02
0.8386E 01	0.2853E 02	0.3748E 01	0.4808E 07	0.1869E 03	-0.4000E 01
-0.1036E-00	0.1147E 07	0.3174E 01	0.2000E 01	0.5733E 06	0.1677E 02
-0.4576E 02	0.5647E 01	0.9881E 02	0.1016E 03	0.9025E 02	-0.1174E 01
0.2317E 02	0.2072E-00	0.1953E 03	0.3487E 01	-0.3487E 01	0.1991E 03
0.9615E 07	0.1677E 02	-0.1441E 03	0.7496E 01	0.2000E 01	-0.6518E 03
-0.3836E-04	-0.5871E 00	0.	0.6153E 00	0.4000E 01	
NO. 6					
0.1000E 01	0.8386E 01	-0.2288E 02	0.2824E 01	0.3836E-04	0.9042E 02
-0.1918E-04	0.5871E 00	-0.3174E 01	0.1036E-00	-0.2207E 01	0.
0.1743E 01	0.3077E-00	-0.2615E 01	0.4808E 07	0.1869E 03	-0.9060E 02
0.8386E 01	0.2853E 02	0.3748E 01	0.6518E 03	0.5733E 06	-0.4000E 01
-0.1036E-00	0.1147E 07	0.3174E 01	0.2000E 01	-0.9025E 02	0.1677E 02
-0.4576E 02	0.5647E 01	0.9881E 02	0.1016E 03	0.1174E 01	-0.1174E 01
0.2317E 02	0.2072E-00	0.1953E 03	0.3487E 01	-0.3487E 01	0.1991E 03
0.9615E 07	0.1677E 02	-0.1441E 03	0.7496E 01	0.2000E 01	-0.6518E 03
-0.3836E-04	-0.5871E 00	0.	0.6153E 00	0.4000E 01	
NO. 7					
0.1000E 01	0.1737E 02	-0.1558E 02	0.1000E 01	0.3836E-04	0.1334E 03
-0.1918E-04	0.2834E-00	-0.2567E 01	0.5001E-01	-0.2100E 01	-0.1838E 02
0.4923E 01	0.8687E 00	-0.3737E 01	0.3318E 07	0.2142E 03	-0.1260E 03
0.2193E 02	0.1915E 02	0.1000E 01	0.6518E 03	0.1513E 06	-0.4000E 01
-0.3962E-01	0.3027E 06	0.2567E 01	0.2000E 01	-0.1621E 03	0.3474E 02
-0.3117E 02	0.2000E 01	0.1508E 03	0.1518E 03	0.5668E 00	-0.5668E 00
0.2317E 02	0.1000E-00	0.2361E 03	0.9846E 01	-0.9846E 01	0.2371E 03
0.6637E 07	0.4386E 02	-0.8461E 02	0.2000E 01	0.2000E 01	-0.6518E 03
-0.3836E-04	-0.2834E-00	0.	0.1737E 01	0.4000E 01	
NO. 8					
0.1000E 01	0.8386E 01	-0.1558E 02	0.1000E 01	0.3836E-04	0.7948E 02
-0.1918E-04	0.5871E 00	-0.3174E 01	0.1036E-00	-0.2207E 01	0.
0.4923E 01	0.8687E 00	-0.3737E 01	0.1196E 07	0.7948E 02	-0.4513E 02
0.8386E 01	0.5792E 01	0.1000E 01	0.6518E 03	0.1426E 06	-0.4000E 01
-0.1036E-00	0.2852E 06	0.3174E 01	0.2000E 01	-0.9025E 02	0.1677E 02
-0.3117E 02	0.2000E 01	0.8787E 02	0.8887E 02	0.1174E 01	-0.1174E 01
0.2317E 02	0.2072E-00	0.8787E 02	0.9846E 01	-0.9846E 01	0.8887E 02
0.2392E 07	0.1677E 02	-0.3117E 02	0.2000E 01	0.2000E 01	-0.6518E 03
-0.3836E-04	-0.5871E 00	0.	0.1737E 01	0.4000E 01	
NO. 9					
0.1000E 01	0.8386E 01	-0.1558E 02	0.1000E 01	0.3836E-04	0.7948E 02
-0.1918E-04	0.5871E 00	-0.3174E 01	0.1036E-00	-0.2207E 01	0.
0.4923E 01	0.8687E 00	-0.3737E 01	0.1196E 07	0.7948E 02	-0.4513E 02
0.8386E 01	0.5792E 01	0.1000E 01	0.6518E 03	0.1426E 06	-0.4000E 01
-0.1036E-00	0.2852E 06	0.3174E 01	0.2000E 01	-0.9025E 02	0.1677E 02
-0.3117E 02	0.2000E 01	0.8787E 02	0.8887E 02	0.1174E 01	-0.1174E 01
0.2317E 02	0.2072E-00	0.8787E 02	0.9846E 01	-0.9846E 01	0.8887E 02
0.2392E 07	0.1677E 02	-0.3117E 02	0.2000E 01	0.2000E 01	-0.6518E 03
-0.3836E-04	-0.5871E 00	0.	0.1737E 01	0.4000E 01	

FINITE DIFFERENCE METHOD FOR SANDWICHED SHELL, CORE THICKNESS = 0.90000E 00 MBAR = 0.17120E-04

\*\* INPUT DATA \*\*

ROW NO. 10	0.1000E 01	-0.4513E 02	0.8386E 01	-0.1558E 02	0.5792E 01	0.1000E 01	0.3836E-04	0.7948E 02
	-0.1918E-04	0.	0.5871E 00	-0.3174E 01	0.	0.1036E-00	-0.2207E 01	0.
	0.4923E 01	-0.1185E 02	0.8687E 00	-0.3737E 01	-0.2392E 07	0.1196E 07	0.7948E 02	-0.4513E 02
	0.8386E 01	-0.1558E 02	0.5792E 01	-0.1000E 01	-0.2000E 01	0.6518E 03	0.1426E 06	-0.4000E 01
	-0.1036E-00	-0.3896E 01	0.2852E 06	0.3174E 01	-0.4923E 01	0.2000E 01	-0.9025E 02	0.1677E 02
	-0.3117E 02	0.1158E 02	0.2000E 01	0.8787E 02	0.8048E 02	0.8887E 02	0.1174E 01	-0.1174E 01
	0.2317E 02	0.2317E 02	0.2072E-00	0.8787E 02	0.8048E 02	0.9846E 01	-0.9846E 01	0.8887E 02
	0.2392E 07	-0.9025E 02	0.1677E 02	-0.3117E 02	0.1158E 02	0.2000E 01	0.2000E 01	-0.6518E 03
	-0.3836E-04	-0.1000E 01	-0.5871E 00	0.	0.1185E 02	0.1737E 01	0.4000E 01	

ROW NO. 11

0.1000E 01	-0.4513E 02	0.8386E 01	-0.1558E 02	0.5792E 01	0.1000E 01	0.3836E-04	0.7948E 02
-0.1918E-04	0.	0.5871E 00	-0.3174E 01	0.	0.1036E-00	-0.2207E 01	0.
0.4923E 01	-0.1185E 02	0.8687E 00	-0.3737E 01	-0.2392E 07	0.1196E 07	0.7948E 02	-0.4513E 02
0.8386E 01	-0.1558E 02	0.5792E 01	-0.1000E 01	-0.2000E 01	0.6518E 03	0.1426E 06	-0.4000E 01
-0.1036E-00	-0.3896E 01	0.2852E 06	0.3174E 01	-0.4923E 01	0.2000E 01	-0.9025E 02	0.1677E 02
-0.3117E 02	0.1158E 02	0.2000E 01	0.8787E 02	0.8048E 02	0.8887E 02	0.1174E 01	-0.1174E 01
0.2317E 02	0.2317E 02	0.2072E-00	0.8787E 02	0.8048E 02	0.9846E 01	-0.9846E 01	0.8887E 02
0.2392E 07	-0.9025E 02	0.1677E 02	-0.3117E 02	0.1158E 02	0.2000E 01	0.2000E 01	-0.6518E 03
-0.3836E-04	-0.1000E 01	-0.5871E 00	0.	0.1185E 02	0.1737E 01	0.4000E 01	

ROW NO. 12

0.1000E 01	-0.4513E 02	0.8386E 01	-0.1558E 02	0.5792E 01	0.1000E 01	0.3836E-04	0.7948E 02
-0.1918E-04	0.	0.5871E 00	-0.3174E 01	0.	0.1036E-00	-0.2207E 01	0.
0.4923E 01	-0.1185E 02	0.8687E 00	-0.3737E 01	-0.2392E 07	0.1196E 07	0.7948E 02	-0.4513E 02
0.8386E 01	-0.1558E 02	0.5792E 01	-0.1000E 01	-0.2000E 01	0.6518E 03	0.1426E 06	-0.4000E 01
-0.1036E-00	-0.3896E 01	0.2852E 06	0.3174E 01	-0.4923E 01	0.2000E 01	-0.9025E 02	0.1677E 02
-0.3117E 02	0.1158E 02	0.2000E 01	0.8787E 02	0.8048E 02	0.8887E 02	0.1174E 01	-0.1174E 01
0.2317E 02	0.2317E 02	0.2072E-00	0.8787E 02	0.8048E 02	0.9846E 01	-0.9846E 01	0.8887E 02
0.2392E 07	-0.9025E 02	0.1677E 02	-0.3117E 02	0.1158E 02	0.2000E 01	0.2000E 01	-0.6518E 03
-0.3836E-04	-0.1000E 01	-0.5871E 00	0.	0.1185E 02	0.1737E 01	0.4000E 01	

ROW NO. 13

0.1000E 01	-0.8107E 02	0.1737E 02	-0.1558E 02	0.5792E 01	0.1000E 01	0.3836E-04	0.1334E 03
-0.1918E-04	0.	0.2834E-00	-0.2567E 01	0.	0.5001E-01	-0.2100E 01	-0.
0.4923E 01	-0.1185E 02	0.8687E 00	-0.3737E 01	-0.6637E 07	0.3318E 07	0.2142E 03	-0.1260E 03
0.2193E 02	-0.4231E 02	0.1915E 02	0.1000E 01	-0.2000E 01	0.6518E 03	0.1513E 06	-0.4000E 01
-0.3962E-01	-0.3960E 01	0.3027E 06	0.2567E 01	-0.4923E 01	0.2000E 01	-0.1621E 03	0.3474E 02
-0.3117E 02	0.1158E 02	0.2000E 01	0.1508E 03	0.1344E 03	0.1518E 03	0.5668E 00	-0.5668E 00
0.2317E 02	0.7661E 02	0.1000E-00	0.2361E 03	0.2152E 03	0.9846E 01	-0.9846E 01	0.2371E 03
0.6637E 07	-0.2520E 03	0.4386E 02	-0.8461E 02	0.3831E 02	0.2000E 01	0.2000E 01	-0.6518E 03
-0.3836E-04	-0.1000E 01	-0.2834E-00	0.	0.1185E 02	0.1737E 01	0.4000E 01	

