## OUR PRODUCT RANGE CONSTRUCTION WITH CLT



BIM ONLINE PORTAL

## SOLID TIMBER CONSTRUCTION

## WOOD CONNECTORS

## CONSTRUCTIVE FASTENING

## SPECIAL COMPONENTS




SOLID TIMBER CONSTRUCTION

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## PRODUCT FINDER

|  | Sill plate | Wall-Concrete | Wall-Wall | Beam | Wall-Ceiling |
| :---: | :---: | :---: | :---: | :---: | :---: |
| Wood connectors |  |  |  |  |  |
| CLT system inside corner | x | x | $\checkmark$ | x | $\checkmark$ |
| CLT system angle | x | x | $\checkmark$ | x | $\checkmark$ |
| Shearing angle | x | $\checkmark$ | $\checkmark$ | x | $\checkmark$ |
| HB flat shearing angle | x | $\checkmark$ | x | x | x |
| HH flat shearing angle | x | x | x | $x$ | x |
| Shearing plate | x | $\checkmark$ | $\checkmark$ | x | x |
| Tension strap HB60/70 | $\checkmark$ | $\checkmark$ | x | x | x |
| Tension strap HH60/70 | x | x | $\checkmark$ | $x$ | $\checkmark$ |
| Shear wall connector | x | $x$ | $\checkmark$ | x | x |
| Assembly connector | x | $x$ | $\checkmark$ | x | x |
| Magnus hook connector | x | x | x | $\checkmark$ | x |
| T-profile | x | x | x | $\checkmark$ | x |
| Constructive fastening |  |  |  |  |  |
| Rock concrete screw | $\checkmark$ | $\checkmark$ | x | x | x |
| KonstruX fully threaded screw | x | x | $\checkmark$ | $\checkmark$ | $\checkmark$ |
| Angle-bracket screw | x | $\checkmark$ | $\checkmark$ | x | $\checkmark$ |
| Panelwwistec | x | $x$ | $\checkmark$ | $\checkmark$ | $\checkmark$ |
| SawTec | x | x | $\checkmark$ | $\checkmark$ | $\checkmark$ |
| Topduo | x | x | x | x | x |

## Furher products

| Lifting anchor, ball supporting bolt | x | x | x | x | x |
| :---: | :---: | :---: | :---: | :---: | :---: |
| IdeeFix | x | $\checkmark$ | x | $\checkmark$ | $\checkmark$ |
| SonoTec sound insulation cork | $\checkmark$ | $\checkmark$ | $\checkmark$ | $\checkmark$ | $\checkmark$ |
| Bolt anchor | $\checkmark$ | x | x | x | x |
| Silent EPDM decoupling profile | $\checkmark$ | $\checkmark$ | $\checkmark$ | $\checkmark$ | $\checkmark$ |
| Ecktec | x | x | x | x | x |

$\checkmark$ Usable<br>X Not Usable<br>- Irrelevant

| Ceiling-Ceiling | Wall-Floor | Roof | Stairs | Insulation | Handling | Page |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| x | $\checkmark$ | - | - | - | - | 14-17 |
| x | $\checkmark$ | - | - | - | - | 18-20 |
| x | $\checkmark$ | - | - | - | - | 22-25 |
| x | x | - | - | - | - | 26-27 |
| x | $\checkmark$ | - | - | - | - | 26-27 |
| x | x | - | - | - | - | 28-31 |
| x | x | - | - | - | - | 32-33 |
| x | $\checkmark$ | - | - | - | - | 34-35 |
| $x$ | x | - | - | - | - | 36-37 |
| $x$ | x | - | - | - | $\checkmark$ | 38-39 |
| $x$ | x | - | - | - | - | 40-59 |
| x | x | - | - | - | - | 60-61 |


| $x$ | x | x | x | x | - | 66-71 |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| $\checkmark$ | $\checkmark$ | $\checkmark$ | $\checkmark$ | $\checkmark$ | - | 72-97 |
| $\checkmark$ | $\checkmark$ | x | x | x | - | 98-99 |
| $\checkmark$ | $\checkmark$ | $\checkmark$ | $\checkmark$ | $\checkmark$ | - | 100-115 |
| $\checkmark$ | $\checkmark$ | $\checkmark$ | $\checkmark$ | $\checkmark$ | - | 116-119 |
| x | x | x | x | $\checkmark$ | - | 120-125 |
| x | x | $x$ | x | $x$ | $\checkmark$ | 128-139 |
| x | $\checkmark$ | x | x | x | - | 140-147 |
| $\checkmark$ | $\checkmark$ | $\checkmark$ | x | x | - | 150-161 |
| x | x | x | x | $x$ | - | 162-165 |
| $\checkmark$ | $\checkmark$ | x | $\checkmark$ | x | - | 166-167 |
| x | x | x | x | x | x | 168-169 |




## OUR EUROTEC BIM-ONLINEPORTAL ALL DATA AT ONE SIGHT

## THE EUROTEC BIM PORTAL FOR YOUR CONSTRUCTION PLANNING!

Building information modelling (BIM) has become an indispensable part of modern planning. On our userfriendly plafform, you will find product specifications as BIM-enabled data for use in your construction project. Some of the versatile file formats include 3D/CAD objects, DWG and PDF files, along with notes about our ETA certifications.

CLT (Cross Laminated Timber) panels consist of several layers of wooden boards stacked crosswise (typically at an angle of 90 degrees). They are glued together on their broad faces and sometimes also on the narrow faces.

A cross-section of a CLT element has at least three bonded sheet layers arranged in an alternating way and orthogonal to the adjacent layers. In special configurations, successive layers can be arranged in the same direction, creating a double layer (for example, double longitudinal layers on the outer surfaces and/or additional double layers at the core of the panel) to achieve specific structural capacities.

CLT products will typically be manufactured with an odd number of layers. Gluing three to seven layers together is common. The thickness of the individual layers of wood can vary from 16 mm to 51 mm , while the width can vary from about 60 mm to 240 mm .

The panel sizes vary depending on the manufacturer. Typical widths are $0.6 \mathrm{~m}, 1.2 \mathrm{~m}, 2.4 \mathrm{~m}$, and 3 m . The length can be up to 18 m . In special cases, the thickness can be up to 500 mm . Typical thicknesses are between 60 and 300 mm , however.
(Transport regulations may limit the CLT panel sizes).
The timber in the outer layers of the CLT panels that are used as walls are aligned up and down, parallel to the gravity loads, to maximise the vertical loading capacity of the wall. Similarly, in floor and roof systems, the outer layers run parallel to the main tension direction.

## ADVANTAGES OF BUIILDING WITH CLT

- CLT allows screw connection in any direction, irrespective of the grain direction, as the layering of the boards means that no grain direction has to be observed.

Reduced construction time due to prefabrication of the elements
Enables almost film-free construction due to the diffusion-open properties of the CLT elements.

CLT has both sound and heat insulating properties.

- A wide range of architectural design options.
- All components of a house (walls, ceilings, and roof) can be made of CIT.
- Lower weight compared to concrete and bricks
- No construction waste when demolishing buildings. CLT is completely ecologically recyclable.



## PRODUCTION OF CLT




The boards are sorted after the soffwood boards have gone through a drying process (more than 48 hours). Growth deviations in the wood that would reduce the strength, or are simply unsightly, are marked. The sections that have such defects are cut out.

The boards of different lengths are joined together to create an almost endless strand of wooden boards, which is necessary for CLT production. This is done by means of finger joints. The resulting boards are then planed to eliminate thickness deviations between the boards.

The manufactured boards are applied manually or mechanically to form a layer. Adhesive is applied to the resulting surface after a layer has been completely applied. The most common method here is a glue curtain through which the layer is passed.

Another layer is placed on top of the glued layer. This is aligned so that the fibre direction of the new layer runs at an angle of $90^{\circ}$ to the fibres of the board below. Glue is then applied to the new layer also. This process is repeated until the desired number of board layers is achieved.

Once the desired number of layers is reached, the glued lamellas are pressed. The size of the press bed determines the possible panel size. As soon as the adhesive has cured, the CLT panel is reworked to remove any dirt, adhesive residues, or protruding wood. This is done by planing and grinding the CLT panel.

## BUILDING WITH CROSS LAMINATED TIMBER

The construction phases of modern timber construction methods, such as building with cross laminated fimber, are very different from that of the conventional solid construction method. Whereas with solid construction most of the work takes place on the building site, with timber construction much of the work has now shifted from the construction site to the factory.

The keyword here is prefabrication. All wall, ceiling, and roof elements are delivered to the construction site not as unprocessed CLT panels and thus raw material. They are prepared in special joinery centres for later assembly.

In the CNC joinery centres, the manufactured CLT panels are further processed into individual elements. All necessary work that is required on the construction site for fasteners of all kinds and/or for geometries that would be too difficult to realise on the construction site, is carried out here. Common joinery work carried out in the factory includes:

- Windows and door cut-outs
- Angled cuts in the gable area
- Cuts and notches
- Milling of folding systems (for example: joint deck board fold, fier fold)
- Special geometries for special connectors

Such complex processing steps, especially through the use of computer-controlled processing machines, increase the amount of upfront planning work. Positions for connectors and installations within the house (electrical/water) must be able to be provided with the necessary information. Furthermore, care is taken to ensure that all components are matched to each other to the millimetre in the final assembly, so that there are no problems in the final assembly.


|  |  |  |
| :--- | :--- | :--- |



## A NEW ERA OF WOOD CONNECTORS

We offer a solution for every load case occurring in solid wood and timber frame construction in the form of brackets, straps, hook connectors or beam girders. We are currently working flat out on unique solutions for system connectors. This solution is a system of all kinds of connections in modular and system design. Our optimised screw patterns enable absorption of high tensile and shearing forces, so fewer connectors are required.

Versatility is very important to us. One of our new products is the CLT system inside corner. A strong connection of wall nodes is achieved when it is used in combination. The inside corner is also an unbeatable solution for timber-timber connections at corner points.

## CLT SYSTEM INSIDE CORNER

Developed for modern timber construction

CLT system inside corner


| Art. no. | Dimensions $[\mathrm{mm}]^{\text {a }}$ | Material | Material thickness $[\mathrm{mm}]$ | PU |
| :--- | :--- | :--- | :--- | ---: |
| 954188 <br> a) Lenght $\times$ Width $\times$ Height | $120 \times 120 \times 120$ | S250 Galvanised | 4 | 1 |

The CLT system inside corner can be used to connect internal corners with each other. It can be used both individually and in combination with several CLT system inside corners. A hexagon head screw can be led from one element, through the wall, to the other element, for this purpose. If this is applied in all possible directions, a stable construction for wall nodes is created. This can also be achieved with the combination of our IdeeFix. Although the individual corners are not directly connected to each other, it results in a very secure connection between the wall and ceiling or floor elements.

## ADVANTAGES

- Combining several CLT system inside corners, an effective connection of different elements with each other is created
- Fewer connectors required
- Versatile applications


With KonstruX and Hexagon head screw M16


With KonstruX and IdeeFix


## CLT SYSTEM INSIDE CORNER - COMBINATION

The CLT system inside corner is an extremely combinable connector. Wall nodes can be connected in a number of different ways.

A construction can be extremely strengthened by connecting several interior corners of a system through the wood. This can be achieved with our Ideefix or also hexagonal bolts, for example. There are numerous possibilities.

In contrast to using the connector individually (see examples), the most force can be absorbed and distributed when the internal corners of the system are positioned opposite each other.


Common combination example

## Gurotec <br> Wood connector

## POSSIBLE APPLICATIONS

WALL JUNCTION - VIIIBLE SOLID WOOD CEILING




CANTILEVER STRUCTURES


## CLT SYSTEM ANGLE

Developed for modern timber construction

CLT system angle


| Art. no. | Dimensions [mm] $]^{\text {a }}$ | Material | Material thickness $[\mathrm{mm}]$ | PU |
| :--- | :--- | :--- | :--- | :--- |
| 954180 | $230 \times 80 \times 120$ | S250 Galvanised | 4 | 1 |

a) Lenght $x$ Width $x$ Height

The CLT system angle is ideally suited for use in solid timber construction. The scope of application is limited to the use of CLT (cross-laminated timber). The solid construction allows it to transmit major forces. In contrast to the standard angles (on the following pages), the system angle CLT can be combined with our IdeeFix. This makes it possible to construct complex connections.

ADVANTAGES

- High load bearing capacity
- Versatile applications
- Compatible with SK04


INSTRUCTIONS FOR USE
Either $5 \times 60 \mathrm{~mm}$ angle fitting screws in combination with the KonstruX SK $10 \times 125 \mathrm{~mm}$, are used for the CLT system angle. When used with Ideefix, only 4 IdeeFix and 4 KonstruX are needed - see application picture. It is also possible to combine Ideefix and screw bolts through a wall. The load values of the ETA must be observed.
For further information, please contact our technical department technik@eurotec.team.


## CLT SYSTEM ANGLE - STATIC VALUES



| Slip Modulus |  |  |  |  |
| :---: | :---: | :---: | :---: | :---: |
| $\mathrm{K}_{1, \text { ser }}$ | $\mathrm{K}_{23, \text { ser }}$ | $\mathrm{K}_{4, \text { esr }}$ | $\mathrm{K}_{5, \text { eer }}$ |  |
| $\mathrm{F}_{\mathrm{l}, \text { Rk }} / 6 \mathrm{~mm}$ | $\mathrm{~F}_{23, \mathrm{Rk}} / 2 \mathrm{~mm}$ | $\mathrm{~F}_{4, \text { Rkk }} / 2,5 \mathrm{~mm}$ | $\mathrm{~F}_{1, \mathrm{Rk}} / 2,5 \mathrm{~mm}$ |  |


| Lood direction F1; F2/F3; F4; F5 |  |  |  |  |  |  |  |  |  |  |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Veritial leg connection Angle-bracket screw $05 \mathrm{~mm} \mathrm{n}=43$ | 5,0x 40 | 5,0x50 | 5,0×60 | 5,0 $\times 70$ | 5,0x 40 | 5,0x50 | 5,0×60 | 5,0x70 | 5,0 $\times 40$ | 5,0 50 | 5,0 60 | 5,0×70 |
| Horizontal leg comnection | $\begin{gathered} \text { Angle brackesestrem } \\ 5,0 \times 40 \\ \mathrm{n}=43 \end{gathered}$ | $\begin{gathered} \text { ngebebrackestser } \\ 5,0 \times 50 \\ n=43 \end{gathered}$ | $\begin{aligned} & \text { gelebracketsan } \\ & 5,0 \times 60 \\ & \mathrm{n}=43 \end{aligned}$ | $\begin{gathered} \text { ngebercatestreem } \\ 5,0 \times 700 \\ n=43 \end{gathered}$ | $\underset{\substack{\text { Ideefix } \\ n=3 \\ \hline}}{ }$ | $\underset{\substack{\text { Idefix } \\ n=3 \\ \hline}}{ }$ | $\underset{\substack{\text { Ideefix } \\ n=3 \\ \hline}}{ }$ | $\underset{\substack{\text { Idefix } \\ n=3 \\ \hline}}{ }$ | $\begin{gathered} \text { M16 } 6.8 \\ n=3 \end{gathered}$ | $\begin{gathered} \begin{array}{c} \text { M168.8 } \\ \mathrm{n}=3 \end{array} \end{gathered}$ | $\begin{gathered} \text { M168.8 } \\ \mathrm{n}=3 \end{gathered}$ | $\begin{gathered} M 168.8 \\ \mathrm{n}=3 \end{gathered}$ |
| Konstrux $10 \times 125 \mathrm{n}=4$ |  |  |  |  |  |  |  |  |  |  |  |  |
| $F_{1, \text { Rk pul }}$ | 55,8 kN | 62,4kN | 69, 1 kN | 75,7 kN | 43, lN | $43,1 \mathrm{kN}$ | 43, kN | 43, 1 kN | 43, lN | 43,1 kN | 43, 1 kN | 43, kN |
| F23, 裸 | 49, 1 kV | 58,3kN | 62, l kN | 66, kN | 49, 1 kN | 55,9 kN | 55,9 kN | 55,9 kV | 49, 1 kN | 58,3kN | $\begin{aligned} & 62,1 \mathrm{kN} \\ & 60,5 \mathrm{kN} \end{aligned}$ | $66,0 \mathrm{kN}$ 60,5 kN |
| $F_{4}$, Rk | 54kV |  |  |  | 54kV |  |  |  | 54kV |  |  |  |
| F5, Rkpull $\perp$ on CII | 6,9 kN | 6,9 kN | 6,9 kN | 6,9 kN | 6,9 kN | 6,9 kN | 6,9 kN | 6,9 kN | 6,9 kN | 6,9 kN | 6,9 kN | 6,9 kN |


| Lood direction F1; F2/F3; F4; F5 |  |  |  |  |  |  |  |  |  |  |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Veritial leg comnection | $\underset{\substack{\text { Ideefix } \\ \mathrm{n}=3 \\ \hline 0}}{\substack{0}}$ |  |  | $\begin{gathered} \text { Ideefix } 840 \\ n=2 \end{gathered}$ |  |  | $\underset{\substack{M 168.8 \\ n=3}}{ }$ |  |  | $\underset{\substack{M 168.8 \\ \mathrm{n}=2}}{ }$ |  |  |
| Konstrux 10x $125 \mathrm{n}=4$ |  |  |  |  |  |  |  |  |  |  |  |  |
| Horizontal leg comection | Angle-bracketescrew $05,0 ; n=43$ | $\underset{\substack{\text { Ideefix } \varnothing \\ n=3}}{ }$ | $\underset{\substack{M 168.8 \\ n=3}}{\substack{\text { n }}}$ | Anglebracket screw $05,0 ; n=43$ | $\begin{gathered} \text { Ideefix } \varnothing 40 \\ n=3 \\ \hline \end{gathered}$ | $\underset{\substack{\text { M16 } \\ \mathrm{n}=2 \\ \hline}}{ }$ | Angle-bracket screw $05,0 ; n=43$ | $\underset{\substack{\text { Ideefix } 840 \\ n=3}}{\substack{\text { and }}}$ | $\underset{\substack{\text { M16 } \\ \mathrm{n}=3}}{ }$ | Angle-bracket screw <br> 05,$0 ; n=43$ | $\underset{\substack{\text { Ideefix } \varnothing 40 \\ n=3}}{ }$ | $\begin{gathered} M 168.8 \\ \mathrm{n}=3 \end{gathered}$ |
| $F_{1}$, Rk pull |  | 43, 1 kN |  |  | 29,9 kN |  |  | 43, 1 kN |  |  | 43, 1 kN |  |
| F23, Rk |  | 26, lN |  |  | 22, 3 kN |  |  | $\begin{aligned} & 34,4 \mathrm{kN} \\ & 29,3 \mathrm{kN} \end{aligned}$ |  |  | $\begin{aligned} & 29,6 \mathrm{kN} \\ & 25,2 \mathrm{kN} \end{aligned}$ |  |
| F4, Rk |  | 54,0 kV |  |  | 54, kN |  |  | 54,0 kN |  |  | 54,0 kN |  |
| F5, Rk poll $\perp$ on ClI |  | 4,8 kN |  |  | 4,8 kN |  |  | 4,8 kN |  |  | 4,8 kN |  |

$F 4$, Rk $=54 \mathrm{kN}$ Druck $\perp$ on CLI ; independent of connections.
For connections with M16 8.8 if bolt head or nut is not located on CLI: Washer with $\mathrm{d}_{0}=40 \mathrm{~mm}$.
$\rho_{\mathrm{k}}=350 \mathrm{~kg} / \mathrm{m}^{3}$ conservative for some approved cross-laminated timber, increase of load-bearing capacities according to $E A A-19 / 0020$ with $\mathrm{kdens}=\left(\frac{\rho \mathrm{k}}{\left.350 \mathrm{~kg} / \mathrm{m}^{3}\right)}\right)$ possible.
he construction of the supporting structure should prevent the twisting of the cross laminated timber components.
In case of connection with CLI system angles on both sides, the values of this table may be applied for each of the two angles. The values for $F 23$, Rk only change for the connection with M16 screws.
In other words, the values in italics must be used if CLI system brackets are fitted to the top and bottom of the ceiling.

