

11 Beams

Goal: understand approximations & equations for solids with special shapes

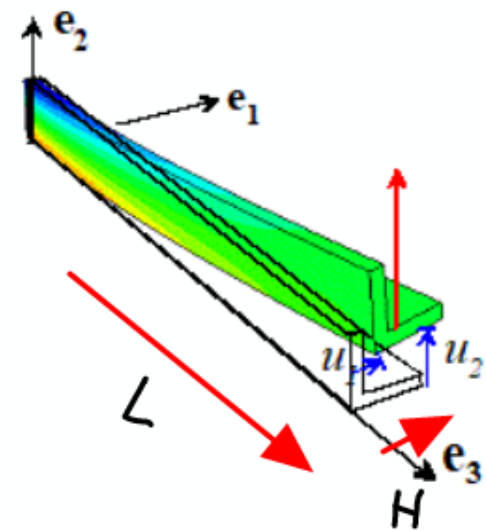
Assumptions:

- (1) Length \gg x-sect dimension
- (2) Small deflections
- (3) Linear elastic
- (4) Neglect twist

[ABAQUS does not assume (2) - (4)]

Two versions of beam theory exist

① "Euler-Bernoulli"	} Valid for	$L > 15H$
"Cubic Formulation" in ABAQUS		
② "Timoshenko"	} Valid for	$L > 5H$
"Shear Flexible" in ABAQUS		

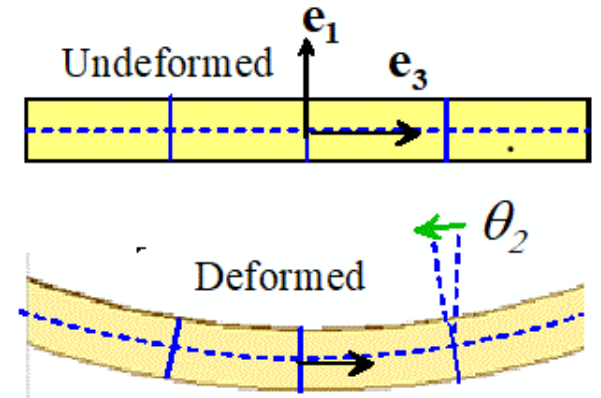


ABAQUS
default

In ABAQUS can change to Euler-Bernoulli in mesh module by using "element type" module

① Bernoulli Theory:

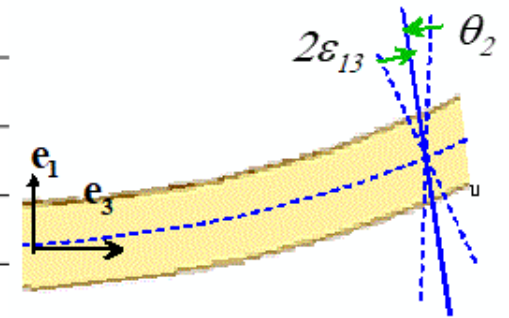
x-sections remain perpendicular to centerline



② Timoshenko

x-sections can rotate relative to centerline

Focus mostly on case ① here

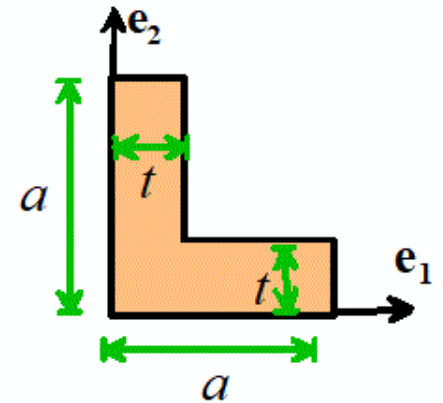


Describing x-sect geometry (elastic beams)

X-sect area $A = \int_A dA$

Centroid :

$$\underline{r} = r_1 \underline{e}_1 + r_2 \underline{e}_2 = \frac{1}{A} \int_A (x_1 \underline{e}_1 + x_2 \underline{e}_2) dA$$