ROW NO. 14

0.1000E 01	-0.4513E 02	0.8386E 01	-0.1558E 02	0.5792E 01	0.1000E 01	0.3836E-04	0.7948E 02
-0.1918E-04	0.	0.5871E 00	-0.3174E 01	0.	0.1036E-00	-0.2207E 01	0.
0.4923E 01	-0.1185E 02	0.8687E 00	-0.3737E 01	-0.2392E 07	0.1196E 07	0.7948E 02	-0.4513E 02
0.8386E 01	-0.1558E 02	0.5792E 01	-0.1000E 01	-0.2000E 01	0.6518E 03	0.1426E 06	-0.4000E 01
-0.1036E-00	-0.3896E 01	0.2852E 06	0.3174E 01	-0.4923E 01	0.2000E 01	-0.9025E 02	0.1677E 02
-0.3117E 02	0.1158E 02	0.2000E 01	0.8787E 02	0.8048E 02	0.8887E 02	0.1174E 01	-0.1174E 01
0.2317E 02	0.2317E 02	0.2072E-00	0.8787E 02	0.8048E 02	0.9846E 01	-0.9846E 01	0.8887E 02
0.2392E 07	-0.9025E 02	0.1677E 02	-0.3117E 02	0.1158E 02	0.2000E 01	0.2000E 01	-0.6518E 03
-0.3836E-04	-0.1000E 01	-0.5871E 00	0.	0.1185E 02	0.1737E 01	0.4000E 01	

FINITE DIFFERENCE METHOD FOR SANDWICHED SHELL, CORE THICKNESS = 0.90000E 00 MBAR = 0.17120E-04

\*\* INPUT DATA \*\*

ROW NU. 15  
 0.1000E 01 -0.4513E 02 0.8386E 01 -0.1558E 02 0.5792E 01 0.1000E 01 0.3836E-04 0.7948E 02  
 -0.1918E-04 0. 0.5871E 00 -0.3174E 01 0. 0.1036E-00 -0.2207E 01 0.  
 0.4923E 01 -0.1185E 02 0.8687E 00 -0.3737E 01 -0.2392E 07 0.1196E 07 0.7948E 02 -0.4513E 02  
 0.8386E 01 -0.1558E 02 0.5792E 01 0.1000E 01 -0.2000E 01 -0.6518E 03 -0.1426E 06 -0.4000E 01  
 -0.1936E-00 -0.3896E 01 0.2852E 06 0.3174E 01 -0.4923E 01 0.2000E 01 -0.9025E 02 0.1677E 02  
 -0.3117E 02 0.1158E 02 0.2000E 01 0.8787E 02 0.8048E 02 0.8887E 02 -0.1174E 01 -0.1174E 01  
 0.2317E 02 0.2317E 02 0.2072E-00 0.8787E 02 0.8048E 02 0.9846E 01 -0.9846E 01 0.8887E 02  
 0.2392E 07 -0.9025E 02 0.1677E 02 0.3117E 02 -0.1158E 02 0.2000E 01 0.2000E 01 -0.6518E 03  
 -0.3836E-04 -0.1000E 01 -0.5871E 00 0. 0.1185E 02 0.1737E 01 0.4000E 01 0.4000E 01

ROW NU. 16  
 0.1000E 01 -0.4513E 02 0.8386E 01 -0.1558E 02 0.5792E 01 0.1000E 01 0.3836E-04 0.7948E 02  
 -0.1918E-04 0. 0.5871E 00 -0.3174E 01 0. 0.1036E-00 -0.2207E 01 0.  
 J.4923E 01 -0.1185E 02 0.8687E 00 -0.3737E 01 -0.2392E 07 0.1196E 07 0.7948E 02 -0.4513E 02  
 0.8386E 01 -0.1558E 02 0.5792E 01 0.1000E 01 -0.2000E 01 -0.6518E 03 0.1426E 06 -0.4000E 01  
 -C.1036E-00 -0.3896E 01 0.2852E 06 0.3174E 01 -0.4923E 01 0.2000E 01 -0.9025E 02 0.1677E 02  
 -0.3117E 02 0.1158E 02 0.2000E 01 0.8787E 02 0.8048E 02 0.8887E 02 0.1174E 01 -0.1174E 01  
 0.2317E 02 0.2317E 02 0.2072E-00 0.8787E 02 0.8048E 02 0.9846E 01 -0.9846E 01 0.8887E 02  
 0.2392E 07 -0.9025E 02 0.1677E 02 0.3117E 02 -0.1158E 02 0.2000E 01 0.2000E 01 -0.6518E 03  
 -0.3836E-04 -0.1000E 01 -0.5871E 00 0. 0.1185E 02 0.1737E 01 0.4000E 01 0.4000E 01

ROW NU. 17  
 0.1000E 01 -0.4513E 02 0.8386E 01 -0.1558E 02 0.5792E 01 0.1000E 01 0.3836E-04 0.7948E 02  
 -0.1918E-04 0. 0.5871E 00 -0.3174E 01 0. 0.1036E-00 -0.2207E 01 0.  
 0.4923E 01 -0.1185E 02 0.8687E 00 -0.3737E 01 -0.2392E 07 0.1196E 07 0.7948E 02 -0.4513E 02  
 0.8386E 01 -0.1558E 02 0.5792E 01 0.1000E 01 -0.2000E 01 -0.6518E 03 0.1426E 06 -0.4000E 01  
 -0.1036E-00 -0.3896E 01 0.2852E 06 0.3174E 01 -0.4923E 01 0.2000E 01 -0.9025E 02 0.1677E 02  
 -0.3117E 02 0.1158E 02 0.2000E 01 0.8787E 02 0.8048E 02 0.8887E 02 0.1174E 01 -0.1174E 01  
 0.2317E 02 0.2317E 02 0.2072E-00 0.8787E 02 0.8048E 02 0.9846E 01 -0.9846E 01 0.8887E 02  
 0.2392E 07 -0.9025E 02 0.1677E 02 0.3117E 02 -0.1158E 02 0.2000E 01 0.2000E 01 -0.6518E 03  
 -0.3836E-04 -0.1000E 01 -0.5871E 00 0. 0.1185E 02 0.1737E 01 0.4000E 01 0.4000E 01

ROW NU. 18  
 0.1000E 01 -0.4513E 02 0.8386E 01 -0.1558E 02 0.5792E 01 0.1000E 01 0.3836E-04 0.7948E 02  
 -0.1918E-04 0. 0.5871E 00 -0.3174E 01 0. 0.1036E-00 -0.2207E 01 0.  
 0.4923E 01 -0.1185E 02 0.8687E 00 -0.3737E 01 -0.2392E 07 0.1196E 07 0.7948E 02 -0.4513E 02  
 0.8386E 01 -0.1558E 02 0.5792E 01 0.1000E 01 -0.2000E 01 -0.6518E 03 0.1426E 06 -0.4000E 01  
 -0.1036E-00 -0.3896E 01 0.2852E 06 0.3174E 01 -0.4923E 01 0.2000E 01 -0.9025E 02 0.1677E 02  
 -0.3117E 02 0.1158E 02 0.2000E 01 0.8787E 02 0.8048E 02 0.8887E 02 0.1174E 01 -0.1174E 01  
 0.2317E 02 0.2317E 02 0.2072E-00 0.8787E 02 0.8048E 02 0.9846E 01 -0.9846E 01 0.8887E 02  
 0.2392E 07 -0.9025E 02 0.1677E 02 0.3117E 02 -0.1158E 02 0.2000E 01 0.2000E 01 -0.6518E 03  
 -0.3836E-04 -0.1000E 01 -0.5871E 00 0. 0.1185E 02 0.1737E 01 0.4000E 01 0.4000E 01



FINITE DIFFERENCE METHOD FOR SANDWICHED SHELL, CORE THICKNESS = 0.90000E 00 MBAR = 0.17120E-04

\*\* INPUT DATA \*\*

ROW NO. 19  
 0.1000E 01 0.1737E 02 0.1558E 02 0.5792E 01 0.1000E 01 0.3836E-04 0.1334E 03  
 -0.1918E-04 0.2834E-00 0.2567E 01 0.3737E 01 0.5001E-01 -0.2100E 01 -0.1838E 02  
 0.4923E 01 0.8687E 00 0.3737E 01 -0.6637E 07 0.3318E 07 0.2142E 03 0.1260E 03  
 0.2193E 02 0.1915E 02 0.1000E 01 0.2000E 01 0.6518E 03 0.1513E 06 -0.4000E 01  
 -0.3962E-01 0.3027E 06 0.2567E 01 -0.4923E 01 -0.2000E 01 -0.1621E 03 0.3474E 02  
 -0.3117E 02 0.1158E 02 0.2000E 01 0.1344E 03 0.1518E 03 0.5668E 00 -0.5668E 00  
 0.2317E 02 0.7661E 02 0.1000E-00 0.2152E 03 0.9846E 01 -0.9846E 01 0.2371E 03  
 0.6637E 07 -0.2520E 03 0.4386E 02 -0.3831E 02 0.2000E 01 0.2000E 01 -0.6518E 03  
 -0.3836E-04 -0.1000E 01 -0.2834E-00 0.1185E 02 0.1737E 01 0.4000E 01

ROW NO. 20  
 C.1000E 01 0.8386E 01 0.1558E 02 0.5792E 01 0.1000E 01 0.3836E-04 0.7948E 02  
 -0.1918E-04 0.5871E 00 0.3174E 01 0.3737E 01 0.1036E-00 0.2207E 01 0.  
 0.4923E 01 0.8687E 00 0.3737E 01 -0.2392E 07 0.1196E 07 -0.7948E 02 -0.4513E 02  
 0.386E 01 0.5792E 01 0.1000E 01 -0.2000E 01 0.6518E 03 0.1426E 06 -0.4000E 01  
 -0.1036E-00 0.2852E 06 0.3174E 01 -0.4923E 01 0.2000E 01 -0.9025E 02 0.1677E 02  
 -0.3117E 02 0.1158E 02 0.8787E 02 0.8048E 02 0.8887E 02 0.1174E 01 -0.1174E 01  
 0.2317E 02 0.2072E-00 0.8787E 02 0.8048E 02 0.9846E 01 -0.9846E 01 0.8887E 02  
 0.2392E 07 -0.9025E 02 0.1677E 02 0.1158E 02 0.2000E 01 0.2000E 01 -0.6518E 03  
 -0.3836E-04 -0.1000E 01 -0.5871E 00 0.1185E 02 0.1737E 01 0.4000E 01