## Gurotec <br> Wood connector

## EXAMPLES OF COMBINATIONS



KonstruX + Angle-bracket screw $5 \times 60 \mathrm{~mm}$
KonstruX + IdeeFix


KonstruX + Angle-bracket screw + IdeeFix



## SHEARING ANGLE

Connector developed for modern timber construction to absorb shear forces

Shearing angle

Suitable for use with: Bolt anchor, Rock concrete screw, Angle-bracket screw Paneltwistec, Anchor nails, Pressure plate

| Art. no. | Dimensions $[\mathrm{mm}]$ | Material | Material thickness $[\mathrm{mm}]$ | PU |
| :--- | :--- | :--- | :--- | ---: |
| 954112 | $230 \times 120$ | S250 Galvanised | 3 | 1 |

ADVANTAGES Suitable for use with:

- Many different fields of application
- For installation in timber-concrete, as well as timber-timber connections
- Very high shear load-bearing capacity
- Fewer connectors required
- In combination with the pressure plate, the following tensile forces can be absorbed when fixing in concrete.


## INSTRUCTIONS FOR USE

6 slanted screw connection holes and 41 holes, which are optionally intended for angle-bracket screws (ABSs) or anchor nails, are provided for anchoring in wood.
Depending on the application, we have provided two additional partial utilisations of the fixing holes which are also available as static-type calculations. Anchoring in concrete is carried out using the holes ( $\varnothing 14 \mathrm{~mm})$ provided for this purpose with our rock concrete screw $\varnothing 12,5 \mathrm{~mm}$ or bolt anchors $\varnothing 12 \mathrm{~mm}$.

| Art. no. | Dimensions [mm] | Material | Material thickness [mm] | PU |
| :--- | :--- | :--- | :--- | ---: |
| 954111 | $230 \times 70$ | S235 Gavanised | 12 | 1 |



Shearing angle for fixing a wall to the concrete foundation.

## SHEARING ANGLE - STATIC FULL UTILISATION VALUES



| Lood direction F2/F3 |  |  |  |  |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Connection Timber-Timber |  |  |  |  |  |  |
| Vertical leg connection | Anchor nails $04 \times 40 \mathrm{n}=41$ | Anchor nails $¢ 4 \times 50 \mathrm{n}=41$ | Anchor nails $04 \times 60 \mathrm{n}=41$ | Angle-bracket screw $05 \times 40 \mathrm{n}=41$ | Angle-bracket screw $05 \times 50 \mathrm{n}=41$ | Angle-bracket screw $05 \times 60 n=41$ |
|  | Paneltwistec CH $05 \times 120 \mathrm{n}=6$ |  |  |  |  |  |
| Horizontal leg connection | Anchor nails $04 \times 40 \mathrm{n}=41$ | Anchor nails $04 \times 50 \mathrm{n}=41$ | Anchor nails $04 \times 60 \mathrm{n}=41$ | Angle-bracket screw $05 \times 40 \mathrm{n}=41$ | Angle-bracket screw $05 \times 50 \mathrm{n}=41$ | Angle-bracket screw $05 \times 60 n=41$ |
|  | Paneltwistec $\mathrm{CH} 05 \times 120 \mathrm{n}=6$ |  |  |  |  |  |
| Char. Shear carrying capacity [ kN ] | 37,3 | 44,3 | 47,9 | 41,9 | 44,6 | 47,6 |
| Char. Shear carrying capacity [KN] (Use of Sonotec SK04) | 28,9 | 34,4 | 37,4 | 32,7 | 34,8 | 37,1 |

The load-bearing capacities were determined based on ETA-19/0020 Characteristic load-bearing capacity in kN , wood strength class $350 \mathrm{~kg} / \mathrm{m}^{3}$ char. Gross density.
The minimum distances between the connectors and the edges according to EC5 must be complied with.

| Lood direction F2/F3 |  |  |  |  |  |  |  |  |  |  |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Connection Timber-Concrete |  |  |  |  |  |  |  |  |  |  |  |  |
| Vertical leg connection | $\begin{aligned} & \text { Anchor nails } \\ & 04 \times 40 \mathrm{n}=41 \end{aligned}$ | Anchor noils $04 \times 40 n=41$ | Anchor nails $04 \times 50 \mathrm{n}=41$ | $\begin{aligned} & \text { Anchor nails } \\ & 04 \times 50 \mathrm{n}=41 \end{aligned}$ | Anchor nails $04 \times 60 \mathrm{n}=41$ | $\begin{aligned} & \text { Anchor nails } \\ & 04 \times 60 \mathrm{n}=41 \end{aligned}$ | Angle-bracket <br> screw $05 \times 40 n=41$ | Angle-bracket <br> screw <br> $05 \times 40 n=41$ | Angle-bracket <br> screw $05 \times 50 \mathrm{n}=41$ | Angle-bracket screw $05 \times 50 \mathrm{n}=41$ | Angle-bracket <br> screw <br> $05 \times 60 n=41$ | Angle-bracket <br> screw $05 \times 60 n=41$ |
|  | Paneltwistec $\mathrm{CH} 05 \times 120 \mathrm{n}=6$ |  |  |  |  |  |  |  |  |  |  |  |
| Horizontal leg connection | Rock concrete screws $012,5 \times 120 \mathrm{n}=2$ | $\begin{gathered} \text { Bolt anchor } \\ 012 \times 110 n=2 \end{gathered}$ | Rock concrete screws $012,5 \times 120 \mathrm{n}=2$ | $\begin{gathered} \text { Bolt anchor } \\ 012 \times 110 n=2 \end{gathered}$ | $\begin{gathered} \text { Rock } \\ \text { concrete screws } \\ 012,5 \times 120 \mathrm{n}=2 \end{gathered}$ | $\begin{aligned} & \text { Bolt anchor } \\ & 012 \times 110 n=2 \end{aligned}$ | Rock concrete screws $012,5 \times 120 \mathrm{n}=2$ | $\begin{gathered} \text { Bolt anchor } \\ 012 \times 110 n=2 \end{gathered}$ | Rock concrete screws $012,5 \times 120 \mathrm{n}=2$ | $\begin{gathered} \text { Bolt anchor } \\ 012 \times 110 n=2 \end{gathered}$ | Rock concrete screws $012,5 \times 120 \mathrm{n}=2$ | $\begin{aligned} & \text { Bolt anchor } \\ & 012 \times 110 \mathrm{n}=2 \end{aligned}$ |
|  | incl. pressure plate $230 \times 70$ |  |  |  |  |  |  |  |  |  |  |  |
| Char. Shear carrying capacity [kN] | 37,3 | 23,4 | 44,3 | 23,4 | 47,9 | 23,4 | 41,9 | 23,4 | 44,6 | 23,4 | 47,6 | 23,4 |

The lood-bearing capacities were determined based on ETA-19/0020. Characterisic load-bearing capacity in kN, wood strength class $350 \mathrm{~kg} / \mathrm{m}^{3}$ char. Gross density.
The minimum distances between the connectors and the edges according to EC5 must be complied with.
Please note: Verity the assumptions made. The stated values, and type and number of joining devices are based on preliminary measurements. Projects are to be dimensioned exclusively by authorised persons in accordance with the State Building Code. As per LBuuO, please contact a qualified structural engineer for a paid proof of stability. We will be happy to refer you to someone.

## PARTIAL UTIIISATION 1



| Lood direction F2/F3 |  |  |  |  |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Connection Timber-Iimber |  |  |  |  |  |  |
| Vericial leg connection | Anclor nils $84 \times 40 \mathrm{n}=34$ | Anchor nails $04 \times 50 \mathrm{n}=34$ | Anchor nails $04 \times 60 \mathrm{n}=34$ | Angle-bracket screw $05 \times 40 n=34$ | Angle-bracket screw $05 \times 50 n=34$ | Angle-bracket screw $05 x 60 \mathrm{n}=34$ |
| Paneltwistec CH $05 \times 120 \mathrm{n}=6$ |  |  |  |  |  |  |
| Horizontal leg comection | Anchor nails $84 \times 40 \mathrm{n}=34$ | Anchor nails $04 \times 50 \mathrm{n}=34$ | Anchor nails $04 \times 60 \mathrm{n}=34$ | Angle-bracket screw $05 \times 40 n=34$ | Angle-bracket screw $05 \times 50 n=34$ | Angle-bracket screw $05 \times 60 \mathrm{n}=34$ |
| Panelwistec CH $05 \times 120 \mathrm{n}=6$ |  |  |  |  |  |  |
| Chor:Shearing capaity [KN] | 29,1 | 34,6 | 37,4 | 32,7 | 34,9 | 37,2 |
| Char:-shearing capacity [k]) (Use Sonotes SS04) | 22,6 | 26,9 | 29,4 | 25,5 | 27,2 | 29 |


| Lood direction F2/F3 |  |  |  |  |  |  |  |  |  |  |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Connection Timber-Iimber |  |  |  |  |  |  |  |  |  |  |  |  |
| Verical leg connection | $\begin{gathered} \text { Anchor nails } \\ 04 \times 40 \\ n=34 \end{gathered}$ | Anchor nails <br> $04 \times 40$ <br> n=34 | $\begin{gathered} \text { Anchor nails } \\ 04 \times 50 \\ n=34 \end{gathered}$ | $\begin{aligned} & \text { Anthor nils } \\ & 04 \times 50 \\ & n=34 \end{aligned}$ | $\begin{aligned} & \text { Anthor nils } \\ & 04 \times 60 \\ & n=34 \end{aligned}$ | Anchor nails <br> $04 \times 60$ <br> n=34 | Angle-bracker <br> screw <br> $05 \times 40$ <br> $n=34$ | $\begin{gathered} \text { Angleb-bracket } \\ \text { screw } \\ 05 \times 40 \\ n=34 \end{gathered}$ | $\begin{gathered} \text { Angle-hracket } \\ \text { strew } \\ 05 \times 50 \\ n=34 \end{gathered}$ | $\begin{gathered} \text { Angle-bracket } \\ \text { strew } \\ 05 \times 50 \\ n=34 \end{gathered}$ | Angle-bracker <br> screw <br> $05 \times 60$ <br> $n=34$ |  |
| Paneltwistec CH $05 \times 120 \mathrm{n}=6$ |  |  |  |  |  |  |  |  |  |  |  |  |
| Horizontal leg comenecion | Rock concrete $012,5 \times 120$ $012,5 \times 120$ n=2 | $\begin{aligned} & \text { Bolt anthor } \\ & 012 \times \times 10 \\ & n=2 \end{aligned}$ | Rock concrete screw $012,5 \times 120$ n=2 | $\begin{aligned} & \text { Bolt anthor } \\ & 012 \times 10 \\ & n=2 \end{aligned}$ | Rock concrete screw $012,5 \times 120$ n=2 | $\begin{aligned} & \text { Bolt anchor } \\ & 012 \times \times 10 \\ & n=2 \end{aligned}$ | Rock concrete -125 $012,5 \times 12$ $\mathrm{n}=2$ | $\begin{aligned} & \text { Bot anchor } \\ & 012 \times 110 \\ & n=2 \end{aligned}$ | Rock concrete screw <br> $012,5 \times 120$ <br> $\mathrm{n}=2$ | $\begin{gathered} \text { Bolt anchor } \\ 012 \times 110 \\ \mathrm{n}=2 \end{gathered}$ | Rock concrete 012,5 $012,5 \times 120$ $\mathrm{n}=2$ | $\begin{aligned} & \text { Bolt anthor } \\ & 012 \times \times 10 \\ & n=2 \end{aligned}$ |
| inc. pressure plate $230 \times 70 \mathrm{~mm}$ |  |  |  |  |  |  |  |  |  |  |  |  |
| Char:shearing capacity [kN] | 29,1 | 23,4 | 34,6 | 23,4 | 37,4 | 23,4 | 32, | 23,4 | 34,9 | 23,4 | 37,2 | 23,4 |

The load-bearing capactities were determined based on ETA-19/O020. Characterisic load-bearing capacity in KN , wood strenght class $350 \mathrm{~kg} / \mathrm{m}^{3}$ char. Gross density. The minimum distances between the connectors and the edges according to E( 5 must be complied with.
Please note: Verify the assumptions made. The stated values, und type and number of joining devices are based on preiminary measurements. Projects are to be dimensioned exclusively by authorised persons in accordance wihh the State Building Code. As per LBauO, please contact a qualified structural engineer for a paid proof of stability. We will be happy to refer you to someone.


## Note

All values given refer to the drilling pattern shown. We recommend using this as it has a considerably higher shear carrying capacity compared to the rear holes.

## PARTIAL UTILISATION 2



| Lood direction F2/F3 |  |  |  |  |  |  |  |  |  |  |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Connection Timber-Iimber |  |  |  |  |  |  |  |  |  |  |  |  |
| Verital leg connection |  |  | Anctor noils $94 x$ | 40n=29 Anchor | hor nails $9 \times 50 \mathrm{n}=29$ | Anchor nails | $1504 \times 60 \mathrm{n}=29$ | Angle-brackele $05 \times 40 n=$ |  | $\begin{gathered} \text { Angle-bracket screw } \\ 05 \times 50 \mathrm{n}=29 \end{gathered}$ |  | $\begin{aligned} & \text { bracketescrew } \\ & \times 60 n=29 \end{aligned}$ |
|  |  |  | Panelwistec CH $05 \times 120 \mathrm{n}=4$ |  |  |  |  |  |  |  |  |  |
| Horizontal leg comnection |  |  | Anctor noils 94 | 40n=29 Anchor | hor nails $9 \times 50 \mathrm{n}=29$ | Anctor nails | $1504 \times 60 \mathrm{n}=29$ | $\begin{aligned} & \text { Angle-hrackenten } \\ & 05 \times 40 \text { n } \end{aligned}$ |  | $\begin{gathered} \text { Angle-bracket screw } \\ 05 \times 50 n=29 \end{gathered}$ |  | $\begin{aligned} & \text { bracketescrew } \\ & \times 60 n=29 \end{aligned}$ |
|  |  |  | Panelwisitec CH05x 120n=4 |  |  |  |  |  |  |  |  |  |
| Char. Shear carrying capaity [KN] |  |  | 23,6 |  | 28,0 | 30,4 |  | 26,5 |  | 28,3 |  | 30,1 |
| Char. Shear carrying capaciy [KN] (Use of Sonotes SK04) |  |  | 18,3 |  | 21,8 | 23,9 |  | 20,7 |  | 22,1 | 23,5 |  |
| Lood direction F2/F3 |  |  |  |  |  |  |  |  |  |  |  |  |
| Comenetion Timber-Conceite |  |  |  |  |  |  |  |  |  |  |  |  |
| Verical leg connection | $\begin{gathered} \text { Anchor nails } \\ 04 \times 40 \mathrm{n}=29 \end{gathered}$ | $\begin{gathered} \text { Anchor nails } \\ 04 \times 40 \mathrm{n}=29 \end{gathered}$ | $\begin{gathered} \text { Anchor nails } \\ 04 \times 50 \mathrm{n}=29 \end{gathered}$ | $\begin{gathered} \text { Anchor nails } \\ 04 \times 50 \mathrm{n}=29 \end{gathered}$ | $\begin{aligned} & \text { Anchor nails } \\ & 04 \times 60 \mathrm{n}=29 \end{aligned}$ | Anchor nails $4 \times 60 \mathrm{n}=29$ | $\begin{aligned} & \text { Angle-bracket } \\ & \text { screw } \\ & 05 \times 40 \mathrm{n}=29 \end{aligned}$ | $\begin{gathered} \text { Angle-bracket } \\ \text { strew } \\ 05 \times 40 \mathrm{n}=29 \end{gathered}$ | Angle-bracket screw $05 \times 50 n=29$ | $\begin{gathered} \text { Angle-bracket } \\ \text { screw } \\ 05 \times 50 \mathrm{n}=29 \end{gathered}$ | Angle-bracket screw $05 \times 60 n=29$ | $\begin{gathered} \text { Angle-hracket } \\ \text { screw } \\ 05 \times 60 \mathrm{n}=29 \end{gathered}$ |
|  | Panelwistec $\mathrm{CH} 05 \times 120 \mathrm{n}=4$ |  |  |  |  |  |  |  |  |  |  |  |
| Horiontal leg comnection | $\begin{gathered} \text { Rock } \\ \text { concefe screws } \\ 012,5 \times 120 \mathrm{n}=2 \end{gathered}$ | $\begin{aligned} & \text { Boltanthor } \\ & 012 \times 110 n=2 \end{aligned}$ | Rock concret screws <br> 012,5 $\times 120 \mathrm{n}=2$ | $\begin{aligned} & \text { Boltantior } \\ & 012 \times 110 \mathrm{n}=2 \end{aligned}$ | $\begin{gathered} \text { Rock } \\ 2 \begin{array}{c} \text { concete screves } \\ \otimes 12,5 \times 120 \mathrm{n}=2 \end{array} \end{gathered}$ | $\begin{aligned} & \text { Bolt anchor } \\ & 012 \times 110 \mathrm{n}=2 \end{aligned}$ | Rock conceies screws $0125 \times 120 \mathrm{n}=2$ | $\begin{gathered} \text { Bolt anchor } \\ 012 \times 110 n=2 \end{gathered}$ | Rock concrete screws $012,5 \times 120 \mathrm{n}=2$ | $\begin{gathered} \text { Bolt anchor } \\ 012 \times 110 \mathrm{n}=2 \end{gathered}$ | Rock concrete screws $012,5 \times 120 \mathrm{n}=2$ | $\begin{gathered} \text { Bolt anchor } \\ 012 \times 110 \mathrm{n}=2 \end{gathered}$ |
|  | ind. pressure plate $330 \times 70$ |  |  |  |  |  |  |  |  |  |  |  |
| Char:Shering copacity [kN] | 23,6 | 19,3 | 28,0 | 22,8 | 30,4 | 23,4 | 26,5 | 23,4 | 28,3 | 23,4 | 30,1 | 23,4 |

The load-bearing capacities were determined based on ETA-19/0020. Characterisic load-bearing capacity in KN, wood strength class $350 \mathrm{~kg} / \mathrm{m}^{3}$ char. Gross density.
The minimum distances between the connectors and the edges according to EC 5 must be complied with.
Please note: Verity the assumptions made. The stated values, and type and number of joining devices are bassed on preliminary measurements. Projects are to be dimensioned exclusively by authorised persons in accordance with the State Building Code. As per LBauO, please contact a qualified structural engineer for a paid proof of stability. We will be happy to refer you to someone.

## HB/HH FLAT SHEARING ANGLE



## ADVANTAGES

- For assembly on concrete
- Very high shear load-bearing capacity
- Fewer connectors required
- In combination with the pressure plate, the following tensile forces can be absorbed when fixing in concrete.

| Art no. | Dimensions [mm] ${ }^{\text {a/ }}$ | Material | Material thickness [mm] | PU |
| :---: | :---: | :---: | :---: | :---: |
| 954087 | $230 \times 100 \times 70$ | 5250 Gavanised | 3 | 1 |
| Pressure plate |  |  |  |  |
| 954111 | $230 \times 68$ | S233 Gavanised | 12 | 1 |
| a) Length $x$ |  |  |  |  |

The HB flat shearing angle (wood-concrete) is a bracket connector for absorbing shearing forces that was specifically developed for modern timber construction. Its low height means it is ideally suited to use in timber frame construction. The pressure plate allows the occurring loads to be optimally conducted into the concrete.


| Art. no. | Dimensions $[\mathrm{mm}]^{\text {a) }}$ | Material | Material thickness $[\mathrm{mm}]$ | PU |
| :--- | :--- | :--- | :--- | ---: |
| 954088 | $230 \times 70$ | $S 250$ Galvanised | 3 | 1 |

## a) Length $x$ Width

The HH flat shearing angle (wood-wood) is a bracket connector for absorbing shearing forces that was specifically developed for modern timber construction. Its low height means it is ideally suited to use in timber frame construction.