Area moment of inertia tensor

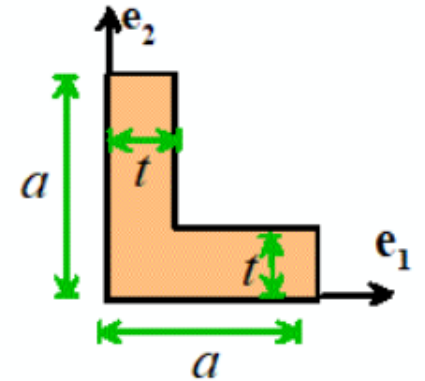
$$[I] = \begin{bmatrix} I_{11} & -I_{12} \\ -I_{12} & I_{22} \end{bmatrix}$$

$$I_{11} = \int_A (x_2 - \bar{r}_2)^2 dA$$

$$I_{22} = \int_A (x_1 - \bar{r}_1)^2 dA$$

$$I_{12} = \int_A (x_1 - \bar{r}_1)(x_2 - \bar{r}_2) dA$$

Example: Calculate the area moments of inertia for the L section shown



$$\text{Here } A = \int_0^t \int_0^a dx_1 dx_2 + \int_t^a \int_0^t dx_1 dx_2$$

$$\underline{r} = \frac{1}{A} \left\{ \int_0^t \int_0^a (x_1 \underline{e}_1 + x_2 \underline{e}_2) dx_1 dx_2 + \int_t^a \int_0^t (x_1 \underline{e}_1 + x_2 \underline{e}_2) dx_1 dx_2 \right\}$$

I_{11}, I_{22}, I_{12} follow: use MATLAB to do integrals

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syms x1 x2 a t rbar
integrand = 1
A = simplify(int(int(integrand,x1,[0,a]),x2,[0,t]) + int(int(integrand,x1,[0,t]),x2,[t,a]))
integrand = [x1,x2];
rbar = simplify((int(int(integrand,x1,[0,a]),x2,[0,t]) + int(int(integrand,x1,[0,t]),x2,[t,a]))/A)
integrand = (x2-rbar(2))^2;
I11 = simplify(int(int(integrand,x1,[0,a]),x2,[0,t]) + int(int(integrand,x1,[0,t]),x2,[t,a]))
integrand = (x1-rbar(1))^2;
I22 = simplify(int(int(integrand,x1,[0,a]),x2,[0,t]) + int(int(integrand,x1,[0,t]),x2,[t,a]))
integrand = (x1-rbar(1))*(x2-rbar(2));
I12 = simplify(int(int(integrand,x1,[0,a]),x2,[0,t]) + int(int(integrand,x1,[0,t]),x2,[t,a]))

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$$A = t(2a - t)$$

$$rbar =$$

$$\left(\frac{a^2 + at - t^2}{4a - 2t} \quad \frac{a^2 + at - t^2}{4a - 2t} \right)$$

$$I11 =$$

$$\frac{t(5a^4 - 10a^3t + 11a^2t^2 - 6at^3 + t^4)}{12(2a - t)}$$

$$I22 =$$

$$\frac{t(5a^4 - 10a^3t + 11a^2t^2 - 6at^3 + t^4)}{12(2a - t)}$$

$$I12 =$$

$$-\frac{a^2t(a-t)^2}{4(2a-t)}$$

Describing deformation

Calculate:

① Deflection vector of centroid

$$\underline{u} = u_1(x_3) \underline{e}_1 + u_2(x_3) \underline{e}_2 + u_3(x_3) \underline{e}_3$$

② Rotation vector of x-section

$$\underline{\theta} = \theta_1 \underline{e}_1 + \theta_2 \underline{e}_2 + \theta_3 \underline{e}_3$$

[$\underline{u}, \underline{\theta}$ are U, UR in ABAQUS]

These are related:

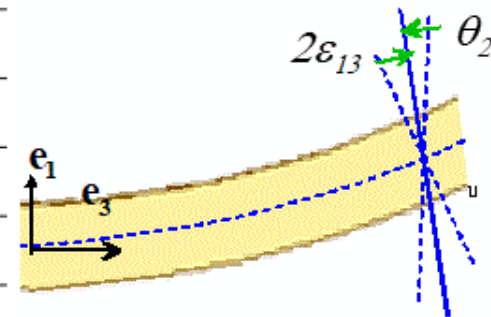
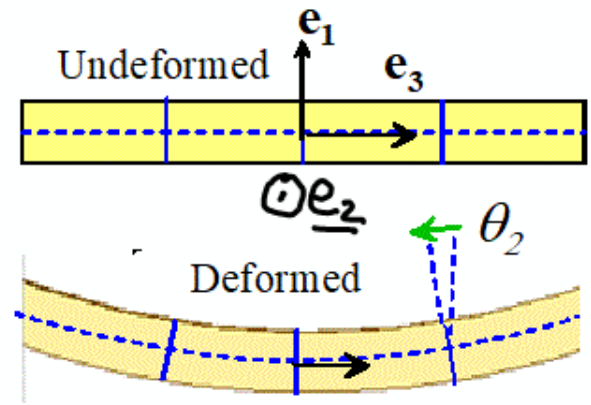
Euler-Bernoulli

$$\theta_2 = \frac{\partial u_1}{\partial x_3}$$

$$\theta_1 = -\frac{\partial u_2}{\partial x_3}$$

Timoshenko

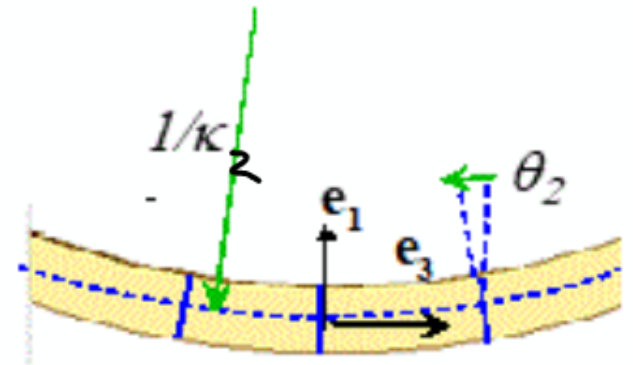
$$\theta_2 = \frac{\partial u_1}{\partial x_3} - 2\varepsilon_{23} \quad \theta_1 = -\frac{\partial u_2}{\partial x_3} + 2\varepsilon_{13}$$

 $\varepsilon_{13}, \varepsilon_{23}$: additional unknowns

Curvature Vector

$$\underline{\kappa} = \frac{d\underline{\theta}}{dx_3} = \kappa_1 \underline{e}_1 + \kappa_2 \underline{e}_2 + \kappa_3 \underline{e}_3$$

$[\kappa_1, \kappa_2]$: Bending κ_3 : Twist
(neglect)



Strains : Assume no stretch of centroid

$$\epsilon_{33} = -\kappa_2 (x_1 - \bar{r}_1) + \kappa_1 (x_2 - \bar{r}_2)$$

$$\epsilon_{11} = \epsilon_{22} = -\nu \epsilon_{33}$$

Euler Bernoulli : all other $\epsilon_{ij} = 0$

Timoshenko has nonzero $\epsilon_{13}, \epsilon_{23}$

Stresses: $\sigma_{33} = E \epsilon_{33}$

All other $\sigma_{ij} = 0$ in Euler-Bernoulli

In Timoshenko: $\sigma_{13} = 2\mu \epsilon_{13}$ $\sigma_{23} = 2\mu \epsilon_{23}$

μ : Shear modulus (need to specify in ABAQUS)