ROW NO. 21  
 0.1000E 01 0.8386E 01 0.1558E 02 0.5792E 01 0.1000E 01 0.3836E-04 0.7948E 02  
 -0.1918E-04 0.5871E 00 0.3174E 01 0.3737E 01 0.1036E-00 0.2207E 01 0.  
 0.4923E 01 0.8687E 00 0.3737E 01 -0.2392E 07 0.1196E 07 -0.7948E 02 -0.4513E 02  
 0.386E 01 0.5792E 01 0.1000E 01 -0.2000E 01 0.6518E 03 0.1426E 06 -0.4000E 01  
 -0.1036E-00 0.2852E 06 0.3174E 01 -0.4923E 01 0.2000E 01 -0.9025E 02 0.1677E 02  
 -0.3117E 02 0.1158E 02 0.8787E 02 0.8048E 02 0.8887E 02 0.1174E 01 -0.1174E 01  
 0.2317E 02 0.2072E-00 0.8787E 02 0.8048E 02 0.9846E 01 -0.9846E 01 0.8887E 02  
 0.2392E 07 -0.9025E 02 0.1677E 02 0.1158E 02 0.2000E 01 0.2000E 01 -0.6518E 03  
 -0.3836E-04 -0.1000E 01 -0.5871E 00 0.1185E 02 0.1737E 01 0.4000E 01

ROW NO. 22  
 0.1000E 01 0.8386E 01 0.1558E 02 0.5792E 01 0.1000E 01 0.3836E-04 0.7948E 02  
 -0.1918E-04 0.5871E 00 0.3174E 01 0.3737E 01 0.1036E-00 0.2207E 01 0.  
 0.4923E 01 0.8687E 00 0.3737E 01 -0.2392E 07 0.1196E 07 -0.7948E 02 -0.4513E 02  
 0.386E 01 0.5792E 01 0.1000E 01 -0.2000E 01 0.6518E 03 0.1426E 06 -0.4000E 01  
 -0.1036E-00 0.2852E 06 0.3174E 01 -0.4923E 01 0.2000E 01 -0.9025E 02 0.1677E 02  
 -0.3117E 02 0.1158E 02 0.8787E 02 0.8048E 02 0.8887E 02 0.1174E 01 -0.1174E 01  
 0.2317E 02 0.2072E-00 0.8787E 02 0.8048E 02 0.9846E 01 -0.9846E 01 0.8887E 02  
 0.2392E 07 -0.9025E 02 0.1677E 02 0.1158E 02 0.2000E 01 0.2000E 01 -0.6518E 03  
 -0.3836E-04 -0.1000E 01 -0.5871E 00 0.1185E 02 0.1737E 01 0.4000E 01

ROW NO. 23  
 C.1000E 01 0.8386E 01 0.1558E 02 0.5792E 01 0.1000E 01 0.3836E-04 0.7948E 02  
 -0.1918E-04 0.5871E 00 0.3174E 01 0.3737E 01 0.1036E-00 0.2207E 01 0.  
 0.4923E 01 0.8687E 00 0.3737E 01 -0.2392E 07 0.1196E 07 -0.7948E 02 -0.4513E 02  
 0.386E 01 0.5792E 01 0.1000E 01 -0.2000E 01 0.6518E 03 0.1426E 06 -0.4000E 01  
 -0.1036E-00 0.2852E 06 0.3174E 01 -0.4923E 01 0.2000E 01 -0.9025E 02 0.1677E 02  
 -0.3117E 02 0.1158E 02 0.8787E 02 0.8048E 02 0.8887E 02 0.1174E 01 -0.1174E 01  
 0.2317E 02 0.2072E-00 0.8787E 02 0.8048E 02 0.9846E 01 -0.9846E 01 0.8887E 02  
 0.2392E 07 -0.9025E 02 0.1677E 02 0.1158E 02 0.2000E 01 0.2000E 01 -0.6518E 03  
 -0.3836E-04 -0.1000E 01 -0.5871E 00 0.1185E 02 0.1737E 01 0.4000E 01



FINITE DIFFERENCE METHOD FOR SANDWICHED SHELL, CORE THICKNESS = 0.90000E 00 MBAR = 0.17120E-04

\*\* INPUT DATA \*\*

\*\* A21 MATRIX \*\*

** ROW 1**	0.	0.	0.	0.	0.	0.	0.	0.2567E 01	-0.2834E-00
	0.	0.	0.	-0.1000E 01	0.	0.	0.	0.	0.
	0.	0.	0.	0.	0.	0.	0.	0.	0.
** ROW 2**	-0.	0.	0.	0.	0.	0.	-0.5871E 00	0.	0.3174E 01
	0.	0.	0.	0.	0.	0.	0.	0.	0.
	0.	0.	0.	0.	0.	0.	-0.1000E 01	0.	0.
** ROW 3**	0.	-0.	0.	0.	0.	0.	0.	0.	-0.5871E 00
	0.	0.	0.	0.	0.	0.	0.	-0.1000E 01	0.
	0.	0.	0.	0.	0.	0.	0.	0.	0.
** ROW 4**	0.	0.	0.	0.	-0.	0.	0.	0.	0.
	-0.	0.	0.	0.	0.	0.	0.	0.	0.
	0.	0.	0.	0.	0.	0.	0.	0.	0.
** ROW 5**	0.	0.	0.	0.	0.	0.	0.	0.	0.
	-0.	0.	0.	0.	0.	0.	0.	0.	0.
	0.	0.	0.	0.	0.	0.	0.	0.	0.
** ROW 6**	0.	0.	0.	0.	0.	0.	0.	0.	0.
	-0.	0.	0.	0.	0.	0.	0.	0.	0.
	0.	0.	0.	0.	0.	0.	0.	0.	0.
** ROW 7**	0.	0.	0.	0.	0.	0.	0.	0.	0.
	0.	0.	0.	0.	0.	0.	0.	0.	0.
	0.	0.	0.	0.	0.	0.	0.	0.	0.
** ROW 8**	0.	0.	0.	0.	0.	0.	0.	0.	0.
	-0.	0.	0.	0.	0.	0.	0.	0.	0.
	0.	0.	0.	0.	0.	0.	0.	0.	0.
** ROW 9**	0.	0.	0.	0.	0.	0.	0.	0.	0.
	0.	0.	0.	0.	0.	0.	0.	0.	0.
	0.	0.	0.	0.	0.	0.	0.	0.	0.
** ROW 10**	0.	0.	0.	0.	0.	0.	0.	0.	0.
	0.	0.	0.	0.	0.	0.	0.	0.	0.
	0.	0.	0.	0.	0.	0.	0.	0.	0.

FINITE DIFFERENCE METHOD FOR SANDWICHED SHELL, CORE THICKNESS = 0.90000E 00 MBAR = 0.17120E-04

\*\* INPUT DATA \*\*

```

** ROW 11**
0. 0. 0.1000E 01 0.1036E-00 -0.2207E 01 0.1036E-00 0. 0. 0.
0. 0. 0. 0. 0. 0. 0. 0. 0.
0. 0. 0. 0. 0. 0. 0. 0. 0.

** ROW 12**
0. 0. 0. 0.1000E 01 0.1036E-00 -0.2207E 01 0.1036E-00 0. 0.
0. 0. 0. 0. 0. 0. 0. 0. 0.
0. 0. 0. 0. 0. 0. 0. 0. 0.

** ROW 13**
0. 0. 0. 0. 0.1000E 01 0.1036E-00 -0.2207E 01 0.1036E-00 0.
0. 0. 0. 0. 0. 0. 0. 0. 0.
0. 0. 0. 0. 0. 0. 0. 0. 0.

** ROW 14**
0. 0. 0. 0. 0.5487E 01 -0.1000E 01 0.2072E-00 -0.2207E 01 0.
0. 0. 0. 0. 0. 0. 0. 0. 0.
0. 0. 0. 0. 0. 0. 0. 0. 0.

** ROW 15**
0. 0. 0. -0.4923E 01 0. 0. 0. 0. 0. 0.1185E 02
-0.1000E 01 0. 0. 0. 0. -0.4923E 01 0. 0. 0.
0. 0. 0. 0. 0. 0. 0. 0. 0.

** ROW 16**
0. 0. 0. 0. 0. 0. 0. 0. 0. -0.4923E 01
-0.1000E 01 0. 0. 0. 0. 0. 0. 0. 0.
0. 0. 0. 0. 0. 0. 0. 0. 0.

** ROW 17**
0. 0. 0. 0. 0. 0. 0. 0. 0. 0.
0. 0. 0. 0. 0. 0. 0. 0. 0. 0.
0. 0. 0. 0. 0. 0. 0. 0. 0. 0.

** ROW 18**
0. 0. 0. 0. 0.1000E 01 0.1838E 02 0.1185E 02 0.3077E-00 0.1000E 01
-0.2615E 01 0. 0. 0. 0. 0. 0. 0. 0. 0.
0. 0. 0. 0. 0. 0. 0. 0. 0. 0.

** ROW 19**
0. 0. 0. 0. 0. 0. 0. 0. 0. 0.
0. 0. 0. 0. 0. 0. 0. 0. 0. 0.
0. 0. 0. 0. 0. 0. 0. 0. 0. 0.

** ROW 20**
0. 0. 0. 0. 0. 0. 0. 0. 0. 0.
0. 0. 0. 0. 0. 0. 0. 0. 0. 0.
0. 0. 0. 0. 0. 0. 0. 0. 0. 0.

** ROW 21**
0. 0. 0. 0. 0. 0. 0. 0. 0. 0.
0. 0. 0. 0. 0. 0. 0. 0. 0. 0.
0. 0. 0. 0. 0. 0. 0. 0. 0. 0.