## ADVANTAGES

- For assembly on timber
- Very high shear load-bearing capacity
- Fewer connectors required
- Especially high tensile forces can be absorbed in combination with the KonstruX



## HB FLAT SHEARING ANGLE - STATIC VALUES



The load-bearing capacities were determined based on ETA-19/O02O. Characterisicic load-bearing capacity in kN , wood strenght class $350 \mathrm{~kg} / \mathrm{m}^{3}$ char. Gross density. The minimum distances between the connectors and the edges according to EC5 must be complied with.
Attention: Verify the assumptions made. The stated values, and type and number of joining devices are based on preliminary measurements. Projects are to be dimensioned exclusively by authorised persons in accordance with the State Building Code. As per LBauO, please contact a qualified structural engineer for a paid proof of stability. We will be happy to refer you to someone.

HH FLAT SHEARING ANGLE - STATIC VALUES


| Lood direction F2/F3/F4 |  |
| :---: | :---: |
| Connection Wood-Wood |  |
| Vericicl leg connection | Angle-bracket screw $05 \times 25 n=3$ <br> Panelwistec CH $05 \times 120$ n $=12$ |
| Horizontid leg comnection | Angle-bracketescrew $55 \times 25 \mathrm{n}=3$ <br> Panelwisisec CH $05 \times 120 \mathrm{n}=12$ |
| Char:shering capacity $\mathrm{F}_{23}[\mathrm{KN}]$ | 40,0 |
| Char:-shearing capacity $F_{23}[\mathrm{KN}]$ (Use Sonotec SKO4) | 36,0 |
| Char: load.bearing capacity F4 [KN] | 40,0 |
| Char:- lood.beering cppacity $F_{23}[\mathrm{KN}]$ (Use Sonotec SKO4) | 36,0 |



The lood-bearing capacities were determined based on ETA-19/0020. Characterisicic load-bearing capacity in KN , wood strength class $350 \mathrm{~kg} / \mathrm{m}^{3}$ char. Gross density. The minimum distances between the connectiors and the edges according to FC 5 must be complied with.
Attention: Verify the assumptions made. The stated values, and type and number of joining devices are based on preliminary meassurements. Projects are to be dimensioned exclusively by authorised persons in accordance with the State Building Code. As per LBauO, please contact a qualified structural engineer for a paid proof of stability. We will be happy to refer you to someone.

## SHEARING PLATE

Connector developed for modern timber construction to absorb shear forces

Shearing plate

Suitable for use with:
Paneltwistec CH $5 \times 120 \mathrm{~mm}$,
Rock concrete screw, Bolt anchor,
Anchor nails and
Angle-bracket screw

| Art. no. | Dimensions $[\mathrm{mm}]$ | Material | Material thickness $[\mathrm{mm}]$ | PU |
| :--- | :--- | :--- | :--- | ---: |
| 954113 | $230 \times 240$ | S250 Galvanised | 3 | 1 |

INSTRUCTIONS FOR USE
6 slanted screw connection holes and 41 holes each side, which are optionally intended for angle-bracket screws
(ABSs) or anchor nails, are provided for anchoring in wood. Depending on the application, we have provided two additional partial utilisations of the fixing holes which are also available as static-type calculations. Anchoring in concrete is carried out using the holes ( $\varnothing 14 \mathrm{~mm}$ ) provided for this purpose with our Rock concrete screw $\varnothing 12,5 \mathrm{~mm}$ or bolt anchors $\varnothing 12 \mathrm{~mm}$.

ADVANTAGES

- Very high shear load-bearing capacity
- Many different fields of application
- For installation in wood-concrete, and wood-wood connections
- Fewer connectors required



| Lood direction F2/3 |  |  |  |  |  |  |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Timber/Timber | Fixing in the sole plate and solid timber ceiling |  |  |  |  |  |  | Steel |
|  | Fixing in the sole plate and solid fimber ceiling |  |  |  |  |  |  |  |
|  | Anchor noils |  |  | Angle-bracket screw |  |  | Paneltwistec CH | 5250 |
| Dimensions [mm] | $4 \times 40$ | $4 \times 50$ | $4 \times 60$ | $5 \times 40$ | $5 \times 50$ | $5 \times 60$ | $5 \times 120$ |  |
| Quantity ( n ) |  | 41 |  |  | 41 |  | 6 |  |
| Char. shearing capacity [KN] | 37,3 | 44,3 | 47,9 | 41,9 | 44,6 | 47,6 | - | 156 |


| Load direction F2/3 |  |  |  |  |  |  |  |  |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Timber / Concrete | Fixing in the sole plate |  |  |  |  |  |  | Fixing in the concrete ceiling |  | Steel |
|  | Joining devices |  |  |  |  |  |  |  |  |  |
|  | Anchor noils |  |  | Angle-bracket screw |  |  | Paneltwistec CH | Rock concrete screws | Bolt anchor | 5250 |
| Dimensions [mm] | $4 \times 40$ | $4 \times 50$ | $4 \times 60$ | $5 \times 40$ | $5 \times 50$ | $5 \times 60$ | $5 \times 120$ | 012,5 | 012 |  |
| Quantity ( n ) |  | 41 |  |  | 41 |  | 6 | 2 | 2 |  |
| Char. shearing capacity [kN] | 37,3 | 44,3 | 47,9 | 41,9 | 44,6 | 47,6 | - | 21,8 | 12,2 | 156 |

The lood-bearing capaciities were determined on the basis of ETA-19/0020. Characterisic load-bearing capacity in kN , wood strength class $350 \mathrm{~kg} / \mathrm{m}^{3}$ char. gross density.
The minimum distances between the connectors and the edges according to $E C 5$ must be complied with. Boundary bearing force according to $E\left(3: F_{b}, R_{k} 014 \mathrm{~mm}=93,75 \mathrm{kN}\right.$
Please note: Verify the assumptions made. The stated values, and type and number of joining devices are based on preliminary measurements. Projects are to be dimensioned exclusively by authorised persons in accordance with the State Building Code. As per LBauO, please contact a qualified structural engineer for a paid proof of stability. We will be happy to refer you to someone.

| Art. no. | Dimensions | Material | PU |
| :--- | :--- | :--- | :--- | :--- |
| 200240 | $4,0 \times 40$ | Galvanised | 250 |
| 200241 | $4,0 \times 50$ | Galvanised | 250 |
| 200242 | $4,0 \times 60$ | Galvanised | 250 |

[^0]
## PARTIAL UTIIISATION 1



|  |  |  | directio |  |  |  |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Timber/Timber | Fixing in the sole plate and solid timber ceiling |  |  |  |  |  |  | Steel |
|  | Joining devices |  |  |  |  |  |  |  |
|  | Anchor nails |  |  | Angle-bracket screw |  |  | Paneltwistec <br> CH | S250 |
| Dimensions [mm] | $4 \times 40$ | $4 \times 50$ | $4 \times 60$ | $5 \times 40$ | $5 \times 50$ | $5 \times 60$ | $5 \times 120$ |  |
| Quantity ( n ) |  | 34 |  |  | 34 |  | 6 |  |
| Char. shearing capacity [KN] | 29,1 | 34,6 | 37,4 | 32,7 | 34,9 | 37,2 | - | 156 |


|  |  |  |  | Lood | F2/3 |  |  |  |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Timber / Concrete | Fixing in the sole plate |  |  |  |  |  |  | Fixing in the | te ceiling | Steel |
|  | Joining devices |  |  |  |  |  |  |  |  |  |
|  | Anchor nails |  |  | Angle-bracket screw |  |  | Paneltwistec CH | Rock-concrete screws | Bolt anchor | S250 |
| Dimensions [mm] | $4 \times 40$ | $4 \times 50$ | $4 \times 60$ | $5 \times 40$ | $5 \times 50$ | $5 \times 60$ | $5 \times 120$ | 012,5 | 012 |  |
| Quantity (n) | 34 |  |  | 34 |  |  | 6 | 2 | 2 |  |
| Char. shearing capacity [KN] | 29,1 | 34,6 | 37,4 | 32,7 | 34,9 | 37,2 | - | 20,5 | 11,6 | 156 |

The lood-bearing capacities were determined on the basis of ETA-19/0020. Characteristic load-bearing capacity in kN , wood strength class $350 \mathrm{~kg} / \mathrm{m}^{3}$ char. gross density. The minimum edge distances for joining devices according to E( 5 must be observed.

## PARTIAL UTILISATION 2



| Load direction F2/3 |  |  |  |  |  |  |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Timber/Timber | Fixing in the sole plate and solid timber ceiling |  |  |  |  |  |  | Steel |
|  | Joining devices |  |  |  |  |  |  |  |
|  | Anchor nails |  |  | Angle-bracket screw |  |  | Paneltwistec <br> CH | 5250 |
| Dimensions [mm] | $4 \times 40$ | $4 \times 50$ | $4 \times 60$ | $5 \times 40$ | $5 \times 50$ | $5 \times 60$ | $5 \times 120$ |  |
| Quantity (n) |  | 29 |  |  |  |  | 4 |  |
| Char. shearing capacity [KN] | 23,6 | 28,0 | 30,4 | 26,5 | 28,3 | 30,1 | - | 156 |


|  |  |  |  | oad dire |  |  |  |  |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Timber / Concrete | Fixing in the sole plate |  |  |  |  |  |  | Fixing in the concrete ceiling |  | Steel |
|  | Joining devices |  |  |  |  |  |  |  |  |  |
|  | Anchor nails |  |  | Angle-bracket screw |  |  | Paneltwistec SK | Rock concrete screws | Bolt anchor | 5250 |
| Dimensions [mm) | $4 \times 40$ | $4 \times 50$ | $4 \times 60$ | $5 \times 40$ | $5 \times 50$ | $5 \times 60$ | $5 \times 120$ | 012,5 | 012 |  |
| Quantity (n) | 29 |  |  | 29 |  |  | 4 | 2 | 2 |  |
| Char. shearing capacity [kN] | 23,6 | 28,0 | 30,4 | 26,5 | 28,3 | 30,1 | - | 14,4 | 11,2 | 156 |

The lood-bearing capacities were determined on the basis of EIA-19/0020. Characteristic lood-bearing capacity in kN , wood strength class $350 \mathrm{~kg} / \mathrm{m}^{3}$ char. gross density. The minimum edge distances for joining devices according to EC 5 must be observed.
Please note: Verity the assumptions made. The stated values, and type and number of joining devices are based on preliminary measurements. Projects are to be dimensioned exclusively by outhorised persons in accordance with the State Building Code. As per LBauO, please contact a qualified structural engineer for a paid proof of stability. We will be happy to refer you to someone.

## TENSION STRAP HB60/70

Connector developed for modern timber construction to absorb tensile- and shear forces

Tension strap HB60 / HB70


| Art. no. | Dimensions [mm] | Material | Material thickness[mm] | PU |
| :--- | :--- | :--- | :--- | ---: |
| 954095 | $506 \times 60$ | S250 Gavavised | 3 | 1 |
| 954097 | $506 \times 70$ | S250 Gavanaised | 3 | 1 |

ADVANTAGES

- Many different fields of application
- For installation in wood and concrete
- Very high shear load-bearing capacity
- Fewer connectors required


## INSTRUCTIONS FOR USE

Anchoring in wood is carried out using $5 \times 120 \mathrm{~mm}$ countersunk-head screws at an angle of $45^{\circ}$. A non-positive connection is created between the screw head and draw shackle thanks to the speciically designed holes, which can also be used as screw guides.
The tension strap HB 70 also has 2 holes $(\varnothing 5 \mathrm{~mm})$ which are provided for a $90^{\circ}$ screw connection. Anchoring in concrete is carried out using the holes ( $\varnothing 14 \mathrm{~mm})$ provided for this purpose with our rock concrete screw or bolt anchors. Detailed installation instructions can be found in the corresponding product data sheets.



| Loaddirection Fl |  |  |  |  |  |  |  |  |  |  |  |  |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Connection Timber-Concrete |  |  |  |  |  |  |  |  |  |  |  |  |  |  |
| Wood side connection | Panelwwistec CH $05 \times 120 \mathrm{n}=9$ |  |  |  | Anchor noils $84 \times 40 \mathrm{n}=6$ |  |  |  | Anctor noils $84 \times 50 \mathrm{n}=6$ |  |  |  | Anctor nili $54 \times 60 \mathrm{n}=6$ |  |
| Concrete side conection | Rock concrete screws <br> $012,5 \times 120$ <br> $\mathrm{n}=1$ | $\begin{gathered} \text { Rock } \\ \text { concrete scews } \\ 012,5 \times 120 \\ n=2 \end{gathered}$ | $\begin{aligned} & \text { Bolt anchor } \\ & 012 \times 110 \\ & n=1 \end{aligned}$ | $\begin{aligned} & \text { Bolt anthor } \\ & 012 \times 10 \\ & n=2 \end{aligned}$ | $\begin{gathered} \text { Rock } \\ \text { concretes sfews } \\ \emptyset 12,5 \times 120 \\ n=1 \end{gathered}$ | $\begin{gathered} \text { Rock } \\ \text { concrete screws } \\ 012,5 \times 120 \\ n=2 \end{gathered}$ | $\begin{aligned} & \text { Botit anchor } \\ & 012 \times \times 10 \\ & n=1 \end{aligned}$ | $\begin{gathered} \text { Bolt anchor } \\ 012 \times 10 \\ \mathrm{n}=2 \end{gathered}$ | Rock concrete scevens <br> $012,5 \times 120$ <br> $\mathrm{n}=1$ | $\begin{gathered} \text { Rock } \\ \text { concretes screws } \\ \emptyset 12,5 \times 120 \\ n=2 \end{gathered}$ | $\begin{aligned} & \text { Bolt anchor } \\ & 012 \times 10 \\ & n=1 \end{aligned}$ | $\begin{aligned} & \text { Bolt anchor } \\ & 0 \mid 12 \times 110 \\ & n=2 \end{aligned}$ | $\begin{gathered} \text { Rock } \\ \text { concretes scews } \\ 012,5 \times 120 \\ n=1 \end{gathered}$ | Rock concefe screws <br> $012,5 \times 120$ <br> $\mathrm{n}=2$ |
| Char. Shear carrying capacity [KN] | 20, $8^{*}$ | 20,8* | 12,6 | 20,8* | 9,3 | 9,3 | 9,3 | 9,3 | 11,0 | 11,0 | 11,0 | 11,0 | 11,4 | 11,4 |


| Loaddirection Fl |  |  |  |  |  |  |  |  |  |  |  |  |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Connection Timber-Concrete |  |  |  |  |  |  |  |  |  |  |  |  |  |  |
| Wood side connection | Anchor nais $94 \times 60 \mathrm{n}=6$ |  | Angle-hracket screw $05 \times 40 \mathrm{n}=6$ |  |  |  | Angle-hracket screw $05 \times 50 \mathrm{n}=6$ |  |  |  | Angle-bracket screw $05 \times 60 \mathrm{n}=6$ |  |  |  |
| Concrete side connection | $\begin{aligned} & \text { Botanchor } \\ & 012 \times 10 \\ & \mathrm{n}=10 \end{aligned}$ | $\begin{gathered} \text { Boltanchor } \\ 012 \times 10 \\ n=2 \end{gathered}$ |  | Rock concrete screws <br> $012,5 \times 120$ <br> $\mathrm{n}=2$ | $\begin{aligned} & \text { Bolit anchor } \\ & 012 \times 110 \\ & n=1 \end{aligned}$ | $\begin{gathered} \text { Bolt anchor or } \\ 0012 \times 110 \\ \mathrm{n}=2 \end{gathered}$ | Rock concretes stews <br> $012,5 \times 120$ <br> $\mathrm{n}=1$ | Rock concrefe screws <br> $012,5 \times 120$ <br> $\mathrm{n}=2$ | $\begin{aligned} & \text { Bolt anchor } \\ & 012 \times 110 \\ & n=1 \end{aligned}$ | $\begin{aligned} & \text { Bolt anchor } \\ & 012 \times 10 \\ & n=2 \end{aligned}$ | $\begin{gathered} \text { Rock } \\ \text { concrefes screws } \\ 012,5 \times 120 \\ n=1 \end{gathered}$ | Rock concefes stews <br> $012,5 \times 120$ <br> n=2 | $\begin{aligned} & \text { Bolit anchor } \\ & 012 \times 110 \\ & n=1 \end{aligned}$ | $\begin{aligned} & \text { Bolit anthor } \\ & 0.12 \times 110 \\ & n=2 \end{aligned}$ |
| Char. Shear carrying capacity $[k N]$ | 11,4 | 11,4 | 10,9 | 10,9 | 10,9 | 10,9 | 12,0 | 12,0 | 12,0 | 12,0 | 13,1 | 13,1 | 12,6 | 13,1 |

* Concrete edgg brackout for racked concrefe

The lood-bearing capaciifes were determined bused on EA-19/0020. Charateresticic load.bearing capacity in KN , wood strenght clas $350 \mathrm{~kg} / \mathrm{m}^{3}$ char: Gross density.
The minimum distoneses between the coonnectors and the edges cccording to $E(5$ mus be complied with.

As per BuuO, plesse contact a qualified structural engineer for a paid proof of stability. We will be happy to refer you to someone.