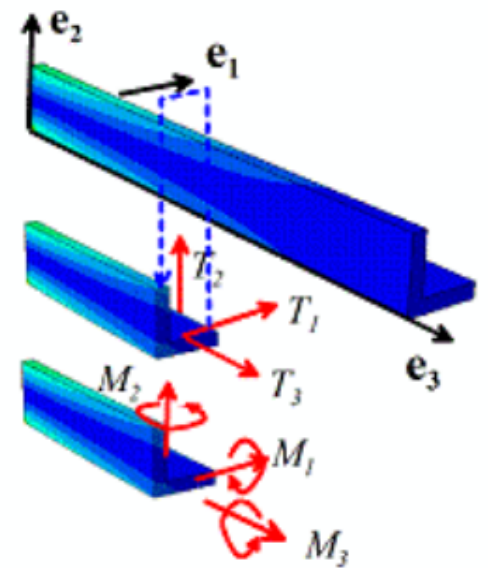
Internal Forces in beams

Internal force vector $\underline{T} = T_1 \underline{e}_1 + T_2 \underline{e}_2 + T_3 \underline{e}_3$

$[T_1, T_2]$: Shear T_3 : Axial

Moment vector $\underline{M} = M_1 \underline{e}_1 + M_2 \underline{e}_2 + M_3 \underline{e}_3$

page 8 $[M_1, M_2]$: Bending M_3 : Twist (neglect)



Moment - Curvature relations

$$\begin{bmatrix} M_1 \\ M_2 \end{bmatrix} = E \begin{bmatrix} I_{11} & -I_{12} \\ -I_{12} & I_{22} \end{bmatrix} \begin{bmatrix} \kappa_1 \\ \kappa_2 \end{bmatrix}$$

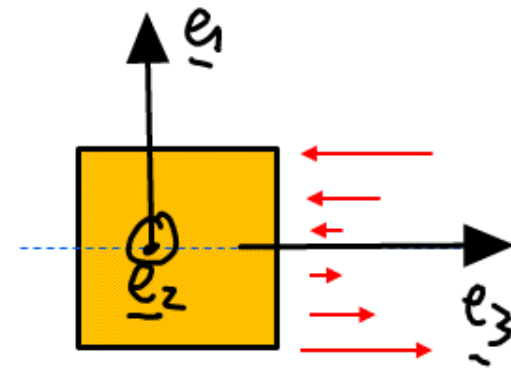
$$\underline{M} = E [I] \underline{\kappa}$$

To see this recall :

$$M_2 = - \int_A \underbrace{(x_1 - \bar{r}_1)}_{\text{moment arm}} \underbrace{\sigma_{33}}_{\text{Force}} dA$$

$$M_1 = \int_A (x_2 - \bar{r}_2) \sigma_{33} dA$$

moments
of stress
distribution



$$\sigma_{33} = E \epsilon_{33} = E (-\kappa_2 (x_1 - \bar{r}_1) + \kappa_1 (x_2 - \bar{r}_2))$$

$$\Rightarrow M_2 = -E \int (x_1 - \bar{r}_1) (-\kappa_2 (x_1 - \bar{r}_1) + \kappa_1 (x_2 - \bar{r}_2)) dA$$

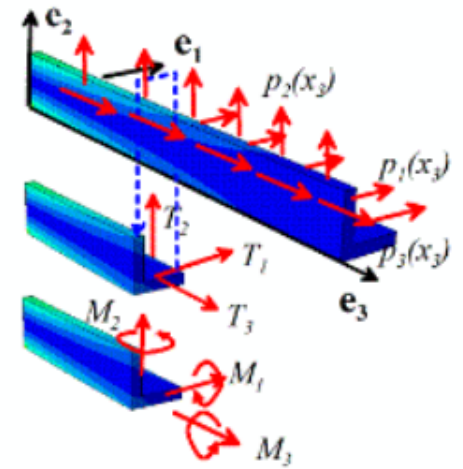
$$= -E \kappa_1 I_{12} + E \kappa_2 I_{22} \quad \checkmark$$