```



FINITE DIFFERENCE METHOD FOR SANDWICHED SHELL, CORE THICKNESS = 0.90000E 00 MBAR = 0.17120E-04

\*\* INPUT DATA \*\*

```

** ROW 12**
  0.  0.  0.  0.  0.  0.  0.  0.  0.  0.  0.  0.  0.  0.  0.  0.  0.  0.  0.  0.
  0.  0.  0.  0.  0.  0.  0.  0.  0.  0.  0.  0.  0.  0.  0.  0.  0.  0.  0.
  0.  0.  0.  0.  0.  0.  0.  0.  0.  0.  0.  0.  0.  0.  0.  0.  0.  0.  0.

** ROW 13**
  0.  0.  0.  0.  0.  0.  0.  0.  0.  0.  0.  0.  0.  0.  0.  0.  0.  0.  0.
  0.  0.  0.  0.  0.  0.  0.  0.  0.  0.  0.  0.  0.  0.  0.  0.  0.  0.  0.
  0.  0.  0.  0.  0.  0.  0.  0.  0.  0.  0.  0.  0.  0.  0.  0.  0.  0.  0.

** ROW 14**
  0.  0.  0.  0.  0.  0.  0.  0.  0.  0.  0.  0.  0.  0.  0.  0.  0.  0.  0.
  0.  0.  0.  0.  0.  0.  0.  0.  0.  0.  0.  0.  0.  0.  0.  0.  0.  0.  0.
  0.  0.  0.  0.  0.  0.  0.  0.  0.  0.  0.  0.  0.  0.  0.  0.  0.  0.  0.

-0.1743E 01
  0.  0.  0.  0.  0.  0.  0.  0.  0.  0.  0.  0.  0.  0.  0.  0.  0.  0.  0.
  0.  0.  0.  0.  0.  0.  0.  0.  0.  0.  0.  0.  0.  0.  0.  0.  0.  0.  0.
  0.  0.  0.  0.  0.  0.  0.  0.  0.  0.  0.  0.  0.  0.  0.  0.  0.  0.  0.

** ROW 15**
  0.  0.  0.  0.  0.  0.  0.  0.  0.  0.  0.  0.  0.  0.  0.  0.  0.  0.  0.
  0.  0.  0.  0.  0.  0.  0.  0.  0.  0.  0.  0.  0.  0.  0.  0.  0.  0.  0.
  0.  0.  0.  0.  0.  0.  0.  0.  0.  0.  0.  0.  0.  0.  0.  0.  0.  0.  0.

-0.1185E 02
** ROW 16**
  0.  0.  0.  0.  0.  0.  0.  0.  0.  0.  0.  0.  0.  0.  0.  0.  0.  0.  0.
  0.  0.  0.  0.  0.  0.  0.  0.  0.  0.  0.  0.  0.  0.  0.  0.  0.  0.  0.
  0.  0.  0.  0.  0.  0.  0.  0.  0.  0.  0.  0.  0.  0.  0.  0.  0.  0.  0.

-0.4923E 01
** ROW 17**
  0.  0.  0.  0.  0.  0.  0.  0.  0.  0.  0.  0.  0.  0.  0.  0.  0.  0.  0.
  0.  0.  0.  0.  0.  0.  0.  0.  0.  0.  0.  0.  0.  0.  0.  0.  0.  0.  0.
  0.  0.  0.  0.  0.  0.  0.  0.  0.  0.  0.  0.  0.  0.  0.  0.  0.  0.  0.

0.4923E 01
** ROW 18**
  0.  0.  0.  0.  0.  0.  0.  0.  0.  0.  0.  0.  0.  0.  0.  0.  0.  0.  0.
  0.  0.  0.  0.  0.  0.  0.  0.  0.  0.  0.  0.  0.  0.  0.  0.  0.  0.  0.
  0.  0.  0.  0.  0.  0.  0.  0.  0.  0.  0.  0.  0.  0.  0.  0.  0.  0.  0.

0.9846E 01
** ROW 19**
  0.  0.  0.  0.  0.  0.  0.  0.  0.  0.  0.  0.  0.  0.  0.  0.  0.  0.  0.
  0.  0.  0.  0.  0.  0.  0.  0.  0.  0.  0.  0.  0.  0.  0.  0.  0.  0.  0.
  0.  0.  0.  0.  0.  0.  0.  0.  0.  0.  0.  0.  0.  0.  0.  0.  0.  0.  0.

0.1000E 01
** ROW 20**
  0.  0.  0.  0.  0.  0.  0.  0.  0.  0.  0.  0.  0.  0.  0.  0.  0.  0.  0.
  0.  0.  0.  0.  0.  0.  0.  0.  0.  0.  0.  0.  0.  0.  0.  0.  0.  0.  0.
  0.  0.  0.  0.  0.  0.  0.  0.  0.  0.  0.  0.  0.  0.  0.  0.  0.  0.  0.

0.1000E 01
** ROW 21**
  0.  0.  0.  0.  0.  0.  0.  0.  0.  0.  0.  0.  0.  0.  0.  0.  0.  0.  0.
  0.  0.  0.  0.  0.  0.  0.  0.  0.  0.  0.  0.  0.  0.  0.  0.  0.  0.  0.
  0.  0.  0.  0.  0.  0.  0.  0.  0.  0.  0.  0.  0.  0.  0.  0.  0.  0.  0.

0.1000E 01

```



\*\* INPUT DATA \*\*

```

** ROW 11**
0. 0. 0. 0. 0. 0. 0. 0. 0. 0.
0. 0. 0. 0. 0. 0. 0. 0. 0. 0.
0. 0. 0. -0.2392E 07 0. 0. 0. 0. 0. 0.

** ROW 12**
0. 0. 0. 0. 0. 0. 0. 0. 0. 0.
0. 0. 0. 0. 0. 0. 0. 0. 0. 0.
0. 0. 0. 0. 0. 0. 0. 0. 0. 0.

** ROW 13**
0. 0. 0. 0. 0. 0. 0. 0. 0. 0.
0. 0. 0. 0. 0. 0. 0. 0. 0. 0.
0. 0. 0. 0. 0. 0. 0. 0. 0. 0.

** ROW 14**
0. 0. 0. 0. 0. 0. 0. 0. 0. 0.
0. 0. 0. 0. 0. 0. 0. 0. 0. 0.
0. 0. 0. 0. 0. 0. 0. 0. 0. 0.

0.2392E 07
** ROW 15**
0. 0. 0. 0. 0. 0. 0. 0. 0. 0.
0. 0. 0. 0. 0. 0. 0. 0. 0. 0.
0. 0. 0. 0. 0. 0. 0. 0. 0. 0.

** ROW 16**
0. 0. 0. 0. 0. 0. 0. 0. 0. 0.
0. 0. 0. 0. 0. 0. 0. 0. 0. 0.
0. 0. 0. 0. 0. 0. 0. 0. 0. 0.

0.2392E 07
** ROW 17**
0. 0. 0. 0. 0. 0. 0. 0. 0. 0.
0. 0. 0. 0. 0. 0. 0. 0. 0. 0.
0. 0. 0. 0. 0. 0. 0. 0. 0. 0.

0.6518E 03
** ROW 18**
0. 0. 0. 0. 0. 0. 0. 0. 0. 0.
0. 0. 0. 0. 0. 0. 0. 0. 0. 0.
0. 0. 0. 0. 0. 0. 0. 0. 0. 0.

-0.6518E 03
** ROW 19**
0. 0. 0. 0. 0. 0. 0. 0. 0. 0.
0. 0. 0. 0. 0. 0. 0. 0. 0. 0.
0. 0. 0. 0. 0. 0. 0. 0. 0. 0.

-0.6518E 03
** ROW 20**
0. 0. 0. 0. 0. 0. 0. 0. 0. 0.
0. 0. 0. 0. 0. 0. 0. 0. 0. 0.
0. 0. 0. 0. 0. 0. 0. 0. 0. 0.

-0.6518E 03
** ROW 21**
0. 0. 0. 0. 0. 0. 0. 0. 0. 0.
0. 0. 0. 0. 0. 0. 0. 0. 0. 0.
0. 0. 0. 0. 0. 0. 0. 0. 0. 0.

```





FINITE DIFFERENCE METHOD FOR SANDWICHED SHELL, CORE THICKNESS = 0.90000E 00 MBAR = 0.17120E-04

\*\* INPUT DATA \*\*

```

** ROW 33**
0. 0. 0. 0. 0. 0. 0. 0.
0. 0. 0. 0. 0. 0. 0. 0.
0. 0. 0. 0. 0. 0. 0. 0.

** ROW 34**
0. 0. 0. 0. 0. 0. 0. 0.
0. 0. 0. 0. 0. 0. 0. 0.
0. 0.1513E 06 0.3027E 06 0.3027E 06 0.3027E 06 0.1513E 06

```

FINITE DIFFERENCE METHOD FOR SANDWICHED SHELL, CORE THICKNESS = 0.90000E 00 MBAR = 0.17120E-04

\*\* INPUT DATA \*\*

```

** A32 MATRIX **
** ROW 1**
0. 0. 0. 0. 0. 0. 0. 0. 0. 0. 0. 0.
0. 0. 0. 0. 0. 0. 0. 0. 0. 0. 0. 0.
0. 0. 0. 0. 0. 0. 0. 0. 0. 0. 0. 0.
** ROW 2**
0. 0. 0. 0. 0. 0. 0. 0. 0. 0. 0. 0.
0. 0. 0. 0. 0. 0. 0. 0. 0. 0. 0. 0.
0. 0. 0. 0. 0. 0. 0. 0. 0. 0. 0. 0.
** ROW 3**
0. 0. 0. 0. 0. 0. 0. 0. 0. 0. 0. 0.
0. 0. 0. 0. 0. 0. 0. 0. 0. 0. 0. 0.
0. 0. 0. 0. 0. 0. 0. 0. 0. 0. 0. 0.
** ROW 4**
0. 0. 0. 0. 0. 0. 0. 0. 0. 0. 0. 0.
0. 0. 0. 0. 0. 0. 0. 0. 0. 0. 0. 0.
0. 0. 0. 0. 0. 0. 0. 0. 0. 0. 0. 0.
** ROW 5**
0. 0. 0. 0. 0. 0. 0. 0. 0. 0. 0. 0.
0. 0. 0. 0. 0. 0. 0. 0. 0. 0. 0. 0.
0. 0. 0. 0. 0. 0. 0. 0. 0. 0. 0. 0.
** ROW 6**
0. 0. 0. 0. 0. 0. 0. 0. 0. 0. 0. 0.
0. 0. 0. 0. 0. 0. 0. 0. 0. 0. 0. 0.
0. 0. 0. 0. 0. 0. 0. 0. 0. 0. 0. 0.
** ROW 7**
0. 0. 0. 0. 0. 0. 0. 0. 0. 0. 0. 0.
0. 0. 0. 0. 0. 0. 0. 0. 0. 0. 0. 0.
0. 0. 0. 0. 0. 0. 0. 0. 0. 0. 0. 0.
** ROW 8**
0. 0. 0. 0. 0. 0. 0. 0. 0. 0. 0. 0.
0. 0. 0. 0. 0. 0. 0. 0. 0. 0. 0. 0.
0. 0. 0. 0. 0. 0. 0. 0. 0. 0. 0. 0.
** ROW 9**
0. 0. 0. 0. 0. 0. 0. 0. 0. 0. 0. 0.
0. 0. 0. 0. 0. 0. 0. 0. 0. 0. 0. 0.
0. 0. 0. 0. 0. 0. 0. 0. 0. 0. 0. 0.
** ROW 10**
0. 0. 0. 0. 0. 0. 0. 0. 0. 0. 0. 0.
0. 0. 0. 0. 0. 0. 0. 0. 0. 0. 0. 0.
0. 0. 0. 0. 0. 0. 0. 0. 0. 0. 0. 0.