## TENSION STRAP HB7O - STATIC VALUES



| Lood direction Fl |  |  |  |  |  |  |  |  |  |  |  |  |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Connection Timber-Concrete |  |  |  |  |  |  |  |  |  |  |  |  |  |  |
| Wood side connection | Panelwistec $\mathrm{CH} 05 \times 120 \mathrm{n}=12$ |  |  |  | Anthor nils $84 \times 40 \mathrm{n}=8$ |  |  |  | Anchor noils $84 \times 50 \mathrm{n}=8$ |  |  |  | Anthor niils $64 \times 60 \mathrm{n}=8$ |  |
| Concrete side comection | $\begin{gathered} \text { Rock } \\ \text { concrete screws } \\ 012,5 \times 120 \\ n=1 \end{gathered}$ | $\begin{gathered} \text { Rock } \\ \text { concrete screws } \\ 012,5 \times 120 \\ n=2 \end{gathered}$ | $\begin{aligned} & \text { Bolt anthor } \\ & 012 \times 110 \\ & \mathrm{n}=1 \end{aligned}$ | $\begin{gathered} \text { Bolt anchor } \\ 012 \times \times 10 \\ n=2 \end{gathered}$ | Rock concrete sceurs <br> $012,5 \times 120$ <br> $\mathrm{n}=1$ | Rock concrete screws $012,5 \times 120$ $\mathrm{n}=2$ | $\begin{aligned} & \text { Bolt anhor } \\ & 012 \times 110 \\ & \mathrm{n}=1 \end{aligned}$ | $\begin{aligned} & \text { Bolt anchor } \\ & 012 \times \times 10 \\ & n=2 \end{aligned}$ | Rock concrete screws $012,5 \times 120$ $\mathrm{n}=1$ | Rock concrete sceuvs <br> $012,5 \times 120$ <br> $\mathrm{n}=2$ | $\begin{aligned} & \text { Bolt andor } \\ & 012 \times 110 \\ & n=1 \end{aligned}$ | $\begin{aligned} & \text { Bolt anhor } \\ & 012 \times 110 \\ & \mathrm{n}=2 \end{aligned}$ | $\begin{gathered} \text { Rock } \\ \text { concrete scews } \\ 012,5 \times 120 \\ \mathrm{n}=1 \end{gathered}$ | Rock concreie screws <br> $012,5 \times 120$ <br> $\mathrm{n}=2$ |
| Char. Sherer carning capaciry[kN] | 20,8* | 20,8* | 12,6 | 20,8* | 12,5 | 12,5 | 12,5 | 12,5 | 14,7 | 14,7 | 12,6 | 14,7 | 15,2 | 15,2 |


| Lood direction Fl |  |  |  |  |  |  |  |  |  |  |  |  |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Connection Timber-Concrete |  |  |  |  |  |  |  |  |  |  |  |  |  |  |
| Wood side connection | Anchor nails $94 \times 60 \mathrm{n}=8$ |  | Angle-bracket screw $05 \times 40 \mathrm{n}=8$ |  |  |  | Angle-bracket scree $05 \times 50 \mathrm{n}=8$ |  |  |  | Angle-hracket screw $05 \times 60 \mathrm{n}=8$ |  |  |  |
| Concrete side connetion | $\begin{aligned} & \text { Bolit anchor } \\ & 0012 \times 110 \\ & \mathrm{n}=1 \end{aligned}$ | $\begin{aligned} & \text { Bolt anchor } \\ & 012 \times 110 \\ & \mathrm{n}=2 \end{aligned}$ | $\begin{gathered} \text { Rock } \\ \text { concrefes screvs } \\ 012,5 \times 120 \\ n=1 \end{gathered}$ | Rock concete screws $012,5 \times 120$ $n=2$ | $\begin{aligned} & \text { Bolt anchor } \\ & 012 \times 110 \\ & \mathrm{n}=1 \end{aligned}$ | $\begin{aligned} & \text { Bolit anchor } \\ & 012 \times 110 \\ & n=2 \end{aligned}$ | $\begin{gathered} \text { Rock } \\ \text { concretescews } \\ 012,5 \times 120 \\ \mathrm{n}=1 \end{gathered}$ | Rock concrete screws $012,5 \times 120$ $\mathrm{n}=2$ | $\begin{aligned} & \text { Botitanhor } \\ & 012 \times 10 \\ & n=1 \end{aligned}$ | $\begin{aligned} & \text { Bolt anchor } \\ & 012 \times 110 \\ & n=2 \end{aligned}$ | Rock concrete scews <br> $012,5 \times 120$ <br> $\mathrm{n}=1$ | $\begin{gathered}\text { Rock } \\ \text { concciet sceuws } \\ 012,5 \times 120 \\ n=2\end{gathered}$ $=2$ | $\begin{aligned} & \text { Bolt anchor } \\ & 012 \times 110 \\ & \mathrm{n}=1 \end{aligned}$ | $\begin{aligned} & \text { Bolt anchor } \\ & 012 \times \times 10 \\ & n=2 \end{aligned}$ |
| Char. Shear carrying capacity [kN] | 12,6 | 15,2 | 17,2 | 17,1 | 12,6 | 17,1 | 18,2 | 18,2 | 12,6 | 18,2 | 19,0 | 19,0 | 12,6 | 19,0 |
| * Concrete edge bradkout for cracked concrete |  |  |  |  |  |  |  |  |  |  |  |  |  |  |
| The lood-bearing capacities were determined based on EA--19/0020. Characterisicic lood-bearing capacity in kN , wood 5 stenght closs $350 \mathrm{~kg} / \mathrm{m}^{3}$ char. Gross density. The minimum distances between the connectors and the edges according to E 55 mus be complied with. |  |  |  |  |  |  |  |  |  |  |  |  |  |  |
|  As per BBu0, plesse cortacta qualified structural engineer for a paid proof of stabilit. We vill be happy to refer you to someone. |  |  |  |  |  |  |  |  |  |  |  |  |  |  |

## TENSION STRAP HH60/70

For absorbing tensile forces and shearing forces developed for modern timber construction

Tension strap HH6O / HH7O


Tension straps HH 6 O / HH 70 for fastening wall- and ceiling elements.

## TENSION STRAP HH6O - STATIC VALUES



| Lood direction Fl |  |  |  |  |  |  |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Connection Timber-Timber |  |  |  |  |  |  |  |  |
| Leg connection 1 | $\begin{gathered} \text { Paneltwistec } \mathrm{CH} \varnothing 5 \times 120 \\ n=9 \end{gathered}$ | $\begin{gathered} \text { Anchor nails } \wp 4 \times 40 \\ n=6 \end{gathered}$ | $\begin{gathered} \text { Anchor nails } \emptyset 4 \times 50 \\ n=6 \end{gathered}$ | $\begin{gathered} \text { Anchor nails } \varnothing 4 \times 60 \\ n=6 \end{gathered}$ | Angle-bracket screw $05 \times 40 ; n=6$ | Angle-bracket screw $05 \times 50 ; n=6$ | Angle-bracket screw $05 \times 60 ; n=6$ | Steel |
| Leg connection 2 | $\begin{gathered} \text { Panelwwistec CH } \varnothing 5 \times 120 \\ n=9 \end{gathered}$ | $\underset{n=6}{\text { Anchor nails } \oslash 4 \times 40}$ | $\begin{gathered} \text { Anchor noils } \oint 4 \times 50 \\ n=6 \end{gathered}$ | $\begin{gathered} \text { Anchor nails } \wp 4 \times 60 \\ n=6 \end{gathered}$ | Angle-bracket screw $05 \times 40 ; n=6$ | Angle-bracket screw $05 \times 50 ; n=6$ | Angle-bracket screw $05 \times 60 ; n=6$ | S250 |
| Char. tensile capacity [kN] | 27 | 9,4 | 11 | 11,4 | 10,9 | 12 | 13,1 | 28,5 |

The load-bearing capacities were determined based on ETA-19/0020. Characterisicic load-bearing capacity in kN , wood strength lass $350 \mathrm{~kg} / \mathrm{m}^{3}$ char. Gross density. The minimum distances between the connectors and the edges according to EC 5 must be complied with.

Please note: Verify the assumptions made. The stated values, and type and number of joining devices are based on preliminary meassurements. Projects are to be dimensioned exclusively by authorised persons in accordance with the State Building Code. As per LBauO, please contact a qualified structural engineer for a paid proof of stability. We will be happy to refer you to someone.

## TENSION STRAP HH7O - STATIC VALUES



| Lood direction Fl |  |  |  |  |  |  |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Connection Timber-Timber |  |  |  |  |  |  |  |  |
| Leg connection 1 | $\begin{gathered} \text { Paneltwistec } \mathrm{CH} \varnothing 5 \times 120 \\ n=12 \end{gathered}$ | $\begin{gathered} \text { Anchor nails } \oint 4 \times 40 \\ n=8 \end{gathered}$ | $\begin{gathered} \text { Anchor nails } \oint 4 \times 50 \\ n=8 \end{gathered}$ | $\begin{gathered} \text { Anchor nails } 04 \times 60 \\ n=8 \end{gathered}$ | Angle-bracket screw $05 \times 40 ; n=8$ | Angle-bracket screw $05 \times 50 ; n=8$ | Angle-bracket screw $05 \times 60 ; n=8$ | Steel |
| Leg connection 2 | $\begin{gathered} \text { Panethwistec } \mathrm{CH} \varnothing 5 \times 120 \\ n=12 \end{gathered}$ | Anchor nails $\emptyset 4 \times 40$ $\mathrm{n}=8$ | $\begin{gathered} \text { Anchor nails } \emptyset 4 \times 50 \\ n=8 \end{gathered}$ | Anchor noils $04 \times 60$ $\mathrm{n}=8$ | Angle-bracket screw $05 \times 4$ $\mathrm{n}=8$ | 0 Angle-bracket screw 05 x $\mathrm{n}=8$ | Angle-bracket screw $05 \times 60$ $\mathrm{n}=8$ | 5250 |
| Char. tensile capacity [kN] | 35 | 12,5 | 14,7 | 15,2 | 17,1 | 18,2 | 19,4 | 37,4 |

The load-bearing capacities were determined based on ETA-19/0020. Characterisicic load-bearing capacity in kN , wood strength class $350 \mathrm{~kg} / \mathrm{m}^{3}$ char. Gross density. The minimum distances between the connectors and the edges according to EC 5 must be complied with.
Please note: Verify the assumptions made. The stated values, and type and number of joining devices are hassed on preliminary meassurements. Projects are to be dimensioned exclusively by authorised persons in accordance with the State Building Code. As per LBauO, please contact a qualified structural engineer for a paid proof of stability. We will be happy to refer you to someone.

## SHEAR WALL CONNECTOR

For the compensation of uneveness in construction elements

Shear wall connector

| Art. no. | Dimensions $[\mathrm{mm}]$ | Material | PU* |
| :--- | :--- | :--- | ---: |
| 800312 | $100 \times 19 \times 80$ | Cass steel | 1 |
| *Scope of delivery includes screws |  |  |  |

ADVANTAGES

- Allows high shear force transmission between the wall elements
- Compensates for unevenness between building elements
- Does not protrude from the wall

INSTRUCTIONS FOR USE
To install the shear wall connector, first cut a groove in each wall at the same height. The shear wall connector is then inserted into the milling and fixed with two screws. The flatress of the connector helps compensate for slight differences in height between the walls. The screw connection also pulls both walls horizontally to the connector, thus straightening out slight unevenness here as well.



## ASSEMBIY CONNECTOR

For connecting two timber construction elements in systems building

Assembly connector


| Art. no. | Dimensions $[\mathrm{mm}]^{0}$ | Material | PU* |
| :--- | :--- | :--- | ---: |
| 800272 | $32,7 \times 175 \times 29,7$ | GFK Polyamid | 50 |

a) Lenght $x$ Width $x$ Height
*incl. 150 screw per PU
The Eurotec assembly connector consists of two individual components that interlock during assembly. It serves as a preparatory element in system construction.

## ADVANTAGES

- Can be used regardless of weather conditions
- Easy assembly
- Quick and easy element positioning


## INSTRUCTIONS FOR USE

We recommend our Paneltwistec AG CH $6 \times 80 \mathrm{~mm}$ for the use of the assembly connector. It is flush-mounted in a groove positioned at any chosen location on the construction elements. Once the elements have been inserted, the assembly connector is hidden inside the wall. The assembly connector must have a screw inserted in every screw hole. Our assembly connector is designed purely for guidance purposes. It cannot be used to absorb forces.



## Note

The assembly connector is not a connector that should be exposed to large, permanent load -
it is only a mounting tool!

## MAGNUS HOOK CONNECTOR

Timber connector for main-secondary beam joints

Magnus hook connector


| Art. no. | Name | Dimensions [mm] ${ }^{\text {a/ }}$ | PU |
| :---: | :---: | :---: | :---: |
| 944874 | Magnus XS $30 \times 30$ | $30 \times 30 \times 9$ | 20 |
| 944875 | Magnus $50 \times 60$ | $50 \times 60 \times 13$ | 10 |
| 944876 | Magnus $50 \times 80$ | $50 \times 80 \times 13$ | 10 |
| 944877 | Magnus $550 \times 100$ | $50 \times 100 \times 13$ | 10 |
| 944878 | Magnus M70x 120 | $70 \times 120 \times 17$ | 10 |
| 944879 | Magnus M70x 140 | $70 \times 140 \times 17$ | 10 |
| 944880 | Magnus M70x 160 | $70 \times 160 \times 17$ | 10 |
| 944881 | Magnus M 70x 180 | $70 \times 180 \times 17$ | 10 |
| 944882 | Magnus L110 220 | $110 \times 220 \times 19$ | 4 |
| 944883 | Magnus LIIO 260 | $110 \times 260 \times 19$ | 4 |
| 944884 | Magnus LIIO 300 | $110 \times 300 \times 19$ | 4 |
| 944887 | Magnus $1110 \times 340$ | $110 \times 340 \times 19$ | 4 |
| 948888 | Magnus L110x 380 | $110 \times 380 \times 19$ | 4 |
| 944889 | MagnusL110x 580 | $110 \times 580 \times 19$ | 4 |

1 comnetor consists of 2 individual parts
a) $\mathrm{F}=$ Assembly thickness

ADVANTAGES

- Easy assembly
- High level of prefabrication
- Suitable for high joints
- Visible and hidden loads
- Milling cutter and milling and assembly iig available
- ECS calculation software for free preliminary calculation


## INSTRUCTIONS FOR USE

The Magnus hook connector should always be fully unscrewed to ensure an easy and safe installation. Whether surface-mounted or recessed, the milling and mounting iig shows the connector where to fit. Sides and end grain surfaces must be flat to avoid any deformation of the connector during the assembly.


1 Insert $90^{\circ}$ fully threaded screws and fix Magnus to the wood


2
Insert $45^{\circ}$ screws


3
Mount the secondary beam on the main beam;
use fixing screws to secure the joint against lifting out


4
Joint complete


Fully threaded screws

## OVERVIEW OF MAGNUS HOOK CONNECTORS



Magnus XS
Magnus S


Magnus M


Magnus L

| Art. no. | Name | Dimensions | PU* | Fully threaded screws ${ }^{\text {b }}$ |  | Fixing screws ${ }^{\text {b) }}$ |  | Main beam |  | Secondary beam surface-mounted |  | Secondary beam flush-mounted |  |  |  | characterisic load-bearing capacity Frke) |  |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
|  |  | WxHx ${ }^{\text {a) }}$ |  | Dimension | $n_{j e}$ Ver- | Dimension | $n_{\text {per }}$ | min. <br> WMB | min. <br> НMB | min. <br> WSB | min. <br> HSB | $\begin{aligned} & \min . \\ & W_{S B}() \end{aligned}$ | min. <br> HSB | $W_{F}$ | $\mathrm{DM}^{\mathrm{d})}$ | $\mathrm{F} 1, \mathrm{Rk}$ | F2,Rk | F3,Rk | F4,Rk |
|  |  | [mm] |  | [mm] | der | [mm] | connector | [mm] | [mm] | [mm] | [mm] | [mm] | [mm] | [mm] | [mm] | [kN] | [kN] | [kN] | [kN] |
| 944874 | Magnus $\times 530 \times 30$ | $30 \times 30 \times 9$ | 20 | $4,0 \times 30$ | 6 | 4,2×26 | 1 | 40 | 40 | 40 | 40 | 40 | 40 | 30 | 9 | 1,2 | 1,57 | 1,70 | 1,19 |
| 944875 | Magnus $550 \times 60$ | $50 \times 60 \times 13$ | 10 | $4,0 \times 60$ | 8 | $4,2 \times 26$ | 2 | 60 | 80 | 60 | 80 | 80 | 80 | 50 | 13 | 3,73 | 7,25 | 5,00 | 1,92 |
| 944876 | Magnus $550 \times 80$ | $50 \times 80 \times 13$ | 10 | $4,0 \times 60$ | 12 | $4,2 \times 26$ | 2 | 60 | 100 | 60 | 100 | 80 | 100 | 50 | 13 | 3,73 | 14,50 | 5,00 | 2,80 |
| 944877 | Magnus $550 \times 100$ | $50 \times 100 \times 13$ | 10 | $4,0 \times 60$ | 18 | $4,2 \times 26$ | 2 | 60 | 120 | 60 | 120 | 80 | 120 | 50 | 13 | 7,46 | 21,75 | 5,00 | 4,41 |
| 944878 | Magnus M $70 \times 120$ | $70 \times 120 \times 17$ | 10 | 5,0×80 | 13 | $4,8 \times 60$ | 2 | 80 | 140 | 80 | 140 | 100 | 140 | 70 | 17 | 5,49 | 21,34 | 13,00 | 5,17 |
| 944879 | Magnus M $70 \times 140$ | $70 \times 140 \times 17$ | 10 | $5,0 \times 80$ | 16 | $4,8 \times 60$ | 2 | 80 | 160 | 80 | 160 | 100 | 160 | 70 | 17 | 5,49 | 32,00 | 13,00 | 6,09 |
| 944880 | Magnus M70 160 | $70 \times 160 \times 17$ | 10 | 5,0×80 | 21 | $4,8 \times 60$ | 2 | 80 | 180 | 80 | 180 | 100 | 180 | 70 | 17 | 10,98 | 37,34 | 13,00 | 8,27 |
| 944881 | Magnus M $70 \times 180$ | $70 \times 180 \times 17$ | 10 | 5,0 $\times 80$ | 24 | $4,8 \times 60$ | 2 | 80 | 200 | 80 | 200 | 100 | 200 | 70 | 17 | 10,98 | 42,67 | 13,00 | 9,32 |
| 944882 | Magnus $1110 \times 220$ | $110 \times 220 \times 19$ | 4 | 8,0×120 | 13 | $4,8 \times 60$ | 2 | 120 | 240 | 120 | 240 | 140 | 240 | 110 | 19 | 9,29 | 36,10 | 23,00 | 13,96 |
| 944883 | Magnus L110 $\times 260$ | $110 \times 260 \times 19$ | 4 | 8,0×120 | 17 | $4,8 \times 60$ | 2 | 120 | 280 | 120 | 280 | 140 | 280 | 110 | 19 | 13,93 | 45,13 | 23,00 | 17,98 |
| 944884 | MagnusL110x 300 | $110 \times 300 \times 19$ | 4 | $8,0 \times 120$ | 20 | $4,8 \times 60$ | 2 | 120 | 320 | 120 | 320 | 140 | 320 | 110 | 19 | 13,93 | 54,15 | 23,00 | 20,56 |
| 944887 | MagnusL110x 340 | $110 \times 340 \times 19$ | 4 | 8,0 $\times 120$ | 22 | $4,8 \times 60$ | 2 | 120 | 360 | 120 | 360 | 140 | 360 | 110 | 19 | 13,93 | 63,18 | 23,00 | 24,67 |
| 944888 | MagnusL110x 380 | $110 \times 380 \times 19$ | 4 | $8,0 \times 120$ | 25 | $4,8 \times 60$ | 2 | 120 | 400 | 120 | 400 | 140 | 400 | 110 | 19 | 9,29 | 72,20 | 23,00 | 26,96 |
| 944889 | Magnus $1110 \times 580$ | $110 \times 580 \times 19$ | 4 | $8,0 \times 120$ | 38 | $4,8 \times 60$ | 2 | 120 | 600 | 120 | 600 | 140 | 600 | 110 | 19 | 9,29 | 126,35 | 23,00 | 43,29 |

* 1 connector consists of 2 individual parts
a) $\mathrm{I}=$ assembly thickness
b) Incuded in delivery
c) Recommended minimum width of the secondary beam with the connector flush-mounted
d) To make installation eassier, it is advantageous to reduce the milling depth slightly, especially for larger wood dimensions.
e) Both heams softwood with a gross density of $\mathrm{p}=380 \mathrm{~kg} / \mathrm{m}^{3}$.

The specified characterisicic values of the load-bearing capacity Frk apply to the specified timber cross-sections, centred force application along the respective beam axis as well as connector installation flush with the top edge of the main and secondary beams.
Calculation according to EAA $15 / 0761$. All mechanical values provided should be viewed as subject to the assumptions that have been made and represent example calculations.
All values are calculated minimum values and are subject to typographical and printing errors.
The characcerisici values of the load-bearing capacity $F_{\text {Rks }}$ should not be treated as equivalent to the max. possible load (the max. force). The characteristic values of the load-bearing capacity $\mathrm{F}_{\text {Rk }}$ should be reduced to the design values $\mathrm{F}_{\text {Rd }}$ in terms of the service class and the lood duration class: $F_{R d}=F_{R k} \times k$ mod $/ \gamma M$.
Please note: These are planning cids. Projects must only be calculated by authorised persons.
Please note: Verify the assumptions made. The stated values, and type and number of joining devices are based on preliminary measurements. Projects are to be dimensioned exclusively by authorised persons in accordance with the State Building Code. As per LBauO, please contact a qualified structural engineer for a paid proof of stability. We will be happy to refer you to someone.