Equations of motion

Let $\underline{f} = f_1 \underline{e}_1 + f_2 \underline{e}_2 + f_3 \underline{e}_3$ be external force per unit length on beam

Let ρ be mass density

Let $\underline{a} = \frac{d^2 \underline{u}}{dt^2}$ be accel of centroid



Linear momentum: $(\underline{F} = m \underline{a})$

$$\frac{d\underline{I}}{dx_3} + \underline{f} = \rho A \underline{a} \quad (3 \text{ equations in } 1, 2, 3 \text{ directions})$$

Angular momentum (neglect mass moment of inertia)

$$\frac{dM_1}{dx_3} - T_2 - \theta_1 T_3 = 0 \quad \frac{dM_2}{dx_3} + T_1 - \theta_2 T_3 = 0$$

Boundary Conditions

Either: Displacement / Rotation

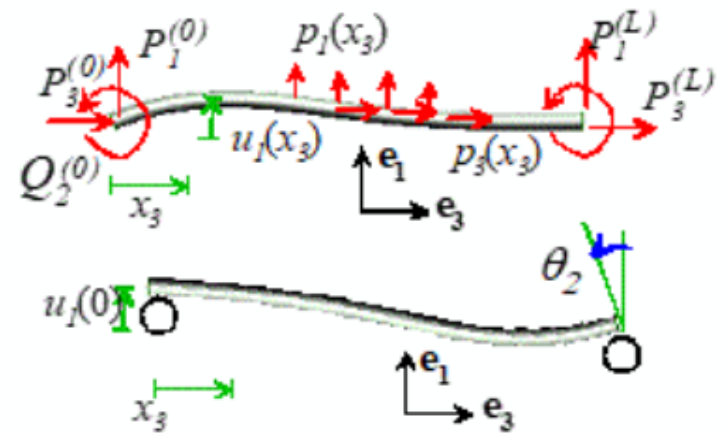
$$\underline{u}, \underline{\theta} \text{ given at } \begin{matrix} x_3=0 \\ x_3=L \end{matrix}$$

Or: Force / moments given

$$\underline{I} = \underline{P}^{(L)} \text{ at } x_3=L \quad \underline{I} = -\underline{P}^{(0)} \text{ @ } x_3=0$$

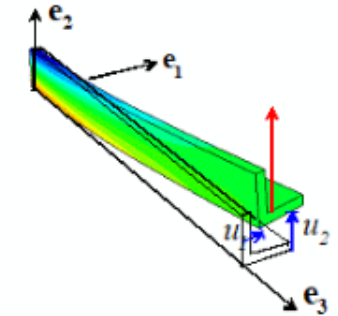
$$\underline{M} = \underline{Q}^{(L)} \text{ at } x_3=L \quad \underline{M} = -\underline{Q}^{(0)} \text{ @ } x_3=0$$

(Free beam $\underline{I} = \underline{M} = \underline{Q} = 0$)



Beams – summary of equations

Goal: Calculate (1) Displacement of centroid $\mathbf{u}(x_3) = u_1\mathbf{e}_1 + u_2\mathbf{e}_2 + u_3\mathbf{e}_3$
 (2) Rotation of x-section $\boldsymbol{\theta}(x_3) = \theta_1\mathbf{e}_1 + \theta_2\mathbf{e}_2 + \theta_3\mathbf{e}_3$
 (3) Curvature vector $\boldsymbol{\kappa}(x_3) = d\boldsymbol{\theta} / dx_3$