```



FINITE DIFFERENCE METHOD FOR SANDWICHED SHELL, CORE THICKNESS = 0.90000E 00 MBAR = 0.17120E-04

\*\* INPUT DATA \*\*

\*\* ROW 33\*\*

0.	0.	0.	0.	0.	0.	0.	0.	0.	0.
0.	0.	0.	0.	0.	0.	0.	0.	0.	0.
0.	0.	0.	0.	0.	0.	0.	0.	0.	0.

\*\* ROW 34\*\*

0.	0.	0.	0.	0.	0.	0.	0.	0.	0.
0.	0.	0.	0.	0.	0.	0.	0.	0.	0.
0.	0.	0.	0.	0.	0.	0.	0.	0.	0.

\*\* INPUT DATA \*\*

```

** ROW 22**      0.      0.      0.      0.      0.      0.      0.      0.      0.      0.      0.      0.
0.      0.      0.      0.      0.      0.      0.      0.      0.      0.      0.      0.
0.      0.      0.      0.      0.      0.      0.      0.      0.      0.      0.      0.

** ROW 23**      0.      0.      0.      0.      0.      0.      0.      0.      0.      0.      0.      0.
0.      0.      0.      0.      0.      0.      0.      0.      0.      0.      0.      0.
0.      0.      0.      0.      0.      0.      0.      0.      0.      0.      0.      0.

** ROW 24**      0.      0.      0.      0.      0.      0.      0.      0.      0.      0.      0.      0.
0.      0.      0.      0.      0.      0.      0.      0.      0.      0.      0.      0.
0.      0.      0.      0.      0.      0.      0.      0.      0.      0.      0.      0.

** ROW 25**      0.      0.      0.      0.      0.      0.      0.      0.      0.      0.      0.      0.
0.      0.      0.      0.      0.      0.      0.      0.      0.      0.      0.      0.
0.      0.      0.      0.      0.      0.      0.      0.      0.      0.      0.      0.

** ROW 26**      0.      0.      0.      0.      0.      0.      0.      0.      0.      0.      0.      0.
0.      0.      0.      0.      0.      0.      0.      0.      0.      0.      0.      0.
0.      0.      0.      0.      0.      0.      0.      0.      0.      0.      0.      0.

** ROW 27**      0.      0.      0.      0.      0.      0.      0.      0.      0.      0.      0.      0.
0.      0.      0.      0.      0.      0.      0.      0.      0.      0.      0.      0.
0.      0.      0.      0.      0.      0.      0.      0.      0.      0.      0.      0.

** ROW 28**      0.      0.      0.      0.      0.      0.      0.      0.      0.      0.      0.      0.
0.      0.      0.      0.      0.      0.      0.      0.      0.      0.      0.      0.
0.      0.      0.      0.      0.      0.      0.      0.      0.      0.      0.      0.

** ROW 29**      0.      0.      0.      0.      0.      0.      0.      0.      0.      0.      0.      0.
0.      0.      0.      0.      0.      0.      0.      0.      0.      0.      0.      0.
0.      0.      0.      0.      0.      0.      0.      0.      0.      0.      0.      0.

** ROW 30**      0.      0.      0.      0.      0.      0.      0.      0.      0.      0.      0.      0.
0.      0.      0.      0.      0.      0.      0.      0.      0.      0.      0.      0.
0.      0.      0.      0.      0.      0.      0.      0.      0.      0.      0.      0.

** ROW 31**      0.      0.      0.      0.      0.      0.      0.      0.      0.      0.      0.      0.
0.      0.      0.      0.      0.      0.      0.      0.      0.      0.      0.      0.
0.      0.      0.      0.      0.      0.      0.      0.      0.      0.      0.      0.

** ROW 32**      0.      0.      0.      0.      0.      0.      0.      0.      0.      0.      0.      0.
0.      0.      0.      0.      0.      0.      0.      0.      0.      0.      0.      0.
0.      0.      0.      0.      0.      0.      0.      0.      0.      0.      0.      0.

```



FINITE DIFFERENCE METHOD FOR SANDWICHED SHELL, CORE THICKNESS = 0.90000E 00 MBAR = 0.17120E-04

\*\* INPUT DATA \*\*

```

** ROW 8**
0. 0. 0.8386E 01 0. 0. 0. 0. 0.8787E 02
0.5792E 01 -0.4513E 02 0.1000E 01 -0.1558E 02 0.8787E 02
-0.1558E 02 0.1000E 01 0.5792E 01 -0.4513E 02 0.
0. 0. 0. 0. 0.
0. 0. 0. 0. 0.

** ROW 9**
0. 0. 0.8386E 01 0. 0. 0. 0. 0.1558E 02
0.5792E 01 -0.4513E 02 0.5792E 01 0.1000E 01 -0.1558E 02
-0.1558E 02 0.5792E 01 0.5792E 01 0.1000E 01 0.5792E 01
0. 0. 0. 0. 0.

** ROW 10**
0. 0. 0.8386E 01 0. 0. 0. 0. 0.2000E 01
0. 0. 0.5792E 01 -0.4513E 02 0.8386E 01 0.1000E 01 0.2000E 01
-0.4513E 02 0.5792E 01 0.5792E 01 0.1000E 01 0.5792E 01
0. 0. 0. 0. 0.

** ROW 11**
0. 0. 0.8386E 01 0. 0. 0. 0. 0.4513E 02
0. 0. 0.5792E 01 -0.4513E 02 0.8386E 01 0.1000E 01 0.4513E 02
-0.4513E 02 0.5792E 01 0.5792E 01 0.1000E 01 0.5792E 01
0. 0. 0. 0. 0.

** ROW 12**
0. 0. 0.8386E 01 0. 0. 0. 0. 0.1677E 02
0. 0. 0.5792E 01 -0.4513E 02 0.8386E 01 0.1000E 01 0.1677E 02
-0.4513E 02 0.5792E 01 0.5792E 01 0.1000E 01 0.5792E 01
0. 0. 0. 0. 0.

** ROW 13**
0. 0. 0.8386E 01 0. 0. 0. 0. 0.9025E 02
0. 0. 0.5792E 01 -0.4513E 02 0.8386E 01 0.1000E 01 0.9025E 02
-0.4513E 02 0.5792E 01 0.5792E 01 0.1000E 01 0.9025E 02
0. 0. 0. 0. 0.

** ROW 14**
0. 0. 0.8386E 01 0. 0. 0. 0. 0.8048E 02
0. 0. 0.5792E 01 -0.4513E 02 0.8386E 01 0.1000E 01 0.8048E 02
-0.4513E 02 0.5792E 01 0.5792E 01 0.1000E 01 0.8048E 02
0. 0. 0. 0. 0.

```













FINITE DIFFERENCE METHOD FOR SANDWICHED SHELL, CORE THICKNESS = 0.90000E 00 MBAR = 0.17120E-04

\*\* INPUT DATA \*\*

```

** ROW 22**
0. 0. 0. 0. 0. 0. 0. 0. 0. 0. 0. 0. 0. 0. 0. 0. 0. 0. 0. 0.
0. 0.1677E 02 0. 0. 0. 0. 0. 0. 0. 0. 0. 0.1158E 02 0. 0.1158E 02 0. 0.9025E 02
0.1158E 02 0. 0.1000E 01 -0.1558E 02 0.7948E 02 -0.1558E 02 0.1000E 01
** ROW 23**
0. 0. 0. 0. 0. 0. 0. 0. 0. 0. 0. 0. 0. 0. 0. 0. 0. 0. 0. 0.
0. 0. 0.1677E 02 0. 0. 0. 0. 0. 0. 0. 0. 0. 0. 0. 0. 0.1158E 02
-0.9025E 02 0.1158E 02 0. 0.1000E 01 -0.1558E 02 0.8048E 02 -0.1558E 02
** ROW 24**
0. 0. 0. 0. 0. 0. 0. 0. 0. 0. 0. 0. 0. 0. 0. 0. 0. 0. 0. 0.
0. 0. 0. 0.1677E 02 0. 0. 0. 0. 0. 0. 0.1677E 02 0. 0. 0. 0.1158E 02
0.2317E 02 -0.9025E 02 0. 0. 0. 0. 0. 0. 0.2000E 01 -0.3117E 02 0.7948E 02

```



\*\* INPUT DATA \*\*

```

** ROW 13**
0. 0. 0. 0. 0. 0. 0. 0. 0. 0. 0. 0. 0. 0. 0. 0. 0. 0. 0. 0.
0. 0. 0. 0. 0. 0. 0. 0. 0. 0. 0. 0. 0. 0. 0. 0. 0. 0. 0. 0.
0.5792E 01 0.1000E 01 -0.1558E 02 0.1000E 01 0.5792E 01 0. 0. 0. 0. 0. 0.
** ROW 14**
0. 0. 0. 0. 0. 0. 0. 0. 0. 0. 0. 0. 0. 0. 0. 0. 0. 0. 0. 0.
0. 0. 0. 0.1000E 01 0. 0. 0. 0. 0. 0. 0. 0. 0. 0. 0. 0. 0. 0. 0. 0.
0. 0. 0. 0. 0. 0. 0. 0. 0. 0. 0. 0. 0. 0. 0. 0. 0. 0. 0. 0.
** ROW 15**
0. 0. 0. 0. 0. 0. 0. 0. 0. 0. 0. 0. 0. 0. 0. 0. 0. 0. 0. 0.
0. 0. 0. 0. 0. 0. 0. 0. 0. 0. 0. 0. 0. 0. 0. 0. 0. 0. 0. 0.
0. 0. 0. 0. 0. 0. 0. 0. 0. 0. 0. 0. 0. 0. 0. 0. 0. 0. 0. 0.
** ROW 16**
0. 0. 0. 0. 0. 0. 0. 0. 0. 0. 0. 0. 0. 0. 0. 0. 0. 0. 0. 0.
0. 0. 0. 0. 0. 0. 0. 0. 0. 0. 0. 0. 0. 0. 0. 0. 0. 0. 0. 0.
0. 0. 0. 0. 0. 0. 0. 0. 0. 0. 0. 0. 0. 0. 0. 0. 0. 0. 0. 0.
** ROW 17**
0. 0. 0. 0. 0. 0. 0. 0. 0. 0. 0. 0. 0. 0. 0. 0. 0. 0. 0. 0.
0. 0. 0. 0. 0. 0. 0. 0. 0. 0. 0. 0. 0. 0. 0. 0. 0. 0. 0. 0.
0. 0. 0. 0. 0. 0. 0. 0. 0. 0. 0. 0. 0. 0. 0. 0. 0. 0. 0. 0.
** ROW 18**
0. 0. 0. 0. 0. 0. 0. 0. 0. 0. 0. 0. 0. 0. 0. 0. 0. 0. 0. 0.
0. 0. 0. 0. 0. 0. 0. 0. 0. 0. 0. 0. 0. 0. 0. 0. 0. 0. 0. 0.
0. 0. 0. 0. 0. 0. 0. 0. 0. 0. 0. 0. 0. 0. 0. 0. 0. 0. 0. 0.
** ROW 19**
0. 0. 0. 0. 0. 0. 0. 0. 0. 0. 0. 0. 0. 0. 0. 0. 0. 0. 0. 0.
0. 0. 0. 0. 0. 0. 0. 0. 0. 0. 0. 0. 0. 0. 0. 0. 0. 0. 0. 0.
0. 0. 0. 0. 0. 0. 0. 0. 0. 0. 0. 0. 0. 0. 0. 0. 0. 0. 0. 0.
** ROW 20**
0. 0. 0. 0. 0. 0. 0. 0. 0. 0. 0. 0. 0. 0. 0. 0. 0. 0. 0. 0.
0. 0. 0. 0. 0. 0. 0. 0. 0. 0. 0. 0. 0. 0. 0. 0. 0. 0. 0. 0.
0. 0. 0. 0. 0. 0. 0. 0. 0. 0. 0. 0. 0. 0. 0. 0. 0. 0. 0. 0.
** ROW 21**
0. 0. 0. 0. 0. 0. 0. 0. 0. 0. 0. 0. 0. 0. 0. 0. 0. 0. 0. 0.
0. 0. 0. 0. 0. 0. 0. 0. 0. 0. 0. 0. 0. 0. 0. 0. 0. 0. 0. 0.
0. 0. 0. 0. 0. 0. 0. 0. 0. 0. 0. 0. 0. 0. 0. 0. 0. 0. 0. 0.
** ROW 22**
0. 0. 0. 0. 0. 0. 0. 0. 0. 0. 0. 0. 0. 0. 0. 0. 0. 0. 0. 0.
0. 0. 0. 0. 0. 0. 0. 0. 0. 0. 0. 0. 0. 0. 0. 0. 0. 0. 0. 0.
0. 0. 0. 0. 0. 0. 0. 0. 0. 0. 0. 0. 0. 0. 0. 0. 0. 0. 0. 0.