## INSTALLATION ACCESSORIES

Milling and assembly iig For Magnus hook connector


| Art. no. | Suitable for | PU |
| :--- | :--- | ---: |
| 944867 | Magnus XS | 1 |
| 944894 | Magnus S | 1 |
| 944895 | Magnus M | 1 |
| 944870 | Magnus L 220/260/300 | 1 |
| 944903 | Magnus L340/380/420 | 1 |
| 944904 | Magnus L 460/500/540/580 | 1 |

DESCRIPTION

- Insertion aid for surface-mounted installation
- Milling jig for flush-mounted installation

| Art. no. | Suitable for | Shaft diameter [mm] | PU |
| :--- | :--- | :--- | ---: |
| 944936 | Magnus XS | 6,35 | 1 |
| 29686 | Magnus S | 8 | 1 |
| 29696 | Magnus M und L | 8 | 1 |

THE FOLLOWING MUST BE OBSERVED IN THE EVENT OF FLUSH-MOUNTED INSTALLATION IN THE SECONDARY BEAM

- The beam's minimum width must be increased so that there is enough surrounding wood remaining at the side for the milling work
- The beam must be milled out at full height

THE FOLLOWING MUST BE OBSERVED IN THE EVENT OF FLUSH-MOUNTED INSTALLATION IN THE MAIN BEAM

- The main beam's load-bearing cross-section is reduced by the connector's assembly thickness
- The beam's minimum width must be adjusted (screw length)



| Art. no. | Name | Dimensions | PU* | Fully threaded screws ${ }^{\text {b }}$ ) |  |  |  |  |  | Fixing screws ${ }^{\text {b }}$ ) |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
|  |  | WxHx $\mathrm{D}^{\text {a }}$ |  | Dimensions | ntotal | In the main beam |  | In the secondary beam |  | Dimensions | n |
|  |  | [mm] |  | [mm] |  | n90 ${ }^{\circ}$ | 144 ${ }^{\circ}$ | n90 ${ }^{\circ}$ | n45 ${ }^{\circ}$ | [mm] |  |
| 94887 | Magnus XS $30 \times 30$ | $30 \times 30 \times 9$ | 20 | 4,0x 30 | 6 | 3 | - | 3 | - | $4,2 \times 26$ | 1 |

${ }^{*} 1$ connector consists of 2 individual parts
a) $D=$ assembly thickness
b) Incuded in delivery

| Art. no. | Name | Dimensions | Main beam |  | Secondary beam sufface-mounted |  | Secondary beam flush-mounted |  |  |  | characterisicic lood-bearing (cppacity FRk ${ }^{\text {d }}$ |  |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
|  |  | WxHx ${ }^{\text {dol }}$ |  |  | min. WSB | min. Hs | min. WS ${ }_{\text {b }}{ }^{\text {b }}$ | min. Hsb | WM | DM ${ }^{\text {c }}$ | $\mathrm{F}_{1, \mathrm{Rk}}$ | F2,Rk | ,Rk | F4,Rk |
|  |  | [mm] | [mm] | [mm] | [mm] | [mm] | [mm] | [mm] |  | [mm] | [kN] | [kN] | [kN] | [kN] |
| 14874 | Magus $\times 530 \mathrm{x}$ | 0x30x9 | 40 | 40 | 40 | 40 | 40 | 40 | 30 | 9 | 1,12 | 1,57 | 1,0 | 1,19 |

a) $D=$ assembly thickness
b) Incuded in delivery
c) Recommended minimum width of the secondary beam with the connector flush-mounted
d) To make installation eassier, it is advantageous to reduce the milling depth slightly, especially for larger wood dimensions.
e) Both beams softwood with a gross density of $\mathrm{pk}=380 \mathrm{~kg} / \mathrm{m}^{3}$.

The specified characterisicic values of the load-bearing capacity FRk apply to the specified timber cross-sections, centred force application along the respective beam axis as well as connector installation flush with the top edge of the main and secondary beams.
Calculation according to ETA $15 / 0761$. All mechanical values provided should be viewed as subject to the assumptions that have been made and represent example calculctions.
All values are calculated minimum valves and are subject to typographical and printing errors.
The characterisicic values of the load-bearing capacity FRk should not be treated as equivalent to the max. possible load (the max. force). The charocterisicic values of the lood-bearing capacity FRk should be redveed to the design values FRd in terms of the service class and the load duration class: FRd $=$ FRk x mod $/ \gamma \mathrm{M}$.
Please note: These are planning aids. Projects must only be calculated by authorised persons.

## MAGNUS S $50 \times 60$



| Art. no. | Name | Dimensions | PU* | Fully threaded screws ${ }^{\text {b }}$ ) |  |  |  |  |  | Fixing screws ${ }^{\text {b }}$ |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
|  |  | WxHx ${ }^{\text {a }}$ |  | Dimensions | ntotal | In the main beam |  | In the secondary beam |  | Dimensions | n |
|  |  | [mm] |  | [mm] |  | n90 ${ }^{\circ}$ | n45 ${ }^{\circ}$ | n90 ${ }^{\circ}$ | n45 ${ }^{\circ}$ | [mm] |  |
| 944875 | Magnus $50 \times 60$ | $50 \times 60 \times 13$ | 10 | 4,0×60 | 8 | 2 | 2 | 2 | 2 | $4,2 \times 26$ | 2 |

${ }^{*} 1$ connector consists of 2 individual parts
a) $D=$ assembly thickness
b) Incuded in delivery

| Art. no. | Name | Dimensions | Moin beam |  | Secondary beam surface-mounted |  | Secondory beum flush-mounted |  |  |  | characterisisic lood -bearing (cpacity $F_{\text {Rk }}(1)$ |  |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
|  |  | WxHx ${ }^{\text {a }}$ |  |  | min. WSB | min. HSB | min. W( S $^{\text {b }}$ | min. HSB | WM | DM ${ }^{\text {c }}$ | Fl,Rk | F2,Rk | $\mathrm{F}_{3, \mathrm{R} k}$ | F4,Rk |
|  |  | [mm] | [mm] | [mm] | [mm] | [mm] | [mm] | [mm] | [mm] | [mm] | [kN] | [kN] | [kN] | [kN] |
| 44875 | Mogus | $50 \times 60 \times 13$ | 60 | 80 | 60 | 80 | 80 | 80 | 50 | 13 | 3,73 | 7,25 | 5,00 | 1,92 |

a) $D=$ assembly thickness
b) Incuded in delivery
c) Recommended minimum width of the secondary beam with the connector flush-mounted
d) To make installation easier, it is advantageous to reduce the milling depth slightly, especially for larger wood dimensions.
e) Both beams softwood with a gross density of $\mathrm{pk}=380 \mathrm{~kg} / \mathrm{m}^{3}$.

The specified characterisicic values of the load-bearing capacity FRk apply to the specified timber cross-sections, centred force application along the respective beam axis as well as connector installation flush with the top edge of the main and secondary beams.
Calculation according to ETA 15/0761. All mechanical values provided should be viewed as subject to the assumptions that have been made and represent example calculations.
All values are calculated minimum values and are subject to typographical and printing errors.
The characteristic values of the load-bearing capacity FRk should not be treated as equivalent to the max. possible lood (the max. force). The characterisic values of the load-bearing capacity FRk should be reduced to the design valves Frd in terms of the service class and the lood duration class: FRd= FRk x mod $/ \gamma \mathrm{M}$.
Please note: These are planning ciids. Projects must only be calculated by authorised persons.

${ }^{*} 1$ connector consists of 2 individual parts
a) $D=$ assembly thickness
b) Incuded in delivery

| Art. no. | Name | Dimensions |  |  | Secondary beam sufface-mounted |  | Secondary beam flush-mounted |  |  | characterisicil lood-bearing (capacity Frk ${ }^{\text {d }}$ |  |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
|  |  | WxHx ${ }^{\text {o }}$ ) | min. WMB | min. HMB | min. WSB | min. HSB | min. Ws ${ }^{\text {b }}$ ) | min. HSB | WM $\mathrm{D}_{\text {M }}{ }^{\text {d }}$ | F/,Rk | F2,Rk | $\mathrm{F}_{3, \mathrm{Rk}}$ | F4, Rk |
|  |  | [mm] | [mm] | [mm] | [mm] | [mm] | [mm] | [mm] | [mm] [mm] | [kN] | [kN] | [kN] | [kN] |
| 94876 | Hagus $550 \times 80$ | $50 \times 80 \times 13$ | 60 | 100 | 60 | 100 | 80 | 100 | $50 \quad 13$ | 3,73 | 14,50 | 5,00 | 280 |

## a) $D=$ assembly thickness

## b) Incuded in delivery

c) Recommended minimum width of the secondary beam with the connector flush-mounted
d) To make installation easier, it is advantageous to reduce the milling depth slightly, especially for larger wood dimensions.
e) Both beams softwood with a gross density of $\mathrm{pk}=380 \mathrm{~kg} / \mathrm{m}^{3}$.

The specified characterisic values of the load-bearing capacity FRk apply to the specified fimber cross-sections, centred force application along the respective heam axis as well as connector installation flush with the top edge of the main and secondary beams.
Calculation according to EAA $15 / 0761$. All mechanical values provided should be viewed as subject to the assumptions that have been made and represent example calculations.
All values are calculated minimum values and are subject to typoographical and printing errors.
The characteristic values of the load-bearing capacity FRk should not be treated as equivalent to the max. possible load (the max. force). The characteristic values of the load-bearing capacity FRk should be reduced to the design values Frd in terms of the service class and the load duration class: FRd= FRk x $\mathrm{kmod}^{\mathrm{m}} / \gamma \mathrm{M}$.
Please note: These are planning aids. Projects must only be calculated by authorised persons.


| Art. no. | Name | Dimensions | PU* | Fully threaded screws ${ }^{\text {b }}$ |  |  |  |  |  | Fixing screws ${ }^{\text {b }}$ ) |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
|  |  | WxHx $\mathrm{D}^{\text {a }}$ |  | Dimensions | ntotal | In the main beam |  | In the secondary beam |  | Dimensions | n |
|  |  | [mm] |  | [mm] |  | n90 ${ }^{\circ}$ | n45 ${ }^{\circ}$ | n90 ${ }^{\circ}$ | n45 ${ }^{\circ}$ | [mm] |  |
| 94887 | Magnus $50 \times 100$ | $50 \times 100 \times 13$ | 10 | 4,0×60 | 18 | 2 | 6 | 4 | 6 | $4,2 \times 26$ | 2 |

* 1 connector consists of 2 individual parts
a) $D=$ assembly thickness
b) Incuded in delivery

| Art. no. | Name | Dimensions | Main heam |  | Secondary beam surface-mounted |  | Secondory beam flush-mounted |  |  |  | characterisitic lood-bearing capacity FRk ${ }^{\text {d }}$ |  |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
|  |  | WxHx ${ }^{0}$ ) |  |  | min. Wsb | min. H SB | min. WSs ${ }^{\text {b/ }}$ | min. HSB | WM | DM ${ }^{\text {c }}$ | Fl,Rk | F2,Rk | $\mathrm{F}_{3,1 \mathrm{Rk}}$ | F4,Rk |
|  |  | [mm] | [mm] | [mm] | [mm] | [mm] | [mm] | [mm] |  | [mm] | [kN] | [kN] | [kN] | [kN] |
| 94887 | S50x 10 | $\times 100 \times$ | 60 | 120 | 60 | 120 | 80 | 120 | 50 | 13 | 7.46 | 21,75 | 500 | 441 |

a) $D=$ assembly thickness
b) Incuded in delivery
c) Recommended minimum width of the secondary beam with the connector flush-mounted
d) To make installation easier, it is advantageous to reduce the milling depth slightly, especially for larger wood dimensions.
e) Both beams softwood with a gross density of $\mathrm{pk}=380 \mathrm{~kg} / \mathrm{m}^{3}$.

The specified characterisic values of the load-bearing capacity FRk apply to the specified timber cross-sections, centred force application along the respective beam axis as well as connector installation flush with the top edge of the main and secondary beams.
Calculation according to ETA 15/0761. All mechanical values provided should be viewed as subject to the assumptions that have been made and represent example calculations.
All values are calculated minimum values and are subject to typographical and printing errors.
The characterisicic values of the load-bearing capacity Frk should not be trected as equivalent to the max. possible lood (the max. force). The characterisicic values of the load-bearing capacity FRk should be recuved to the design values Frd in terms of the service class and the load duration class: $\mathrm{FRd}=\mathrm{FRk} \mathrm{x}$ kmod $/ \gamma \mathrm{M}$.
Please note: These are planning ciids. Projects must only be calculated by outhorised persons.


Symbolic illustrations: f.l.t.r. Main beam, secondary beam surface-mounted, secondary beam flush-mounted, connector dimensions


| Art. no. | Name | Dimensions | PU* | Fully threaded screws ${ }^{\text {b }}$ ) |  |  |  |  |  | Fixing screws ${ }^{\text {b }}$ ) |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
|  |  | WxHx ${ }^{\text {a) }}$ |  | Dimensions | ntotal | In the main beam |  | In the secondary beam |  | Dimensions | n |
|  |  | [mm] |  | [mm] |  | n90 ${ }^{\circ}$ | n45 ${ }^{\circ}$ | n90 ${ }^{\circ}$ | 145 ${ }^{\circ}$ | [mm] |  |
| 944878 | Magus M $70 \times 120$ | $70 \times 120 \times 17$ | 10 | $5,0 \times 80$ | 13 | 2 | 4 | 2 | 5 | 4,8×60 | 2 |

${ }^{*} 1$ connector consists of 2 individual parts
a) $D=$ assembly thickness
b) Incuded in delivery

| Art. no. | Name | Dimensions |  |  | Secondar | suface- | Secondary beam flush-mounted |  |  |  | characterisisic lood bearing (cpacity F Fkk ${ }^{\text {d }}$ |  |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
|  |  | WxHx00) | min. WMB min. HMB |  | min. Wsb | min. HSB | min. W(Sb ${ }^{\text {b }}$ | min. HSB | Wm | DM ${ }^{\text {c }}$ | Fl,Rk | F2,Rk | Rk | F4,Rk |
|  |  | [mm] | [mm] | [mm] | [mm] | [mm] | [mm] | [mm] | [mm] | [mm] | [kN] | [kN] | [kN] | [kN] |
| 44878 | M70 120 | $70 \times 120 \times 17$ | 80 | 140 | 80 | 140 | 100 | 140 | 70 | 17 | 5,49 | 21,34 | 13,0 | 5.17 |

## a) $D=$ assembly thickness

## b) Incuded in delivery

c) Recommended minimum width of the secondary beam with the connector flush-mounted
d) To make installation easier, it is advantageous to reduce the milling depth slightly, especially for larger wood dimensions.
e) Both beams softwood with a gross density of $\mathrm{pk}=380 \mathrm{~kg} / \mathrm{m}^{3}$.

The specified characterisic values of the load-bearing capacity FRk apply to the specified fimber cross-sections, centred force application along the respective heam axis as well as connector installation flush with the top edge of the main and secondary beams.
Calculation according to EAA $15 / 0761$. All mechanical values provided should be viewed as subject to the assumptions that have been made and represent example calculations.
All values are calculated minimum values and are subject to typographical and printing errors.
The characteristic values of the load-bearing capacity Frk should not be treated as equivalent to the max. possible load (the max. force). The characterisicic values of the lood-bearing capacity FRk should be reduced to the design values Frd in terms of the service class and the load duration class: FRd= FRk x $\mathrm{kmod}^{\mathrm{m}} / \gamma \mathrm{M}$.
Please note: These are planning aids. Projects must only be calculated by authorised persons.


| Art. no. | Name | Dimensions | PU* | Fully threaded screws ${ }^{\text {b }}$ ) |  |  |  |  |  | Fixing screws ${ }^{\text {b }}$ ) |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
|  |  | WxHx D ${ }^{\text {a) }}$ |  | Dimensions | ntotal | In the main beam |  | In the secondary beam |  | Dimensions | n |
|  |  | [mm] |  | [mm] |  | n90 ${ }^{\circ}$ | n45 ${ }^{\circ}$ | n90 ${ }^{\circ}$ | n45 ${ }^{\circ}$ | [mm] |  |
| 948879 | Magnu M $70 \times 140$ | $70 \times 140 \times 17$ | 10 | 5,0×80 | 16 | 2 | 6 | 2 | 6 | 4,8×60 | 2 |

* 1 connector consists of 2 individual parts
a) $D=$ assembly thickness
b) Incuded in delivery

| Art. no. | Name | $\begin{aligned} & \text { Dimensions } \\ & \text { WxH } \times \text { Dol }^{0} \end{aligned}$ | Moin beam |  | Secondary beam sufface-mounted |  | Secondary beam flush-mounted |  |  |  | characterisicic load-bearing capacity FRk) |  |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
|  |  |  | min. WMB | min. HMB $^{\text {S }}$ | min. Wsb | min. HSB | min. Wsb ${ }^{\text {b }}$ | min. HSB | WM | DM ${ }^{\text {c }}$ | F1,Rk | F2,Rk | $\mathrm{F}_{3, \mathrm{Rk}}$ | F4,Rk |
|  |  | [mm] | [mm] | [mm] | [mm] | [mm] | [mm] | [mm] | [mm] |  | [kN] | [kN] | [kN] | [kN] |
| 94879 | nus $170 \times 140$ | 10x10x 17 | 80 | 160 | 80 | 160 | 100 | 160 | 70 |  | 5,49 | 32,0 | 13,00 | 69 |

a) $D=$ assembly thickness
b) Incuded in delivery
c) Recommended minimum width of the secondary beam with the connector flush-mounted
d) To make installation easier, it is advantageous to reduce the milling depth slightly, especially for larger wood dimensions.
e) Both beams softwood with a gross density of $\mathrm{pk}=380 \mathrm{~kg} / \mathrm{m}^{3}$.

The specified characterisicic values of the load-bearing capacity FRk apply to the specified fimber cross-sections, centred force application along the respective beam axis as well as connector installation flush with the top edge of the main and secondary beams.
Calculation according to ETA $15 / 0761$. All mechanical values provided should be viewed as subject to the assumptions that have been made and represent example calculations.
All values are calculated minimum values and are subject to typographical and printing errors.
The characteristic values of the load-bearing capacity Frk should not be trected as equivalent to the max. possible load (the max. force). The characterisicic values of the lood-bearing capacity FRk should be reduced to the design values Frd in terms of the service class and the load duration class: $\mathrm{FRd}=\mathrm{FRk} \mathrm{x}$ kmod $/ \gamma \mathrm{M}$.
Please note: These are planning ciids. Projects must only be calculated by outhorised persons.


| Art. no. | Name | Dimensions | PU* | Fully threaded screws ${ }^{\text {b }}$ ) |  |  |  |  |  | Fixing screws ${ }^{\text {b }}$ ) |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
|  |  | WxHx D ${ }^{\text {a) }}$ |  | Dimensions | ntotal | In the main beam |  | In the secondary beam |  | Dimensions | n |
|  |  | [mm] |  | [mm] |  | n90 ${ }^{\circ}$ | 1445 | n90 ${ }^{\circ}$ | 1445 | [mm] |  |
| 948880 | Mognu M $70 \times 160$ | 70×160 17 | 10 | 5,0×80 | 21 | 2 | 8 | 4 | 7 | 4,8×60 | 2 |

${ }^{*} 1$ connector consists of 2 individual parts
a) $D=$ assembly thickness
b) Incuded in delivery

| Art. no. | Name | Dimensions | Moin beammin. WMB min. HMB |  | Secondary beam suffac--mounted |  | Secondary beam flush-mounted |  |  |  | characterisisil load-bearing (capaity FRx d) |  |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
|  |  | WxHx ${ }^{\text {a }}$ |  |  | min. WSB | min. HSb | min. WS ${ }^{\text {b }}$ b | min. HSB | WM | DM ${ }^{\text {c }}$ | $\mathrm{Fl}_{1, \mathrm{kk}}$ | F2,Rk | $\mathrm{F}_{3, \mathrm{Rk}}$ | F4,Rk |
|  |  | [mm] | [mm] | [mm] | [mm] | [mm] | [mm] | [mm] |  | [mm] | [kN] | [kN] | [kN] | [kN] |
| 14880 | mus $170 \times 160$ | $70 \times 160 \times 17$ | 80 | 180 | 80 | 180 | 100 | 180 | 70 | 17 | 10,98 | 37,34 | 13,00 | 8,7 |

## a) $D=$ assembly thicknes

b) Incuded in delivery
c) Recommended minimum width of the secondary beam with the connector flush-mounted
d) To make installation easier, it is advantageous to reduce the milling depth slightly, especially for larger wood dimensions.
e) Both beams softwood with a gross density of $\rho \mathrm{k}=380 \mathrm{~kg} / \mathrm{m}^{3}$.