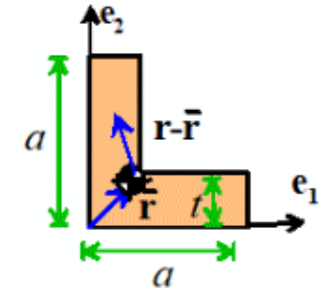


Neglect twist θ_3 here, but ABAQUS will include twist

Section properties:

$$A = \int_A dA \quad \bar{\mathbf{r}} = \frac{1}{A} \int_A (x_1\mathbf{e}_1 + x_2\mathbf{e}_2) dA$$

$$\mathbf{I} = \begin{bmatrix} I_{11} & -I_{12} \\ -I_{12} & I_{22} \end{bmatrix} \quad I_{11} = \int_A (x_2 - \bar{r}_2)^2 dA \quad I_{22} = \int_A (x_1 - \bar{r}_1)^2 dA \quad I_{12} = \int_A (x_1 - \bar{r}_1)(x_2 - \bar{r}_2) dA$$



Deformation:

Euler-Bernoulli theory (no shear): $\theta_1 = -du_2 / dx_3$ $\theta_2 = du_1 / dx_3$

(Timoshenko theory allows x-sect to rotate relative to neutral axis)

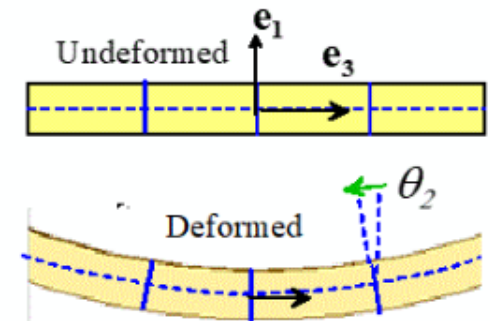
Axial strain: $\varepsilon_{33} = \kappa_1(x_2 - \bar{r}_2) - \kappa_2(x_1 - \bar{r}_1)$

(Timoshenko beam has shear strains)

Stresses: $\sigma_{33} = E\varepsilon_{33}$

Other stresses zero in E-B beams

(Timoshenko beams have shear stresses)



Beams – summary of equations

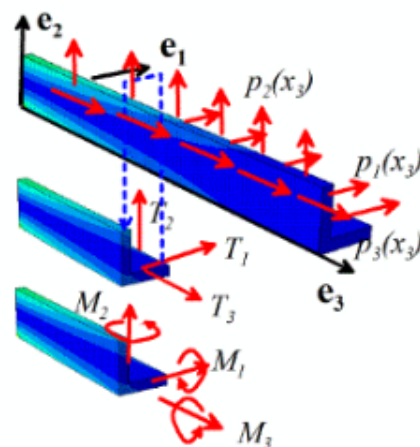
Internal Forces (forces/moments on section normal to \mathbf{e}_3 :

Force vector $\mathbf{T} = T_1\mathbf{e}_1 + T_2\mathbf{e}_2 + T_3\mathbf{e}_3$

Moment vector $\mathbf{M} = M_1\mathbf{e}_1 + M_2\mathbf{e}_2 + M_3\mathbf{e}_3$

$$M_1 = \int_A \sigma_{33}(x_2 - \bar{r}_2) dA \quad M_2 = - \int_A \sigma_{33}(x_1 - \bar{r}_1) dA$$

No twist means $M_3 = 0$



Moment-Curvature relations:

$$\begin{bmatrix} M_1 \\ M_2 \end{bmatrix} = E \begin{bmatrix} I_{11} & -I_{12} \\ -I_{12} & I_{22} \end{bmatrix} \begin{bmatrix} \kappa_1 \\ \kappa_2 \end{bmatrix}$$

Equations of motion:

F=ma: $\frac{dT_i}{dx_3} + p_i = \rho A a_i$

Angular Momentum: $\frac{dM_1}{dx_3} - T_2 - \theta_1 T_3 = 0$ $\frac{dM_2}{dx_3} + T_1 - \theta_2 T_3 = 0$

Boundary Conditions (at ends):

Fixed end: $\mathbf{u} = \mathbf{0}$



Free to move: $\mathbf{T} = \mathbf{0}$

Clamped end: $\theta = \mathbf{0}$



Free to rotate: $\mathbf{M} = \mathbf{0}$