```



FINITE DIFFERENCE METHOD FOR SANDWICHED SHELL, CORE THICKNESS = 0.90000E 00 MBAR = 0.17120E-04

\*\* INPUT DATA \*\*

```

** RCh 23**
0. 0. 0. 0. 0. 0. 0. 0. 0. 0.
0. 0. 0. 0. 0. 0. 0. 0. 0. 0.
0. 0. 0. 0. 0. 0. 0. 0. 0. 0.

** RCh 24**
0. 0. 0. 0. 0. 0. 0. 0. 0. 0.
0. 0. 0. 0. 0. 0. 0. 0. 0. 0.
0. 0. 0. 0. 0. 0. 0. 0. 0. 0.

** A13 MATRIX **
** RCh 1**
0.3836E-04 0. 0. 0. 0. 0. 0. 0. 0. 0.
0. 0. 0. 0. 0. 0. 0. 0. 0. 0.
0. 0. 0. 0. 0. 0. 0. 0. 0. 0.
-0.1918E-04 0. 0. 0. 0. 0. 0. 0. 0. 0.

** RCh 2**
0. 0.3836E-04 0. 0. 0. 0. 0. 0. 0. 0.
0. 0. 0. 0. 0. 0. 0. 0. 0. 0.
0. 0. 0. 0. 0. 0. 0. 0. 0. 0.
-0.1918E-04 0. 0. 0. 0. 0. 0. 0. 0. 0.

** RCh 3**
0. 0. 0.3836E-04 0. 0. 0. 0. 0. 0. 0.
0. 0. 0. 0. 0. 0. 0. 0. 0. 0.
0. 0. 0. 0. 0. 0. 0. 0. 0. 0.
-0.1918E-04 0. 0. 0. 0. 0. 0. 0. 0. 0.

** RCh 4**
0. 0. 0. 0. 0.3836E-04 0. 0. 0. 0. 0.
0. 0. 0. 0. 0. 0. 0. 0. 0. 0.
0. 0. 0. 0. 0. 0. 0. 0. 0. 0.
-0.1918E-04 0. 0. 0. 0. 0. 0. 0. 0. 0.

** RCh 5**
0. 0. 0. 0. 0. 0.3836E-04 0. 0. 0. 0.
0. 0. 0. 0. 0. 0. 0. 0. 0. 0.
0. 0. 0. 0. 0. 0. 0. 0. 0. 0.
0. 0. 0. 0. 0. 0. 0. 0. 0. 0.
-0.1918E-04 0. 0. 0. 0. 0. 0. 0. 0. 0.

```







FINITE DIFFERENCE METHOD FOR SANDWICHED SHELL, CORE THICKNESS = 0.90000E 00 MBAR = 0.17120E-04

\*\* INPUT DATA \*\*

FINAL MATRIX

** ROW 1**	0.2885E-00	0.5906E 01	0.1873E 01	-0.3089E-01	-0.8800E-01	-0.5335E-01	0.6062E 01	0.7048E-01
	-0.9111E 00	-0.2300E-01	-0.5554E 00	-0.1040E 01	0.3324E-01	0.3335E-01	0.7738E-02	-0.2573E-01
** ROW 2**	-0.5118E-01	-0.3024E-01	0.1842E-00	0.1082E-01	0.1739E-02	-0.1323E-01	-0.2535E-01	-0.1493E-01
** ROW 3**	0.1517E-00	0.1105E 02	0.6762E 01	0.1512E-01	-0.1659E-00	-0.1165E-00	0.3091E 01	0.1553E-00
	-0.3981E-00	-0.7413E-02	-0.7061E 00	-0.1280E 01	0.1510E-01	0.1040E-00	0.7469E-01	-0.1845E-01
** ROW 4**	-0.1053E-00	-0.6972E-01	0.4061E-01	0.4396E-01	0.3262E-01	-0.1104E-01	-0.5300E-01	-0.3484E-01
** ROW 5**	0.4865E-01	0.6778E 01	0.9703E 01	0.1338E-00	-0.9031E-01	-0.8952E-01	0.6211E 00	0.1011E-00
	0.1267E 01	0.4222E-01	-0.5206E-01	-0.2740E-01	-0.6223E-02	0.7106E-01	0.9446E-01	0.1686E-01
** ROW 6**	-0.7925E-01	-0.6011E-01	-0.2754E-00	0.3064E-01	0.4009E-01	0.4174E-02	-0.4143E-01	-0.3054E-01
** ROW 7**	-0.2083E-01	0.4452E-00	0.3693E 01	0.2740E-00	0.1722E-00	-0.5688E-01	-0.1194E 01	-0.6859E-02
	0.2347E 01	0.9531E-01	0.9660E 00	0.1828E 01	-0.2619E-01	-0.1818E-01	0.1680E-01	0.5275E-01
** ROW 8**	0.3476E-01	0.9555E-02	-0.6452E 00	-0.1104E-01	0.4003E-02	0.1944E-01	0.1264E-01	0.3009E-02
** ROW 9**	-0.6087E-01	-0.4520E 01	-0.2458E 01	0.1725E-00	0.5057E 00	-0.2811E-00	-0.2304E 01	-0.9914E-01
	0.4447E 01	0.6312E-01	0.3477E 01	0.4766E 01	-0.4007E-01	-0.1003E-00	-0.7817E-01	0.3503E-01
** ROW 10**	0.1684E-00	0.1068E-00	-0.9244E 00	-0.5048E-01	-0.4107E-01	0.1268E-01	0.7561E-01	0.4921E-01
** ROW 11**	-0.7397E-01	-0.6361E 01	-0.4893E 01	0.1142E-00	0.5623E 00	-0.4544E-00	-0.2678E 01	-0.1345E-00
	0.6010E 01	0.4036E-01	0.3477E 01	0.6644E 01	-0.4500E-01	-0.1325E-00	-0.1185E-00	0.1957E-01
** ROW 12**	0.2137E-00	0.1623E-00	-0.1026E 01	-0.6611E-01	-0.6043E-01	0.6111E-02	0.9843E-01	0.7436E-01
** ROW 13**	0.1182E-00	0.2348E 01	0.4650E-00	-0.3342E-01	-0.6418E-01	-0.3727E-01	0.4975E 01	0.4297E-01
	-0.8435E 00	-0.2003E-01	-0.4556E-00	-0.8590E 00	0.7006E-01	0.3436E-01	0.5410E-02	-0.1999E-01
** ROW 14**	-0.3695E-01	-0.2144E-01	0.1351E 01	0.1519E-01	0.2255E-02	-0.9985E-02	-0.1834E-01	-0.1064E-01
** ROW 15**	0.8154E-01	0.7000E 01	0.4557E 01	-0.1136E-01	-0.1627E-00	-0.1103E-00	0.2547E 01	0.1469E-00
	-0.5436E 00	-0.1071E-01	-0.7190E 00	-0.1312E 01	0.3391E-01	0.1255E-00	0.8265E-01	-0.1394E-01
** ROW 16**	-0.9843E-01	-0.6552E-01	0.6531E 00	0.5812E-01	0.3946E-01	-0.7605E-02	-0.4936E-01	-0.3276E-01
** ROW 17**	0.2467E-01	0.4680E 01	0.6268E 01	0.6998E-01	-0.1106E-00	-0.9254E-01	0.4591E-00	0.9164E-01
	0.1244E 01	0.3659E-01	-0.5341E-01	-0.2821E-01	0.1506E-02	0.8133E-01	0.1177E-00	0.2886E-01
** ROW 18**	-0.7223E-01	-0.5696E-01	-0.3227E-01	0.3852E-01	0.5469E-01	0.1269E-01	-0.3682E-01	-0.2870E-01
** ROW 19**	-0.2607E-01	-0.3128E-00	0.1921E 01	0.1568E-00	0.1039E-00	0.3325E-01	-0.1180E 01	-0.1061E-01
	0.2041E 01	0.9642E-01	0.8436E 00	0.1591E 01	-0.2310E-01	-0.1405E-01	0.2862E-01	0.7857E-01
** ROW 20**	0.4815E-01	0.1368E-01	-0.5603E 00	-0.7794E-02	0.1246E-01	0.3520E-01	0.2211E-01	0.6163E-02
** ROW 21**	-0.5959E-01	-0.4617E 01	-0.3081E 01	0.1033E-00	0.3625E-00	0.2165E-00	-0.2226E 01	-0.9789E-01
	0.4142E 01	0.5682E-01	0.2396E 01	0.4492E 01	-0.3848E-01	-0.9695E-01	-0.7223E-01	0.4801E-01
** ROW 22**	0.1957E-00	0.1173E-00	-0.8899E 00	-0.4853E-01	-0.3694E-01	0.2197E-01	0.9315E-01	0.5691E-01