The specified characterisicic values of the load-bearing capacity Fpk apply to the specified fimber cross-sections, centred force application along the respective beam axis as well as connector installation flush with the top edge of the main and secondary beams.
Calculation according to ETA $15 / 0761$. All mechanical values provided should be viewed as subject to the assumptions that have been made and represent example calculations.
All values are calculated minimum values and are subject to typographical and printing errors.
The characteristic values of the load-bearing capacity FFks should not be treated as equivalent to the max. possible load (the max. force). The characterisic values of the lood-bearing capacity Frks should be reduced to the design values Frd in terms of the
service class and the load duration class: Frd $=$ FRk $\times$ kmod $/ \gamma \mathrm{m}$.
Please note: These are planning aids. Projects must only be calculated by outhorised persons.

## MAGNUS M $70 \times 180$



| Art. no. | Name | Dimensions | PU* | Fully threaded screws ${ }^{\text {b }}$ ) |  |  |  |  |  | Fixing screws ${ }^{\text {b }}$ ) |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
|  |  | W xHx Da) |  | Dimensions | $n_{\text {notal }}$ | In the main beam |  | In the secondary beam |  | Dimensions | n |
|  |  | [mm] |  | [mm] |  | n90 ${ }^{\circ}$ | n45 ${ }^{\circ}$ | n90 ${ }^{\circ}$ | 1445 | [mm] |  |
| 94881 | Magus M $70 \times 180$ | $70 \times 180 \times 17$ | 10 | 5,0×80 | 24 | , | 10 | 4 | 8 | 4,8×60 | 2 |

${ }^{*} 1$ connector consists of 2 individual parts
a) $D=$ assembly thickness
b) Incuded in delivery

| Art. no. | Name | Dimensions | Main beam |  | Secondary beam suffac--mounted |  | Secondary beam flush-mounted |  |  |  | characterisitic lood-bearing (capacity FRk |  |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
|  |  | WxHx ${ }^{0}$ ( |  |  | min. WSB | min. HSB | min. WSs ${ }^{\text {b/ }}$ | min. HSB | WM | DM ${ }^{\text {c }}$ | F1,Rk | F2,Rk | $\mathrm{F}_{3, \mathrm{Rk}}$ | F4,Rk |
|  |  | [mm] | [mm] | [mm] | [mm] | [mm] | [mm] | [mm] | [mm] | [mm] | [kN] | [kN] | [kN] | [kN] |
| 44881 | Mgnus M70x | $70 \times 180 \times 17$ | 80 | 200 | 80 | 200 | 100 | 200 | 70 | 17 | 10,98 | 42,67 | 13,00 | ${ }^{32}$ |

a) $D=$ assembly thickness
b) Incuded in delivery
c) Recommended minimum width of the secondary beam with the connector flush-mounted
d) To make installation eassier, it is advantageous to reduce the milling depth slightly, especially for larger wood dimensions.
e) Both beams softwood with a gross density of $\mathrm{pk}=380 \mathrm{~kg} / \mathrm{m}^{3}$.

The specified characteristic valves of the load-bearing capacity FRk apply to the specified timber cross-sections, centred force application along the respective beam axis as well as connector installation flush with the top edge of the main and secondary beams.
Calculation according to ETA 15/0761. All mechanical values provided should be viewed as subject to the assumptions that have been made and represent example calculations.
All values are calculated minimum values and are subject to typographical and printing errors.
The characterisici values of the load-bearing capacity FRk should not be treated as equivalent to the max. possible load (the max. force). The characterisic values of the load bearing capacity FRk should be reduced to the design valves Frd in terms of the service class and the load duration dass: Frd= Frk x kmod / $\gamma \mathrm{M}$.
Please note: These are planning cids. Projects must only be calculated by authorised persons.


| Art. no. | Name | Dimensions | PU* | Fully threaded screws ${ }^{\text {b }}$ ) |  |  |  |  |  | Fixing screws ${ }^{\text {b }}$ ) |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
|  |  | WxHx ${ }^{\text {a) }}$ |  | Dimensions | ntotal | In the main beam |  | In the secondary beam |  | Dimensions | n |
|  |  | [mm] |  | [mm] |  | n90 ${ }^{\circ}$ | n45 ${ }^{\circ}$ | n90 ${ }^{\circ}$ | 1445 | [mm] |  |
| 948882 | Magnus $1110 \times 220$ | $110 \times 220 \times 19$ | 4 | $8,0 \times 120$ | 13 | 2 | 4 | 2 | 5 | 4,8×60 | 2 |

${ }^{*} 1$ connector consists of 2 individual parts
a) $D=$ assembly thickness
b) Incudeded in delivery

| Art. no. | Name | Dimensions | Moin beam |  | Secondory beam sufface-mounted |  | Secondary beam flush-mounted |  |  |  | characterisicil load-bearing (capacity FRk ${ }^{\text {d }}$ ) |  |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
|  |  | WxHx ${ }^{0}$ ( |  |  | min. WSB | min. Hsb | min. W( S $^{\text {b }}$ ) | min. HSB | WM | DM ${ }^{\text {c }}$ | F1,Rk | F2,Rk | $\mathrm{F}_{3,1 \mathrm{Rk}}$ | F4,Rk |
|  |  | [mm] | [mm] | [mm] | [mm] | [mm] | [mm] | [mm] |  | [mm] | [kN] | [kN] | [kN] | [kN] |
| 4482 | L110x20 | $10 \times 220 \times 1$ | 120 | 240 | 120 | 240 | 140 | 240 | 110 | 19 | 9,29 | 36,10 | 23,00 | 13,96 |

a) $D=$ assembly thickness
b) Incuded in delivery
c) Recommended minimum width of the secondary beam with the connector flush-mounted
d) To make installation easier, it is advantageous to reduce the milling depth slightly, especially for larger wood dimensions.
e) Both beams softwood with a gross density of $\mathrm{pk}=380 \mathrm{~kg} / \mathrm{m}^{3}$.

The specified characterisicic values of the load-bearing capacity FRk apply to the specified fimber cross-sections, centred force application along the respective beam axis as well as connector installation flush with the top edge of the main and secondary beams.
Calculation according to ETA $15 / 0761$. All mechanical values provided should be viewed as subject to the assumptions that have been made and represent example calculations.
All values are calculated minimum values and are subject to typographical and printing errors.
The characteristic values of the load-bearing capacity FFks should not be treated as equivalent to the max. possible load (the max. force). The characterisic values of the lood-bearing capacity. FRk should be reduced to the design values Fed in terms of the
service class and the lood duration class: Frd= FRk x kmod $/ \gamma \mathrm{M}$.
The characterisicic load-bearing capacities for the L series were determined vsing $8 \times 120$ VG screws. Higher capacities can be achieved with longer screws (however, the minimum cross-sections of the supports also change)
Please note: These are planning aids. Projects must only be calculated by authorised persons.


| Art. no. | Name | Dimensions | PU* | Fully threaded screws ${ }^{\text {b }}$ ) |  |  |  |  |  | Fixing screws ${ }^{\text {b }}$ ) |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
|  |  | WxHx D ${ }^{\text {a }}$ |  | Dimensions | ntotal | In the main beam |  | In the secondary beam |  | Dimensions | n |
|  |  | [mm] |  | [mm] |  | n90 ${ }^{\circ}$ | n45 ${ }^{\circ}$ | n90 ${ }^{\circ}$ | 1445 | [mm] |  |
| 94888 | Magnus $1110 \times 260$ | $110 \times 260 \times 19$ | 4 | 8,0 $\times 120$ | 17 | $\bigcirc$ | 5 | J | 6 | $4,8 \times 60$ | 2 |

* 1 connector consists of 2 individual parts
a) $D=$ assembly thickness
b) Included in delivery

| Art. no. | Name | Dimensions | Main beammin. WMB min. HMB |  | Secondary beam sufface-mounted |  | Secondary heum flush-mounted |  |  |  | choracterisisic lood-bearing capacity Frk () |  |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
|  |  | WxHx ${ }^{\text {a }}$ |  |  | min. WSB | min. H SB | min. W(Sb) | min. HSB | WM | $\mathrm{DM}^{\text {c }}$ | $\mathrm{F}_{1, \mathrm{Rk}}$ | F2,Rk | $\mathrm{F}_{3, \mathrm{Rk}}$ | F4,k |
|  |  | [mm] | [mm] | [mm] | [mm] | [mm] | [mm] | [mm] | [mm] | [mm] | [kN] | [kN] | [kN] | [kN] |
| 4483 | IIgus $110 \times 200$ | $110 \times 260 \times 19$ | 120 | 280 | 120 | 280 | 140 | 280 | 110 | 19 | 13,93 | 45,13 | 23,00 | 17,98 |

a) $D=$ assembly thickness
b) Incuded in delivery
c) Recommended minimum width of the secondary beam with the connector flush-mounted
d) To make installation eassier, it is advantageous to reduce the milling depth slightly, especially for larger wood dimensions.
e) Both beams softwood with a gross density of $\mathrm{pk}=380 \mathrm{~kg} / \mathrm{m}^{3}$.

The specified characteristic values of the load-bearing capacity Fpk apply to the specified timber cross-sections, centred force application along the respective beam axis as well as connector installation flush with the top edge of the main and secondary beams.
Calculation according to ETA 15/0761. All mechanical values provided should be viewed as subject to the assumptions that have been made and represent example calculations.
All values are calculated minimum values and are subject to typographical and printing errors.
The characterisicic valves of the load-bearing capacity Frk should not be treated as equivalent to the max. possible load (the max. force). The characterisic values of the load-bearing capacity. FRk should be reduced to the design values Frd in terms of the service class and the lood duration class: $\mathrm{Frd}=\mathrm{FRk} \mathrm{x} \mathrm{kmod} / \gamma \mathrm{M}$.
The characterisic load-bearing capacities for the 1 series were determined using $8 \times 120$ VG screws. Figher capacities can be achieved with longer screws (however, the minimum cross-sections of the supports also change)
Please note: These are planning aids. Projects must only be calculated by authorised persons.


| Art. no. | Name | Dimensions | PU* | Fully threaded screws ${ }^{\text {b }}$ ) |  |  |  |  |  | Fixing screws ${ }^{\text {b }}$ ) |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
|  |  | W x H x Da) |  | Dimensions | $n_{\text {notal }}$ | In the main beam |  | In the secondary beam |  | Dimensions | n |
|  |  | [mm] |  | [mm] |  | n90 ${ }^{\circ}$ | n45 ${ }^{\circ}$ | n90 ${ }^{\circ}$ | n45 ${ }^{\circ}$ | [mm] |  |
| 94884 | Magnus $1110 \times 300$ | $110 \times 300 \times 19$ | 4 | 8,0 $\times 120$ | 20 | 4 | 6 | 3 | 7 | 4,8×60 | 2 |

* 1 connector consists of 2 individual parts
a) $D=$ assembly thickness
b) Included in delivery

| Art. no. | Name | Dimensions | manin WMB min. HMB |  | Secondory beam suffac-mounted |  | Secondary beam flush-mounted |  |  |  | characterisicil load-bearing (cpacity FRk ${ }^{\text {d }}$ |  |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
|  |  | WxHxD0) |  |  | min. WSB | min. Hsb | min. W(Sb) | min. HSB | WM | DM ${ }^{\text {c }}$ | $\mathrm{F}_{1, \mathrm{Rk}}$ | F2,Rk | $\mathrm{F}_{3,1 \mathrm{k}}$ | F4,Rk |
|  |  | [mm] | [mm] | [mm] | [mm] | [mm] | [mm] | [mm] | [mm] |  | [kN] | [kN] | [kN] | [kN] |
| 44884 | Henus $110 \times 300$ | $110 \times 300 \times 19$ | 120 | 320 | 120 | 320 | 140 | 320 | 110 | 19 | 13,93 | 54,15 | 23,00 | 20,56 |

## a) $D=$ assembly thickness

## b) Included in delivery

c) Recommended minimum width of the secondary beam with the connector flush-mounted
d) To make installation easier, it is advantageous to reduce the milling depth slightly, especially for larger wood dimensions.
e) Both beams softwood with a gross density of $\rho \mathrm{k}=380 \mathrm{~kg} / \mathrm{m}^{3}$.

The specified characteristic values of the load-bearing capacity FRk apply to the specified timber cross-sections, centred force application along the respective beam axis as well as connector installation flush with the top edge of the main and secondary beams.
Calculation according to EAA $15 / 0761$. All mechanical values provided should be viewed as subject to the assumptions that have been made and represent example calculations.
All values are calculated minimum values and are subject to typographical and printing errors.
The characteristic values of the load-bearing capacity FRks should not be treated as equivalent to the max. possible load (the max. force). The characteristic values of the lood-bearing capacity Frks should be reduced to the design values Frd in terms of the
service class and the load duration class: Frd $=$ FRk $\times$ kmod $/ \gamma \mathrm{m}$.
The characterisicic lood-bearing capacities for the L series were determined using $8 \times 120$ VG screws. Higher capacities can be achieved with longer screws (however, the minimum cross-sections of the supports also change)
Please note: These are planning aids. Projects must only be calculated by authorised persons.

## MAGNUS L $110 \times 340$



| Art. no. | Name | Dimensions | PU* | Fully threaded screws ${ }^{\text {b }}$ ) |  |  |  |  |  | Fixing screws ${ }^{\text {b }}$ ) |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
|  |  | WxHx D ${ }^{\text {a) }}$ |  | Dimensions | ntotal | In the main beam |  | In the secondary beam |  | Dimensions | n |
|  |  | [mm] |  | [mm] |  | n90 ${ }^{\circ}$ | 1445 | n90 ${ }^{\circ}$ | 145 ${ }^{\circ}$ | [mm] |  |
| 94887 | Magns L110x 340 | $110 \times 340 \times 19$ | 4 | $8,0 \times 120$ | 22 | 3 | 7 | 3 | 9 | 4,8×60 | 2 |

* 1 connector consists of 2 individual parts
a) $D=$ assembly thickness
b) Incuded in delivery

| Art. no. | Name | Dimensions | Moin heam |  | Secondary beam suffac--mounted |  | Secondary beam flush-mounted |  |  |  | characterisicic load. -bering (apacity Frk ${ }^{\text {d }}$ |  |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
|  |  | Wx |  |  | min. Wsb | min. HSB | min. WS ${ }^{\text {b }}$ ] | min. HSB | WM | DM ${ }^{\text {()I }}$ | $\mathrm{F}_{1, \mathrm{Rk}}$ | F2,Rk | $\mathrm{F}_{3, \mathrm{Rk}}$ | F4,Rk |
|  |  | [mm] | [mm] | [mm] | [mm] | [mm] | [mm] | [mm] | [mm] |  | [kN] | [kN] | [kN] | [kN] |
| 4887 | Mogus $110 \times 330$ | $110 \times 340 \times 19$ | 120 | 360 | 120 | 360 | 140 | 360 | 110 | 19 | 13,93 | 63,18 | 23,00 | 24,67 |

a) $D=$ assembly thickness
b) Incuded in delivery
c) Recommended minimum width of the secondary beam with the connector flush-mounted
d) To make installation easier, itis advantageous to reduce the milling depth slightly, especially for larger wood dimensions.
e) Both heams softwood with a gross density of $p \mathrm{k}=380 \mathrm{~kg} / \mathrm{m}^{3}$.

The specified characterisicic values of the load-bearing capacity Fpk apply to the specified fimber cross-sections, centred force application along the respective beam axis as well as connector installation flush with the top edge of the main and secondary beams.
Calculation according to ETA $15 / 0761$. All mechanical values provided should be viewed as subject to the assumptions hat have been made and represent example calculations.
All values are calculated minimum values and are subject to typographical and printing errors.
The characteristic values of the load-bearing capacity Frk should not be treated as equivalent to the max. possible load (the max. force). The characterisicic values of the load-bearing capacity FRk should be reduced to the design values Frd in terms of the
service class and the load duration class: $\mathrm{Frd}=\mathrm{Frk} \mathrm{x}$ mod $/ \gamma \mathrm{M}$.
The characteristic load-bearing capacities for the L series were determined using $8 \times 120$ VG screws. Higher capacities can be achieved with longer screws (however, the minimum cross-sections of the supports also change)
Please note: These are planning cidds. Projects must only be calculated by authorised persons.


Symbolic illustrations: f.l.t.r. Main beam, secondary beam surface-mounted, secondary beam flush-mounted, connector dimensions


| Art. no. | Name | Dimensions | PU* | Fully threaded screws ${ }^{\text {b }}$ ) |  |  |  |  |  | Fixing screws ${ }^{\text {b }}$ |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
|  |  | Wx $\mathrm{Hx}^{\text {d }}$ ) |  | Dimensions | ntotal | In the main beam |  | In the secondary beam |  | Dimensions | n |
|  |  | [mm] |  | [mm] |  | n90 ${ }^{\circ}$ | 1445 | n90 ${ }^{\circ}$ | n45 ${ }^{\circ}$ | [mm] |  |
| 94888 | Magnus $1110 \times 380$ | $110 \times 380 \times 19$ | 4 | 8,0 $\times 120$ | 25 | 4 | 8 | 2 | 11 | $4,8 \times 60$ | 2 |

* 1 connector consists of 2 individual parts
a) $D=$ assembly thickness
b) Included in delivery

| Art. no. | Name | Dimensions | Main heam |  | Secondary beam surface-mounted |  | Secondary beam flush-mounted |  |  |  | characterisicil lood beering (cppacity FRk d) |  |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
|  |  | WxHx ${ }^{0}$ ) |  |  | min. WSB | min. HSB | min. W( S $^{6}$ ) | min. HSb | WM | DM ${ }^{\text {c }}$ | Fl,Rk | F2,Rk | $\mathrm{F}_{3,1 \mathrm{k}}$ | F4,Rk |
|  |  | [mm] | [mm] | [mm] | [mm] | [mm] | [mm] | [mm] | [mm] | [mm] | [kN] | [kN] | [kN] | [kN] |
| 94888 | 10x | $10 \times 388 \times 19$ | 120 | 400 | 120 | 400 | 140 | 400 | 110 | 19 | 9,29 | 72,20 | 23,00 | 26,96 |

## a) $D=$ assembly thickness

## b) Incudued in delivery

c) Recommended minimum width of the secondary beam with the connector flush-mounted
d) To make installation easier, it is advantageous to reduce the milling depth slightly, especially for larger wood dimensions.
e) Both beams softwood with a gross density of $\rho \mathrm{k}=380 \mathrm{~kg} / \mathrm{m}^{3}$.