FINITE DIFFERENCE METHOD FOR SANDWICHED SHELL, CORE THICKNESS = 0.90000E 00 MBAR = 0.17120E-04

\*\* INPUT DATA \*\*

** ROW 12**	-0.7121E-01	-0.6266E 01	-0.5147E 01	0.6565E-01	0.4328E-00	0.3352E-00	-0.2585E 01	-0.1314E-00
	0.5882E 01	0.3385E-01	0.3402E 01	0.6520E 01	-0.4371E-01	-0.1289E-00	-0.1138E-00	0.2720E-01
	0.2345E-00	0.1855E-00	-0.1002E 01	-0.6427E-01	-0.5744E-01	0.1214E-01	0.1138E-00	0.8879E-01
** ROW 13**	0.3268E-01	0.5807E 00	-0.2335E-00	-0.3623E-01	-0.5550E-01	-0.3118E-01	0.3541E 01	0.2896E-01
	-0.8615E 00	-0.1960E-01	-0.4331E-00	-0.8205E 00	0.1162E-00	0.4241E-01	0.5823E-02	-0.1828E-01
	-0.3230E-01	-0.1847E-01	0.2817E 01	0.2244E-01	0.3532E-02	-0.8975E-02	-0.1615E-01	-0.9249E-02
** ROW 14**	0.3831E-01	0.4681E 01	0.3205E 01	-0.2938E-01	-0.1637E-00	-0.1082E-00	0.2031E 01	0.1254E-00
	-0.7662E 00	-0.1398E-01	-0.7781E 00	-0.1431E 01	0.4971E-01	0.1523E-00	0.9193E-01	-0.1112E-01
	-0.9611E-01	-0.6416E-01	0.1202E 01	0.7684E-01	0.4735E-01	-0.5155E-02	-0.4807E-01	-0.3212E-01
** ROW 15**	0.8736E-02	0.3361E 01	0.4259E 01	0.2792E-01	-0.1275E-00	-0.9681E-01	0.3164E-00	0.8256E-01
	0.8202E 00	0.2864E-01	-0.2371E-00	-0.3904E-00	0.6901E-02	0.9198E-01	0.1450E-00	0.3921E-01
	-0.6933E-01	-0.5634E-01	0.1471E-00	0.4706E-01	0.7348E-01	0.2094E-01	-0.3435E-01	-0.2819E-01
** ROW 16**	-0.2981E-01	-0.8304E 00	0.7662E 00	0.8688E-01	0.5790E-01	0.1629E-01	-0.1187E 01	-0.1398E-01
	0.1645E 01	0.7856E-01	0.6849E 00	0.1288E 01	-0.2133E-01	-0.1105E-01	0.3924E-01	0.1063E-00
	0.5889E-01	0.1616E-01	-0.5098E 00	-0.5205E-02	0.2088E-01	0.5415E-01	0.3078E-01	0.8432E-02
** ROW 17**	-0.5910E-01	-0.4737E 01	-0.3559E 01	0.5742E-01	0.2764E-00	0.1753E-00	-0.2191E 01	-0.9841E-01
	0.3619E 01	0.4815E-01	0.2148E 01	0.4028E 01	-0.3778E-01	-0.9602E-01	-0.6927E-01	0.5891E-01
	0.2257E-00	0.1276E-00	-0.8761E 00	-0.4785E-01	-0.3439E-01	0.3073E-01	0.1143E-00	0.6514E-01
** ROW 18**	-0.6978E-01	-0.6274E 01	-0.5402E 01	0.3179E-01	0.3505E-00	0.2661E-00	-0.2541E 01	-0.1310E-00
	0.5107E 01	0.2738E-01	0.3038E 01	0.5804E 01	-0.4323E-01	-0.1282E-00	-0.1126E-00	0.3234E-01
	0.2551E-00	0.2131E-00	-0.9949E 00	-0.6392E-01	-0.5640E-01	0.1680E-01	0.1303E-00	0.1076E-00
** ROW 19**	0.7069E-02	0.6478E-01	-0.4060E-00	-0.3491E-01	-0.5014E-01	-0.2786E-01	0.2699E 01	0.2209E-01
	-0.8386E 00	-0.1869E-01	-0.4103E-00	-0.7789E 00	0.1114E-00	0.4051E-01	0.4835E-02	-0.1735E-01
	-0.2969E-01	-0.1684E-01	0.3671E 01	0.2440E-01	0.3483E-02	-0.8513E-02	-0.1492E-01	-0.8480E-02
** ROW 20**	0.2458E-01	0.3954E-01	0.2767E 01	-0.3543E-01	-0.1644E-00	-0.1077E-00	0.1792E 01	0.1161E-00
	-0.8713E 00	-0.1539E-01	-0.8090E 00	-0.1493E 01	0.5267E-01	0.1537E-00	0.9408E-01	-0.1048E-01
	-0.9575E-01	-0.6394E-01	0.1450E 01	0.9217E-01	0.5181E-01	-0.4190E-02	-0.4783E-01	-0.3204E-01
** ROW 21**	0.3494E-02	0.2926E 01	0.3613E 01	0.1368E-01	-0.1337E-00	-0.9853E-01	0.2578E-00	0.7879E-01
	0.4281E-00	0.2504E-01	-0.3212E-00	-0.5562E 00	0.8461E-02	0.9483E-01	0.1470E-00	0.4178E-01
	-0.6876E-01	-0.5638E-01	0.2155E-00	0.5190E-01	0.8918E-01	0.2559E-01	-0.3342E-01	-0.2811E-01
** ROW 22**	-0.3104E-01	-0.1006E 01	0.3775E-00	0.6418E-01	0.4224E-01	0.1040E-01	-0.1192E 01	-0.1531E-01
	0.1470E 01	0.7047E-01	0.6152E 00	0.1155E 01	-0.2081E-01	-0.1011E-01	0.4202E-01	0.1084E-00
	0.6155E-01	0.1684E-01	-0.4937E-00	-0.4055E-02	0.2565E-01	0.6984E-01	0.3559E-01	0.9440E-02

FINITE DIFFERENCE METHOD FOR SANDWICHED SHELL, CORE THICKNESS = 0.90000E 00 MBAR = 0.17120E-04

\*\* INPUT DATA \*\*

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** ROW 23**
-0.5892E-01  -0.4782E 01  -0.3727E 01  0.4189E-01  0.2484E-00  0.1616E-00  -0.2180E 01  -0.9876E-01
 0.3388E 01  0.4430E-01  0.2038E 01  0.3825E 01  -0.3762E-01  -0.9590E-01  -0.6852E-01  0.6170E-01
 0.2286E-00  0.1303E-00  -0.8734E 00  -0.4768E-01  -0.3334E-01  0.3562E-01  0.1310E-00  0.6990E-01

** ROW 24**
-0.6930E-01  -0.6282E 01  -0.5497E 01  0.2017E-01  0.3231E-00  0.2440E-00  -0.2528E 01  -0.1311E-00
 0.4761E 01  0.2473E-01  0.2876E 01  0.5485E 01  -0.4313E-01  -0.1282E-00  -0.1125E-00  0.3387E-01
 0.2607E-00  0.2153E-00  -0.9944E 00  -0.6392E-01  -0.5614E-01  0.1891E-01  0.1398E-00  0.1234E-00
    
```

MODE	EIGENVALUE	ITERATIONS	S.P.	D.P.	AITKENS	S.P.	D.P.
1	0.19479618E 02	-0.	33	0	0	0	0
2	0.10531033E 02	0.	50	7	0	0	0
3	0.55531769E 01	0.	50	11	0	0	0
4	0.35102478E 01	-0.	33	0	0	0	0
5	0.18684153E 01	0.	24	0	0	0	0

EIGENVECTORS

	COLUMN 1	COLUMN 2	COLUMN 3	COLUMN 4	COLUMN 5	COLUMN 6
1	0.51858669E 00	0.	-0.26256932E-00	0.	0.82955208E 00	0.
2	0.09999999E 01	0.	0.58430018E-01	-0.	0.19711509E-00	0.
3	0.83876604E 00	0.	0.49089732E-00	-0.	-0.26645629E-00	-0.
4	0.11453288E-00	0.	0.57833327E 00	-0.	-0.53559955E-01	-0.
5	-0.57907083E 00	-0.	0.80327094E 00	-0.	0.58473975E 00	0.
6	-0.86853939E 00	-0.	0.09999999E 01	-0.	0.09999999E 01	0.
7	0.24821915E-00	0.	-0.32229170E-00	0.	0.80293347E 00	0.
8	0.68749375E 00	0.	-0.78744838E-01	0.	0.15819733E-00	0.
9	0.58132744E 00	0.	0.31116541E-00	-0.	-0.16970488E-00	-0.

FINITE DIFFERENCE METHOD FOR SANDWICHED SHELL, CORE THICKNESS = 0.90000E 00 MBAR = 0.17120E-04

\*\* INPUT DATA \*\*

	COLUMN 7	COLUMN 8	COLUMN 9	COLUMN 10
10	0.63967638E-02	0.	0.43887148E-00	-0.35012328E-01
11	-0.59767152E 00	-0.	0.71052743E 00	0.55494273E 00
12	-0.86276652E 00	-0.	0.94592635E 00	0.98118098E 00
13	0.10783558E-00	0.	-0.366C2819E-00	0.77532082E 00
14	0.50513947E 00	0.	-0.18002120E-00	0.11831706E-00
15	0.43169130E-00	0.	0.14372659E-00	-0.18593792E-00
16	-0.62290946E-01	-0.	0.32419031E-00	-0.55626855E-01
17	-0.60960596E 00	-0.	0.60354750E 00	0.48073553E-00
18	-0.85156702E 00	-0.	0.81131788E 00	0.85065921E 00
19	0.62582181E-01	0.	-0.37343035E-00	0.76783439E 00
20	0.44704827E-00	0.	-0.21693903E-00	0.10090122E-00
21	0.36371947E-00	0.	0.81322035E-01	-0.20127231E-00
22	-0.84884894E-01	-0.	0.28060313E-00	-0.68956271E-01
23	-0.61312911E 00	-0.	0.55999829E 00	0.44659440E-00
24	-0.84668494E 00	-0.	0.75397815E 00	0.79207366E 00
	COLUMN 7	COLUMN 8	COLUMN 9	COLUMN 10
1	-0.87628299E 00	0.	0.09999999E 01	
2	-0.79207735E 00	0.	-0.46350756E-00	
3	0.74732616E 00	-0.	0.30384699E-00	
4	0.50810593E 00	-0.	0.32387317E-00	
5	-0.20617031E-00	0.	0.36590583E-01	
6	-0.56951397E 00	0.	-0.15891776E-00	
7	-0.20692931E-01	0.	0.62807617E 00	
8	-0.16672950E-00	0.	-0.34258884E-00	
9	0.56158400E 00	-0.	0.66472695E-01	
10	0.32397218E-00	0.	0.18065070E-00	
11	-0.29312421E-00	0.	-0.23559903E-01	
12	-0.62014367E 00	0.	-0.20037647E-00	
13	0.67312041E 00	-0.	-0.27697901E-00	
14	0.23614565E-00	-0.	-0.45026598E-00	
15	0.48143043E-00	-0.	-0.65837588E-01	
16	0.21075475E-00	0.	0.99834444E-01	
17	-0.3475454E-00	0.	-0.41262345E-01	
18	-0.63931520E 00	0.	-0.18150035E-00	
19	0.09999999E 01	-0.	-0.87648887E 00	
20	0.38168788E-00	-0.	-0.54694762E 00	
21	0.46102938E-00	-0.	-0.11268654E-00	
22	0.17300409E-00	-0.	0.74506169E-01	
23	-0.36566363E-00	0.	-0.43532053E-01	
24	-0.64436060E 00	0.	-0.16918479E-00	



FINITE DIFFERENCE METHOD FOR SANDWICHED SHELL, CORE THICKNESS = 0.900000E 00 MBAR = 0.17120E-04

\*\* INPUT DATA \*\*

CHECK EIGENVALUES AND EIGENVECTORS

	COLUMN 1	COLUMN 2	COLUMN 3	COLUMN 4	COLUMN 5	COLUMN 6
0.	0.19479618E 02 -0.	0.10531032E 02 0.	-0.26256931E-00	0.55531759E 01 0.	0.82955215E 00	-0.
0.	0.35102473E 01 -0.	0.18684155E 01 -0.	0.58430031E-01		0.19711513E-00	-0.
1	0.5185869E 00	0.	0.58430031E-01		0.19711513E-00	-0.
2	0.0999999E 01 0.	0.	0.49089731E-00		-0.26645630E-00	0.
3	0.83876604E 00 0.	0.	0.57833328E 00		-0.53559976E-01	0.
4	0.11453288E-00 0.	0.	0.80327093E 00		0.58473977E 00	-0.
5	-0.57907083E 00 -0.	0.	0.0999999E 01		0.0999999E 01	-0.
6	-0.8685339E 00 -0.	0.	-0.78744834E-01		0.80293357E 00	-0.
7	0.24821915E-00 0.	0.	0.31116542E-00		0.15819736E-00	-0.
8	0.66749375E 00 0.	0.	0.43887147E-00		-0.16970489E-00	0.
9	0.58132744E 00 0.	0.	0.71052744E 00		-0.35012351E-01	0.
10	0.63967638E-02 0.	0.	0.94592633E 00		0.55494275E 00	-0.
11	-0.59767152E 00 -0.	0.	-0.36602820E-00		0.98118101E 00	-0.
12	-0.86276652E 00 -0.	0.	-0.18002119E-00		0.77532091E 00	-0.
13	0.10783558E-00 0.	0.	0.14372659E-00		0.11831710E-00	-0.
14	0.50513947E 00 0.	0.	0.32419031E-00		-0.18593793E-00	0.
15	0.43169130E-00 0.	0.	0.60354751E 00		-0.55626871E-01	0.
16	-0.62290946E-01 -0.	0.	0.81131787E 00		0.48073554E-00	-0.
17	-0.60960596E 00 -0.	0.	-0.37343036E-00		0.85065920E 00	-0.
18	-0.85156702E 00 -0.	0.	-0.21693902E-00		0.76783450E 00	-0.
19	0.62562181E-01 0.	0.	0.81322035E-01		0.10090126E-00	-0.
20	0.44704827E-00 0.	0.	0.28060315E-00		-0.20127232E-00	0.
21	0.38371947E-00 0.	0.	0.55999829E 00		-0.68956291E-01	0.
22	-0.84884894E-01 -0.	0.	0.75397816E 00		0.44659439E-00	-0.
23	-0.61312911E 00 -0.	0.	COLUMN 9	COLUMN 10	COLUMN	
24	-0.84668494E 00 -0.	0.	0.0999999E 01	0.	0.79207366E 00	-0.
1	-0.87628337E 00	0.	-0.46390782E-00	0.		
2	-0.79207761E 00	0.	0.30384646E-00	0.		
3	0.74732621E 00 0.	0.	0.32387303E-00	0.		
4	0.50810624E 00 0.	0.	0.36590731E-01	0.		
5	-0.20617008E-00 -0.	0.	-0.15851732E-00	0.		
6	-0.56951362E 00 -0.	0.	0.62807622E 00	0.		
7	-0.20693158E-01 -0.	0.	-0.34298901E-00	0.		
8	-0.16672964E-00 -0.	0.	0.66472408E-01	0.		
9	0.56158413E 00 0.	0.	0.18065064E-00	0.		
10	0.32397238E-00 0.	0.	-0.2359695E-01	0.		
11	-0.25312397E-00 -0.	0.	-0.20037591E-00	0.		
12	-0.62014322E 00 -0.	0.		0.		

FINITE DIFFERENCE METHOD FOR SANDWICHED SHELL, CORE THICKNESS = 0.90000E 00 MBAR = 0.17120E-04

\*\* INPUT DATA \*\*

14	U.25614561E-00	U.	-0.45026610E-00	-0.
15	0.48143050E-00	0.	-0.65837806E-01	-0.
16	0.21075485E-00	0.	0.99834414E-01	0.
17	-0.34754938E-00	0.	-0.41262152E-01	-0.
18	-0.63931482E 00	0.	-0.18149985E-00	-0.
19	0.09999999E 01	0.	-0.87648871E 00	-0.
20	0.38168780E-00	0.	-0.54694775E 00	-0.
21	0.46102948E-00	0.	-0.11268668E-00	-0.
22	0.17300419E-00	0.	0.74506135E-01	0.
23	-0.36566336E-00	0.	-0.43531751E-01	-0.
24	-0.64436039E 00	0.	-0.16918444E-00	-0.

\*\*FREQUENCY IN CPS\*\*  
 0.290874E 02      0.      0.395602E 02      0.      0.939199E 02      0.544783E 02  
 0.      0.685213E 02      0.

VALUES OF STRESS FUNCTIONS ARE

FOR NU. 1, EIGENVALUE OF 0.513357E-01					
-0.184208E 05	-0.184208E 05	-0.184208E 05	-0.184208E 05	-0.184208E 05	-0.184208E 05
-0.115071E 04	-0.176170E 05	-0.199603E 05	-0.141514E 05	-0.844934E 04	-0.636473E 04
-0.125596E 05	-0.292725E 05	-0.308387E 05	-0.186144E 05	-0.615749E 04	-0.127328E 04
-0.239685E 05	-0.360064E 05	-0.361200E 05	-0.212548E 05	-0.605339E 04	0.238612E-03
-0.356909E 05	-0.522556E 05	-0.545216E 05	-0.484780E 05	-0.426340E 05	-0.405058E 05
-0.184208E 05	-0.176171E 05	-0.292726E 05	-0.360064E 05		
FOR NU. 3, EIGENVALUE OF 0.949574E-01					
-0.138423E 06	-0.138423E 06	-0.138423E 06	-0.138423E 06	-0.138423E 06	-0.138423E 06
-0.217546E 05	-0.455286E 05	-0.533679E 05	-0.538811E 05	-0.520430E 05	-0.506917E 05
0.580042E 04	-0.333661E 04	-0.142275E 05	-0.144442E 05	-0.117691E 05	-0.107757E 05
0.333554E 05	0.117916E 05	-0.294874E 04	-0.340719E 04	-0.640279E 03	0.186152E-02
-0.255091E 06	-0.278959E 06	-0.286884E 06	-0.287312E 06	-0.285425E 06	-0.284048E 06
-0.138423E 06	-0.455286E 05	-0.333662E 04	0.117916E 05		
FOR NC. 5, EIGENVALUE OF 0.180077E-00					
-0.130294E 06	-0.130294E 06	-0.130294E 06	-0.130294E 06	-0.130294E 06	-0.130294E 06
-0.292528E 05	-0.433851E 05	-0.442211E 05	-0.469772E 05	-0.496250E 05	-0.501455E 05
0.499362E 03	0.234851E 04	0.331543E 04	-0.932216E 03	-0.770595E 04	-0.110985E 05
0.302515E 05	0.198840E 05	0.185151E 05	0.134107E 05	0.478330E 04	0.173821E-02
-0.231335E 06	-0.245472E 06	-0.246057E 06	-0.248972E 06	-0.251727E 06	-0.252274E 06
-0.130294E 06	-0.433851E 05	0.234850E 04	0.198840E 05		

FINITE DIFFERENCE METHOD FOR SANDWICHED SHELL, CORE THICKNESS = 0.90000E 00 MBAR = 0.17120E-04

\*\* INPUT DATA \*\*

FOR NO. 7,	EIGENVALUE OF	0.284880E-00			
-0.230840E 05	-0.230840E 05	-0.230840E 05	-0.230840E 05	-0.230840E 05	-0.230840E 05
0.460472E 04	0.208058E 05	0.217604E 04	0.217604E 04	-0.735287E 04	-0.647665E 04
0.566376E 05	0.664671E 05	0.233954E 05	0.233954E 05	-0.276151E 04	-0.106500E 04
0.108670E 06	0.903362E 05	0.325952E 05	0.325952E 05	-0.193952E 04	0.337982E-03
-0.507727E 05	-0.347735E 05	-0.546791E 05	-0.546791E 05	-0.639354E 05	-0.629865E 05
-0.230840E 05	0.208058E 05	0.664671E 05	0.664671E 05		
FOR NO. 9,	EIGENVALUE OF	0.535213E 00			
0.456284E 05	0.456284E 05	0.456284E 05	0.456284E 05	0.456284E 05	0.456284E 05
0.227783E 05	0.178277E 05	0.121483E 05	0.121483E 05	0.199725E 05	0.216728E 05
-0.330511E 05	-0.239622E 05	-0.202686E 05	-0.202686E 05	0.861574E 03	0.556888E 04
-0.888804E 05	-0.494148E 05	-0.344417E 05	-0.344417E 05	-0.605763E 04	-0.570264E-03
0.684784E 05	0.643211E 05	0.579733E 05	0.579733E 05	0.660853E 05	0.678302E 05
0.456284E 05	0.178277E 05	-0.239622E 05	-0.239622E 05		