The specified characterisic valves of the load-bearing capacity FRk apply to the specified fimber cross-sections, centred force application along the respective beam axis as well as connector installation flush with the top edge of the main and secondary beams.
Calculation according to EAA $15 / 0761$. All mechanical values provided should be viewed as subject to the assumptions that have been made and represent example calculations.
All values are calculated minimum values and are subject to typographical and printing errors.
The characteristic values of the load-bearing capacity FRks should not be treated as equivalent to the max. possible load (the max. force). The characteristic values of the lood-bearing capacity Frks should be reduced to the design values Frd in terms of the
service class and the load duration class: Frd= $=$ Rk x mod $/ \gamma \mathrm{M}$.
The characterisicic lood-bearing capacities for the L series were determined using $8 \times 120$ VG screws. Higher capacities can be achieved with longer screws (however, the minimum cross-sections of the supports also change)
Please note: These are planning aids. Projects must only be calculated by authorised persons.


| Art. no. | Name | Dimensions | PU* | Fully threaded screws ${ }^{\text {b }}$ ) |  |  |  |  |  | Fixing screws ${ }^{\text {b }}$ ) |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
|  |  | Wx $\mathrm{Hx} \mathrm{Da}^{\text {a }}$ |  | Dimensions | $n_{\text {notal }}$ | In the main beam |  | In the secondory beam |  | Dimensions | n |
|  |  | [mm] |  | [mm] |  | n90 ${ }^{\circ}$ | n45 ${ }^{\circ}$ | n90 ${ }^{\circ}$ | n45 ${ }^{\circ}$ | [mm] |  |
| 948889 | y $1110 \times 580$ | $110 \times 580 \times 19$ | 4 | 8,0 $\times 120$ | 38 | 4 | 14 | 2 | 18 | 4,8×60 | 2 |

${ }^{*} 1$ connector consists of 2 individual parts
a) $D=$ assembly thickness
b) Incuded in delivery

| Art. no. | Name | Dimensions | Main beam |  | Secondary beam surface-mounted |  | Secondary beam flush-mounted |  |  |  | characteristic load-bearing capacity Frk ${ }^{\text {d) }}$ |  |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
|  |  | WxHx Da) | min. WMB | min. НMB | min. WSB | min. HSB | min. WS ${ }^{\text {b }}{ }^{\text {b }}$ | min. HSB | WM | DM ${ }^{\text {c) }}$ | F1,Rk | F2,Rk | F3,Rk | F4,Rk |
|  |  | [mm] | [mm] | [mm] | [mm] | [mm] | [mm] | [mm] |  | [mm] | [kN] | [kN] | [kN] | [kN] |
| 94889 | Magnus $1110 \times 580$ | $110 \times 580 \times 19$ | 120 | 600 | 120 | 600 | 140 | 600 | 110 | 19 | 9,29 | 126,35 | 23,00 | 43,29 |

a) $D=$ assembly thickness
b) Incuded in delivery
c) Recommended minimum width of the secondary beam with the connector flush-mounted
d) To make installation eassier, it is advantageous to reduce the milling depth slightly, especially for larger wood dimensions.
e) Both beams softwood with a gross density of $\mathrm{pk}=380 \mathrm{~kg} / \mathrm{m}^{3}$.

The specified characterisicic values of the load-bearing capacity FRk apply to the specified fimber cross-sections, centred force application along the respective beam axis as well as connector installation flush with the top edge of the main and secondary beams.
Calculation according to ETA $15 / 0761$. All mechanical values provided should be viewed as subject to the assumptions that have been made and represent example calculations.
All values are calculated minimum values and are subject to typographical and printing errors.
The characterisici values of the load-bearing capacity FRk should not be treated as equivalent to the max. possible lood (the max. force). The characterisicic values of the lood-bearing capacity FRk should be reduced to the design values Frd in terms of the service class and the load duration class: Frd= Frk x kmod $/ \gamma \mathrm{M}$.
The characteristic load-bearing capacities for the L series were determined using $8 \times 120$ VG screws. Higher capacities can be achieved with longer screws (however, the minimum cross-sections of the supports also change)
Please note: These are planning aids. Projects must only be calculated by authorised persons.

## Eurotec | Wood connector



## EuroTec calculation service

## Magnus Hook Connector according to ETA-15/0761

by phone 02331 6245-444 • by fax 02331 6245-200 • by e-mail technik@eurotec.team

Please contact our technical department or use the free calculation services in the service section of our website.
Contact

Trader:

Contact Person: $\qquad$
email: $\qquad$
Project: $\qquad$

## Project details

## Main Beam

| Width: | $\quad \mathrm{mm}$ |
| :--- | :--- |
| Heigh: | mm |

Strength class: (e.g. C24, GL24h etc.)

## Secondary Beam

Width: $\qquad$ mm

Height: $\qquad$ mm

Strength class: (e.g. C24, GL24h etc.)

## Loads (Characteristic values)

Load duration classPermanentLongMediumShort

## Installation

$\square \quad$ Surface assembly
$\square \quad$ Embedded in secondary beam
$\square \quad$ Embedded in main beam
mm

Contractor:

Contact Person:

Phone: $\qquad$
email:
$\qquad$
$\qquad$
$\qquad$

## T-PROFILE

For hidden aluminium connections


| Art. no. | Dimensions $\left.[\mathrm{mm}]]^{0}\right)$ | Material | Material thickness | PU |
| :--- | :--- | :--- | :--- | ---: |
| 975652 $115 \times 2000 \times 80$ Aluninium | 6 | 1 |  |  |
| a) Height x Lenght $\times$ Width |  |  |  |  |

ADVANTAGES/PROPERTY

- Hole pattern specially for Angle-bracket screw $\varnothing 5,0 \times 50 \mathrm{~mm}$
- Ideal for the timber-concrete connection with the

Rock concrete screw $\varnothing 7,5$

- Creates a hidden connection
- No need of predrilling in combination
with the EST dowel bar


## INSTRUCTIONS FOR USE

The self-drilling EST-Dowel bar $\varnothing 7,5$ can be connected to the $T$-profile without predrilling. The $T$-profile has a hole pattern for the Angle-brackets screw $5,0 \times 5,0 \mathrm{~mm}$. It can also be used together with the Rock concrete screw $\varnothing 7,5$ for the timber-concrete connection. Can be used in service classes 1 and 2 according to DIN EN 1995.

[^1]


Individual cutting of the profile to length


No need of predrilling with the EST dowel bar


Hole pattern with the Angle-bracket screw


Hole pattern with the Rock concrete screw

## EST DOWEL BAR

Double-threaded screw with cylinder head
Eurotec's self-drilling EST dowel bar is a double-threaded screw with an innovative arrow drill and a specifically developed chip-removing groove. Ideally suited for hidden connections in combination with our T-profile. The double-threaded screw has a cylinder head with TX drive. The special geometry of the arrow drill ensures a lower splitting effect when screwing in. The chip-removing groove ensures optimised screwing-in behaviour.

EST dowel bar


| Suitable <br> for <br> T-profile | Art. no. | Dimensions [mm] | Thread length [mm] | Drive | PU |  |
| :---: | :---: | :--- | :--- | :--- | :--- | :--- |
|  | 800291 | $7,5 \times 73$ | $27 / 0$ | TX40 | TX40 | 50 |
|  | 800305 | $7,5 \times 93$ | $27 / 8,5$ | TX40 | 50 |  |
|  | 800306 | $7,5 \times 113$ | $36 / 12,5$ | TX40 | 50 |  |
|  | 800307 | $7,5 \times 133$ | $36 / 12,5$ | TX40 | 50 |  |
|  | 800287 | $7,5 \times 153$ | $36 / 12,5$ | TX40 | 50 |  |
|  | 800288 | $7,5 \times 173$ | $36 / 12,5$ | TX40 | 50 |  |
|  | 800289 | $7,5 \times 193$ | $36 / 12,5$ | TX40 | 50 |  |
|  | 800290 | $7,5 \times 213$ | $36 / 12,5$ | TX40 | 50 |  |

ADVANTAGES / PROPERTIES
Corrosion resistance
Can be used in service classes 1 and 2 according to DIN EN 1991

TECHNICAL DRAWING


स tuluill

Good resistance to mechanical stresses
No pilot-drilling necessary
With innovative arrow drill
No hammering of the screws thanks to TX-drive

Optimum chip-removing groove in the thread
Suitable for timber and aluminum


## DOWEL BAR

The rod dowel is a cylindrical bolt that has a phase at both ends for easier insertion. The rod dowel is suitable for both timber-timber joints and timbersteel joints. It is ideal for combination with our T-profile. The rod dowel is available in different diameters and lengths for an extremely wide range of applications. Please note the product table for this purpose.

| Dowel bar | Suitable | Art. no. | Dimensions [mm] | PU |
| :---: | :---: | :---: | :---: | :---: |
|  |  | 800212 | 12×98 | 50 |
|  |  | 800213 | $12 \times 118$ | 50 |
|  |  | 800214 | $12 \times 138$ | 50 |
|  |  | 800215 | $12 \times 158$ | 50 |
|  |  | 800216 | $12 \times 178$ | 50 |
|  |  | 800217 | $12 \times 198$ | 50 |
|  |  | 800218 | $12 \times 218$ | 50 |
|  |  | 800219 | 12x238 | 50 |
|  |  | 800220 | $12 \times 258$ | 50 |
|  |  | 800221 | $12 \times 278$ | 50 |
|  |  | 800222 | $12 \times 298$ | 50 |
|  |  | 800223 | $16 \times 138$ | 50 |
|  |  | 800224 | $16 \times 158$ | 50 |
|  |  | 800225 | $16 \times 178$ | 50 |
|  |  | 800226 | $16 \times 198$ | 50 |
|  |  | 800227 | $16 \times 218$ | 50 |
|  |  | 800228 | $16 \times 238$ | 50 |
|  |  | 800229 | $16 \times 258$ | 50 |
|  |  | 800230 | $16 \times 278$ | 50 |
|  |  | 800231 | $16 \times 298$ | 50 |
|  |  | 800241 | $16 \times 340$ | 50 |
|  |  | 800243 | $16 \times 480$ | 25 |
| ADVANTAGES |  | 800232 | $16 \times 500$ | 25 |
| Easy to use |  | 800242 | $16 \times 588$ | 25 |
|  |  | ${ }^{800233}$ | 20×158 | 50 |
| Can be combined with the Eurotec T-profile and all |  | 800234 | $20 \times 178$ | 50 |
| common T-profiles |  | 800235 | 20× 198 | 50 |
| Can be used in service classes 1 and 2 |  | 800236 | 20x218 | 50 |
|  |  | 800237 | $20 \times 238$ | 50 |
|  |  | 800238 | $20 \times 258$ | 50 |
| INSTRUCTIONS FOR USE |  | 800239 | 20×278 | 50 |
|  |  | 800240 | 20×298 | 50 |





## CONSTRUCTVE FASTENERS

| Rock concrete screw | $66-71$ |
| :--- | ---: |
| Konstrux fully threaded screw | $72-97$ |
| Angle-bracket screw | $98-99$ |
| Panelwwistec | $100-115$ |
| SawTec | $116-119$ |
| Topduo roofing screw | $120-125$ |

## Gurotec | Constructive fastening

## ROCK CONCRETE SCREW

For fastening to concrete without plugs

The rock-concrete screw is screwed directly into the drill hole without inserting dowels or other additional components. Thanks to the short axial and edge distances in the installation, they also have no spreading effect. Besides being very simple, this type of installation is also impressively time-saving and offers maximum cost savings.

The high-strength bolt steel and the complex hardening process ensure reliable use in both cracked and non-cracked concrete of class $\mathrm{C} 20 / 25$ to C50/60.



| Rock concrete screw <br> Hexagonal, galvanised steel | (iv) CE | Art. no. | Dimensions [mm] | Head | PU |
| :---: | :---: | :---: | :---: | :---: | :---: |
|  |  | 110338* | 7,5 $\times 40$ | SWI3 | 100 |
|  |  | 110339* | 7,5 $\times 50$ | SWI3 | 100 |
| T |  | 110340 | 7,5 $\times 60$ | SWI3 | 100 |
| 11 |  | 110341 | 7,5 $\times 80$ | SWI3 | 100 |
| 11 |  | 110342* | $10,5 \times 60$ | SW15 | 100 |
|  |  | 110343 | $10,5 \times 80$ | SW15 | 100 |
|  |  | 110344 | 10,5 $\times 100$ | SW15 | 100 |
|  |  | 110345 | $10,5 \times 120$ | SW15 | 100 |
|  |  | 110346 | $10,5 \times 140$ | SW15 | 100 |
|  |  | 110347 | $10,5 \times 160$ | SW15 | 100 |
|  |  | 110336* | $12,5 \times 60$ | SWI7 | 100 |
|  |  | 110337 | $12,5 \times 80$ | SWI7 | 100 |
|  |  | 110327 | 12,5 $\times 100$ | SW17 | 100 |
|  |  | 110328 | $12,5 \times 120$ | SWI7 | 100 |
|  |  | 110329 | 12,5x 140 | SW17 | 100 |
|  |  | 110330 | $12,5 \times 160$ | SWI7 | 50 |
|  |  | 110331 | 12,5 $\times 180$ | SWI7 | 50 |
|  |  | 110332 | $12,5 \times 200$ | SW17 | 50 |
|  |  | 110333 | $12,5 \times 240$ | SWI7 | 50 |
|  |  | 110334 | $12,5 \times 280$ | SWI7 | 50 |
|  |  | $110335$ | $12,5 \times 320$ | SWI7 | 50 |
|  |  | * Screws n |  |  |  |
| Rock concrete screw <br> Countersunk head, galvanised steel |  | Art. no. | Dimensions [mm] | Drive | PU |
|  | $\frac{5 \pi}{50 \cdot 15 / 6 \sin }$ | 110348* | 7,5 $\times 40$ | TX40 - | 100 |
|  |  | 110349 | 7,5 $\times 60$ | TX40 - | 100 |
| N. |  | 110350 | 7,5×80 | TX40 - | 100 |
|  |  | 110351 | 7,5 5100 | TX40 | 100 |
|  |  | 110352 | 7,5×120 | TX40 - | 100 |
|  |  | 110353 | 7,5×140 | TX40 - | 100 |
|  |  | $110354$ | 7,5×160 | TX40 ${ }^{\circ}$ | 100 |
|  |  | *Screws |  |  |  |

## USING THE ROCK CONCRETE SCREW



Create drill hole (hammer drill).


Clean drill hole.


Mount attachment.


Done!


The rock-concrete screw in a wood-concrete combination.


The rock-concrete screw in stair railings (steel-concrete combination).

## TECHNICAL INFORMATION ROCK CONCRETE SCREW



Rock, hexagonol with flange

| $\begin{aligned} & 7,5 \times 60 \\ & 1,5 \times 80 \end{aligned}$ | SW13 | 16,5 | 100 | 5 25 | 55 | 6,0 | 3,0 | 11,0 | 19,0 | 6 | 70 | 9 | 40 |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| 10,5x80 |  |  |  | 5 |  |  |  |  |  |  |  |  |  |
| 10,5 $\times 100$ |  |  |  | 25 |  |  |  |  |  |  |  |  |  |
| 10,5 $\times 120$ | SW15 | 17,5 | 160 | 45 | 75 | 6,0 | 3,0 | 22,0 | 51,0 | 9 | 90 | 12 | 55 |
| 10,5 140 |  |  |  | 65 |  |  |  |  |  |  |  |  |  |
| 10,5 $\times 160$ |  |  |  | 85 |  |  |  |  |  |  |  |  |  |
| 16,5x $\times 15$ |  |  |  | 5 |  |  |  |  |  |  |  |  |  |
| 16,5x 135 | SW18 | 30,5 | 175 | 25 | 110 | 40,0 | 30,0 | 57,9 | 235,9 | 14 | 130 | 18 | 100 |
| 16,5 $\times 160$ |  |  |  | 50 |  |  |  |  |  |  |  |  |  |

Rock, hexagonal

| $\begin{aligned} & 7,5 \times 60 \\ & 7,5 \times 80 \end{aligned}$ | SW13 | n/a | 100 | $\begin{gathered} 5 \\ 25 \end{gathered}$ | 55 | 6,0 | 3,0 | 11,0 | 19,0 | 6 | 70 | 9 | 40 |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| $10,5 \times 80$ |  |  |  | 5 |  |  |  |  |  |  |  |  |  |
| $10,5 \times 100$ |  |  |  | 25 |  |  |  |  |  |  |  |  |  |
| $10,5 \times 120$ | SW15 | n/a | 160 | 45 | 75 | 6,0 | 3,0 | 22,0 | 51,0 | 9 | 90 | 12 | 55 |
| $10,5 \times 140$ |  |  |  | 65 |  |  |  |  |  |  |  |  |  |
| $10,5 \times 160$ |  |  |  | 85 |  |  |  |  |  |  |  |  |  |
| 12,5 $\times 80$ | SWI7 | n/a | 200 | 5 | 75 | 25,0 | 12,0 | 35,0 | 98,0 | 10 | 90 | 14 | 65 |
| $12,5 \times 100$ |  |  |  | 5 |  |  |  |  |  |  |  |  |  |
| $12,5 \times 120$ |  |  |  | 25 |  |  |  |  |  |  |  |  |  |
| 12,5 $\times 140$ |  |  |  | 45 |  |  |  |  |  |  |  |  |  |
| $12,5 \times 160$ |  |  |  | 65 |  |  |  |  |  |  |  |  |  |
| $12,5 \times 180$ | SWI7 | n/a | 200 | 85 | 95 | 25,0 | 12,0 | 35,0 | 98,0 | 10 | 110 | 14 | 65 |
| $12,5 \times 200$ |  |  |  | 105 |  |  |  |  |  |  |  |  |  |
| $12,5 \times 240$ |  |  |  | 145 |  |  |  |  |  |  |  |  |  |
| 12,5 $\times 280$ |  |  |  | 185 |  |  |  |  |  |  |  |  |  |
| 12,5 $\times 320$ |  |  |  | 225 |  |  |  |  |  |  |  |  |  |



Setting tool: Electrical tangential impact wrench, max. power rating Tmax according to manvfacturer's data, recommended $\mathrm{T}_{\text {max }}: 250 \mathrm{Nm}$ for Rock $7,5 \times \mathrm{L} ; 450 \mathrm{Nm}$ for Rock $10,5 \times \mathrm{L}$. and $12,5 \mathrm{~L}$. and $16,5 \mathrm{~L}$. Note: A higher max. torque of the setting tool can lead to destruction of the drilling hole or damage to the screw.

Assembly with torque wrench: Recommended installation torgue Tinst: 20 Nm for Rock $7,5 \mathrm{xL} ; 40 \mathrm{Nm}$ for Rock $10,5 \mathrm{~L} .60 \mathrm{Nm}$ for Rock $12,5 \mathrm{x}$. and 120 Nm for $16,5 \mathrm{xL}$.
a) The calculation for a joint is to be performed according to EAAG-001 Annex C. b) Partial sofefy factors: $\gamma \mathrm{Mms}_{s}, \mathrm{~V}=1,5 ; \gamma \mathrm{Ms}, \mathrm{M}=1,5$.

Please note: These are planning dids. Projects must only be calculated by authorised persons.

## EuroTec calculation service

## Rock concrete screw according to ETA-15/0886

by phone 02331 6245-444 • by fax 02331 6245-200 • by e-mail technik@eurotec.team

Please contact our technical department or use the free calculation services in the service section of our website.

## Contact

Trader:
Contact Person: $\qquad$
e-mail: $\qquad$
Project: $\qquad$

## Project details

## Concrete

Strength category:
(if known; min. C20/25)
Construction component:
(e.g. strip footing, floor slab, wall, ceiling, etc.)

Component thickness h: $\qquad$ mm

## Attachment

$\square$ Steel


Screw selection

## Contractor:

Contact Person: $\qquad$
Phone: $\qquad$
e-mail: $\qquad$

A detailed sketch of the joint must be enclosed with the inquiry, stating the following details:

- Geometry of concrete and attachment
- Edge and centre distances $C$ and $S$
- Position of attachment relative to concrete component
- Position (and angle, where applicable) of force application point on the attachment


| $\square$ | $\varnothing 7,5 \mathrm{~mm}$ countersunk head |  |
| :--- | :--- | :--- |
| $\square$ | $\varnothing 7,5 \mathrm{~mm}$ hex head, flange $\quad \square \quad \varnothing 7,5 \mathrm{~mm}$ hex head | $\square$ |

$\varnothing 7,5 \mathrm{~mm}$ hex head
$\varnothing 10,5 \mathrm{~mm}$ hex head
$\varnothing 12,5 \mathrm{~mm}$ hex, flange
$\varnothing$ 12,5 hex head, flange

## KONSTRUX FULLY THREADED SCREW

The high-performance solution for new construction and refurbishment


Konstrux fully threaded screws maximize the load-bearing capacity of a connection due to the high thread extraction resistance in both components. When using partially threaded screws, the significantly lower head pull-through resistance in the attachment partl limits the load-bearing capacity of the connection. KonstruX fully threaded screwn provide a cost-saving alternative to traditional connectors or timber connectors such as joist shoes and joist girders.



KonstruX in order to connect a wall with a sill plate.


## Gurotec | Constructive fastening

| KonstruX threaded screw | $C E$ | Art. no. | Dimensions [mm] | Drive | PU |
| :---: | :---: | :---: | :---: | :---: | :---: |
| Cylinder head, drill point, A4 |  | 944780 | 6,5 5140 | TX30 - | 100 |
|  |  | 944781 | 6,5 $\times 160$ | TX30 | 100 |
|  |  | 944782 | 6,5 195 | TX30 - | 100 |
| 1. |  | 944783 | 8,0 $\times 155$ | TX40 - | 50 |
| 1. |  | 944784 | 8,0 195 | TX40 - | 50 |
| F |  | 944785 | 8,00220 | TX40 | 50 |
|  |  | 944786 | 8,00245 | TX40 - | 50 |
| 1 |  | 944787 | 8,0 $\times 270$ | TX40 | 50 |
|  |  | 944788 | 8,0 2295 | TX40 - | 50 |
| 7 |  | 944789 | 8,0 $\times 330$ | TX40 - | 50 |
| , |  | 944790 | $8,0 \times 375$ | TX40 - | 50 |
|  |  | 94479 | $8,0 \times 400$ | TX40 ${ }^{\circ}$ | 50 |

## KonstruX ST fully threaded screw <br> Countersunk head, drill point, galvanised <br> 

| Art. no. | Dimensions [mm] | Drive | PU |
| :---: | :---: | :---: | :---: |
| 908857 | 6,5x80 | TX30 | 100 |
| 90858 | 6,5×100 | TX30 | 100 |
| 908859 | 6,5×120 | TX30 | 100 |
| 908860 | $6,5 \times 140$ | TX30 | 100 |
| 904790 | 8,0 $\times 95$ | TX40 - | 50 |
| 90479 | $8,0 \times 125$ | TX40 - | 50 |
| 90479 | $8,0 \times 155$ | TX40 | 50 |
| 904793 | $8,0 \times 195$ | TX40 - | 50 |
| 90479 | 8,0 220 | TX40 - | 50 |
| 904795 | 8,0 245 | TX40 - | 50 |
| 90479 | 8,0 272 | TX40 | 50 |
| 90479 | $8,0 \times 295$ | TX40 - | 50 |
| 904798 | 8,0×330 | TX40 - | 50 |
| 90479 | 8,0x375 | TX40 - | 50 |
| 90880 | $8,0 \times 400$ | TX40 - | 50 |
| 908801 | $8,0 \times 430$ | TX40 | 50 |
| 908802 | $8,0 \times 480$ | TX40 | 50 |
| 908803 | 8,0x545 | TX40 - | 50 |
| 90470 | $10,0 \times 125$ | TX50 - | 25 |
| 90477 | $10,0 \times 155$ | TX50 - | 25 |
| 904772 | 10,0 $\times 195$ | TX50 - | 25 |
| 90473 | $10,0 \times 220$ | TX50 | 25 |
| 90477 | 10,0 245 | TX50 - | 25 |
| 90475 | 10,0 $\times 270$ | TX50 - | 25 |
| 90476 | 10,0 $\times 300$ | TX50 - | 25 |
| 90477 | $10,0 \times 330$ | TX50 - | 25 |
| 904778 | $10,0 \times 360$ | TX50 - | 25 |
| 90479 | 10,0 $\times 400$ | TX50 | 25 |
| 904780 | $10,0 \times 450$ | TX50 - | 25 |
| 904781 | 10,0 $\times 500$ | TX50 - | 25 |
| 90478 | 10,0x550 | TX50 - | 25 |
| 904783 | $10,0 \times 600$ | TX50 - | 25 |


| KonstruX ST fully threaded screw | 迷: | Art. no. | Dimensions [mm] | Drive | PU |
| :---: | :---: | :---: | :---: | :---: | :---: |
| Countersunk head, galvanised |  | 905737 | 11,3x300 | TX50 - | 20 |
|  |  | 905738 | 11,3x340 | TX50 - | 20 |
| 1 |  | 905739 | 11,3x380 | TX50 - | 20 |
| 1 |  | 905740 | 11,3x420 | TX50 - | 20 |
| 1 |  | 90574 | $11,3 \times 460$ | TX50 - | 20 |
| 1 |  | 905742 | 11,3x500 | TX50 - | 20 |
| 1 |  | 905743 | 11,3x540 | TX50 - | 20 |
| 1 |  | 90574 | $11,3 \times 580$ | TX50 - | 20 |
| 1 |  | 905745 | $11,3 \times 620$ | TX50 - | 20 |
| 1 |  | 905746 | 11,3x660 | TX50 - | 20 |
| 1 |  | 90574 | 11,3x700 | TX50 | 20 |
| 1 |  | 905748 | $11,3 \times 750$ | TX50 - | 20 |
| 1 |  | 905749 | 11,3x800 | TX50 - | 20 |
| 1 |  | 904750 | 11,3x900 | TX50 - | 20 |
| 1 |  | 90475 | $11,3 \times 1000$ | TX50 | 20 |


| KonstruX fully threaded screw | M $7 M$ | Art. no. | Dimensions [mm] | Drive | PU |
| :---: | :---: | :---: | :---: | :---: | :---: |
| TX head, galvanised | to our protut range | 90883 | $13,0 \times 200$ | TX50 - | 20 |
| 㬉 |  | 908836 | $13,0 \times 220$ | TX50 | 20 |
|  | CE | 904837 | $13,0 \times 240$ | TX50 - | 20 |
|  | Applied for | 90438 | $13,0 \times 260$ | TX50 - | 20 |
| 11. |  | 90833 | $13,0 \times 280$ | TX50 - | 20 |
|  |  | 908840 | $13,0 \times 300$ | TX50 - | 20 |
|  |  | 90884 | $13,0 \times 320$ | TX50 - | 20 |
|  |  | 908842 | $13,0 \times 340$ | TX50 - | 20 |
|  |  | 908843 | $13,0 \times 360$ | TX50 - | 20 |
| - |  | 90884 | $13,0 \times 380$ | TX50 - | 20 |
|  |  | 908845 | $13,0 \times 420$ | TX50 - | 20 |
|  |  | 908846 | $13,0 \times 460$ | TX50 - | 20 |
|  |  | 90884 | $13,0 \times 500$ | TX50 - | 20 |
|  |  | 904848 | $13,0 \times 540$ | TX50 - | 20 |
|  |  | 90849 | $13,0 \times 580$ | TX50 - | 20 |
| 1 |  | 908850 | $13,0 \times 620$ | TX50 - | 20 |
|  |  | 90485 | $13,0 \times 660$ | TX50 - | 20 |
|  |  | 90855 | $13,0 \times 700$ | TX50 - | 20 |
|  |  | 90855 | $13,0 \times 750$ | TX50 - | 20 |
| F |  | 90855 | $13,0 \times 800$ | TX50 - | 20 |
| F |  | 90885 | $13,0 \times 900$ | TX50 - | 20 |
|  |  | 90855 | $13,0 \times 1000$ | TX50 - | 20 |
| 1 |  | 90886 | $13,0 \times 1200$ | TX50 - | 20 |
| 1: |  | 90862 | $13,0 \times 1400$ | TX50 - | 20 |

## KONSTRUX DUO

Fully threaded screw with compressive effect


The KonstruX DUO combines the strengths of fully threaded and partially threaded screws: Maximisation of the connection's load-bearing capacity through equally high pull-out resistance in both structural elements and compression effect achieved by having different thread pitches in the section underneath the head and in the driving thread.


KonstruX DUO
Cylinder head, drill point, galvanised

| 滑 | Art. no. | Dimensions [mm] | Drive | PU |
| :---: | :---: | :---: | :---: | :---: |
|  | 100606 | 6,5 590 | TX30 | 100 |
|  | 10060 | 6,5×130 | TX30 | 100 |
|  | 10060 | 6,5×160 | TX30 - | 100 |
|  | 100609 | 6,5×190 | TX30 | 100 |
|  | 100610 | 6,5 $\times 220$ | TX30 - | 100 |
|  | 100611 | $8,0 \times 160$ | TX40 | 100 |
|  | 100612 | $8,0 \times 190$ | TX40 - | 100 |
|  | 100613 | 8,0×220 | TX40 | 100 |
|  | 100614 | 8,0×245 | TX40 - | 100 |
|  | 100615 | 8,0×280 | TX40 - | 100 |
|  | 100616 | 8,0× 300 | TX40 ${ }^{\circ}$ | 100 |
|  | 100617 | 8,0×330 | TX40 | 100 |
|  | 100618 | 8,0×400 | TX40 | 100 |



## Gurotec | Constructive fastening

## EXAMPLE APPLCATIONS: CEILING ELEMENTS



EXAMPLE APPLCATIONS: WALL ELEMENTS


## Gurotec | Constructive fastening

## EXAMPLE APPLCATIONS: ROOF ELEMENTS



Mitred roof panels, screw connection with ridge purlin.


Mitred roof panels, diagonal screw connection.


Roof panels on butt joint, diagonal screw connection.

EXAMPLE APPLICATIONS: STAIRCASE CONSTRUCTION WITH CLT



Attach the tier end frontal to the tier support.



## Gurotec | Constructive fastening

THE FAST AND SECURE TIMBER-JOINT SYSTEM KONSTRUX CYLINDER-HEAD/COUNTERSUNK-HEAD SCREWS

| Appliction examples | Cylinder head |  |  | Countersunk head |  |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
|  | $\begin{gathered} 06,5 \\ {[\mathrm{~mm}]} \end{gathered}$ | $\begin{aligned} & 08,0 \\ & {[\mathrm{~mm}]} \end{aligned}$ | $\begin{aligned} & 010,0 \\ & {[\mathrm{~mm}]} \end{aligned}$ | $\begin{aligned} & 06,5 \\ & {[\mathrm{~mm}]} \end{aligned}$ | $\begin{aligned} & 98,0 \\ & {[\mathrm{~mm}]} \end{aligned}$ | $\begin{aligned} & 010,0 \\ & {[\mathrm{~mm}]} \end{aligned}$ | $\begin{gathered} 011,3 \\ {[\mathrm{~mm}]} \end{gathered}$ |
|  | X | X | X | X | X | X | $x$ |
|  | X | X | x | X | x | X | x |
|  | - | - | - | X | X | X | X |
|  | - | - | - | X | $x$ | X | x |
| Main-secondary beam connection | X | X | $x$ | X | $x$ | X | - |
|  | X | X | x | X | $x$ | X | X |
|  | X | X | X | X | X | X | x |
|  | - | X | X | - | X | X | X |

Transverse-shear reinforcement of building trusses


## KONSTRUX FULLY THREADED SCREW

## Technical information

KONSTRUX ST WITH CYLINDER HEAD AND DRILL POINT
6,5 BIS 10,0 MM: TIMBER-TIMBER JOINTS


[^2]Example:
Characterisic value for constant load (dead weight) $G_{k}=2,00 \mathrm{kN}$ and variable load (e. g. snow lood) $Q_{k}=3,00 \mathrm{kN} . \mathrm{kmod}=0,9 \cdot \gamma M=1,3$.
$\rightarrow$ Dimensioning value of the load $\mathrm{E}=2,00 \cdot 1,35+3,00 \cdot 1,5=7,20 \mathrm{kN}$.
The lood-bearing capacity of the joint is therefore considered to have been demonstrated if $R_{d} \geq \mathrm{E}_{d} \rightarrow \min R_{k}=R_{d} \cdot \gamma_{M} / k_{\text {mod }}$
i.e. the characteristic minimum value is calculated based on: $\min \mathrm{Rk}=\mathrm{Rd}_{\mathrm{d}} \cdot \gamma \mathrm{M}_{\mathrm{M}} / \mathrm{kmod} \rightarrow \mathrm{Rk}=7,20 \mathrm{kN} \cdot 1,3 / 0,9=10,40 \mathrm{kN} \rightarrow$ comparison with table values.

Please note: These are planning ciids. Projects must only be calculated by outhorised persons.

## KONSTRUX ST WITH CYLINDER HEAD AND DRILL POINT 6,5 BIS 10,0 MM: TIMBER-TIMBER JOINTS

Dimensions Tension comnection


Calculation according to $\mathrm{EA}-\mathrm{II} / 0024$. Wood density $\rho \mathrm{k}=380 \mathrm{~kg} / \mathrm{m}^{3}$. All mechanical values provided should be viewed as subject to the assumptions that have been made and represent example calculations.
All values are calculated minimum values and are subject to typographical and printing errors.
a) The characterisic values of the lood-bearing capacity $R_{k}$ cannot be treated as equivalent to the max. possible load (the max. force). Characterisicic values of the load-bearing capacity $R_{k}$ should be reduced to dimensioning values
$R_{d}$ with regard to the usage class and dass of the load duration: $R_{d}=R_{k} \cdot k m o d / \gamma M$. The dimensioning values of the lood- bearing capacity $R_{d}$ should be contrasted with the dimensioning values of the loods $\left(R_{d} \geq E_{d}\right)$.
Example:
Characteristic value for constant lood (dead weight) $G \mathrm{Gk}=2,00 \mathrm{kN}$ and variable load (e. g. snow load) $\mathrm{Qk}=3,00 \mathrm{kN}$. kmod= $=0,9 . \gamma \mathrm{M}=1,3$.
$\rightarrow$ Dimensioning value of the load $\mathrm{Ed}=2,00 \cdot 1,35+3,00 \cdot 1,5=7,20 \mathrm{kN}$.
The lood-bearing capacity of the joint is therefore considered to have been demonstrated if Rd $\geq \mathrm{Ed} . \rightarrow \min \mathrm{Rk}=\mathrm{Rd} \cdot \gamma \mathrm{M} / \mathrm{kmod}$
i.e. the characteristic minimum value is calculated based on: $\min \mathrm{Rk}=\mathrm{Rd} \cdot \gamma \mathrm{M} / \mathrm{kmod} \rightarrow \mathrm{Rk}=7,2 \mathrm{kN} \cdot 1,3 / 0,9=10,40 \mathrm{kN} \rightarrow$ comparison with table values.

Please note: These are planning aids. Projects must only be calculated by authorised persons.

KONSTRUX ST WITH COUNTERSUNK HEAD AND DRILL POINT 6,5 BIS 10,0 MM: TIMBER-TIMBER JOINTS


[^3]KONSTRUX ST WITH COUNTERSUNK HEAD AND DRILL POINT 8,0 AND 10,0 MM: TIMBER-TIMBER JOINTS


Characterisicic value of the join's loadbearing capacity Rax,k bzw. Rk acc. to ETA-II/0024

| dl xL [mm] | A[mm] | $B$ [mm] |  | $\mathrm{Rk}^{\text {a) }} \cdot[\mathrm{KN}]$ | $\mathrm{Rax}_{\text {a }} \mathrm{k}^{\text {a) }}$-[kN] | $\mathrm{Rk}^{\text {a) }}$-[kN] | $\mathrm{Raxa}_{\text {a }} \mathrm{k}^{\text {a) }}$-[kN] | $\mathrm{Rk}^{\text {a) }} \cdot[\mathrm{kN}]$ | $\mathrm{Rax}_{\mathrm{ax}} \mathrm{K}^{\text {a) }} \cdot[\mathrm{KN}]$ | $\mathrm{Rk}^{\text {a) }} \cdot[\mathrm{kN}]$ |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
|  |  |  | $\alpha=45^{\circ}$ |  | $\begin{aligned} & \alpha A=90^{\circ} \\ & \alpha B=45^{\circ} \end{aligned}$ |  | $\begin{aligned} & \alpha_{A}=90^{\circ} \\ & \alpha_{B}=90^{\circ} \end{aligned}$ |  | $\begin{aligned} & \alpha A=45^{\circ} \\ & \alpha B=90^{\circ} \end{aligned}$ |  |
| 8,0 155 | 60 | 60 | 6,65 | 4,70 | 6,65 | 4,70 | 6,65 | 4,70 | 6,65 | 4,70 |
| $8,0 \times 195$ | 80 | 80 | 7,76 | 5,49 | 7,76 | 5,49 | 7,16 | 5,49 | 7,76 | 5,49 |
| 8,0 220 | 80 | 100 | 10,13 | 7,17 | 10,13 | 7,17 | 10,13 | 7,17 | 10,13 | 7,17 |
| 8, 0245 | 100 | 100 | 9,82 | 6,95 | 9,82 | 6,95 | 9,82 | 6,95 | 9,82 | 6,95 |
| 8,0 $\times 270$ | 100 | 120 | 12,19 | 8,62 | 12,19 | 8,62 | 12,19 | 8,62 | 12,19 | 8,62 |
| 8, $\times 295$ | 120 | 100 | 11,88 | 8,40 | 11,88 | 8,40 | 11,88 | 8,40 | 11,88 | 8,40 |
| 8,0 3330 | 120 | 140 | 15,20 | 10,75 | 15,20 | 10,75 | 15,20 | 10,75 | 15,20 | 10,75 |
| 8,0x 375 | 140 | 140 | 16,79 | 11,87 | 16,79 | 11,87 | 16,79 | 11,87 | 16,79 | 11,87 |
| $8,0 \times 400$ | 160 | 140 | 16,48 | 11,65 | 16,48 | 11,65 | 16,48 | 11,65 | 16,48 | 11,65 |
| $8,0 \times 430$ | 160 | 160 | 19,32 | 13,66 | 19,32 | 13,66 | 19,32 | 13,66 | 19,32 | 13,66 |
| $8,0 \times 480$ | 180 | 180 | 21,38 | 15,12 | 21,38 | 15,12 | 21,38 | 15,12 | 21,38 | 15,12 |
| $10,0 \times 220$ | 80 | 100 | 12,33 | 8,72 | 12,33 | 8,72 | 12,33 | 8,72 | 12,33 | 8,72 |
| 10,0 245 | 100 | 100 | 11,95 | 8,45 | 11,95 | 8,45 | 11,95 | 8,45 | 11,95 | 8,45 |
| $10,0 \times 270$ | 100 | 120 | 14,83 | 10,49 | 14,83 | 10,49 | 14,83 | 10,49 | 14,83 | 10,49 |
| $10,0 \times 300$ | 120 | 120 | 15,03 | 10,63 | 15,03 | 10,63 | 15,03 | 10,63 | 15,03 | 10,63 |
| $10,0 \times 330$ | 120 | 140 | 18,49 | 13,07 | 18,49 | 13,07 | 18,49 | 13,07 | 18,49 | 13,07 |
| $10,0 \times 360$ | 140 | 140 | 18,69 | 13,21 | 18,69 | 13,21 | 18,69 | 13,21 | 18,69 | 13,21 |
| $10,0 \times 400$ | 160 | 140 | 20,04 | 14,17 | 20,04 | 14,17 | 20,04 | 14,17 | 20,04 | 14,17 |
| $10,0 \times 450$ | 160 | 180 | 25,81 | 18,25 | 25,81 | 18,25 | 25,81 | 18,25 | 25,81 | 18,25 |
| $10,0 \times 500$ | 180 | 200 | 28,31 | 20,02 | 28,31 | 20,02 | 28,31 | 20,02 | 28,31 | 20,02 |
| 10,0 $\times 550$ | 200 | 200 | 30,82 | 21,79 | 30,82 | 21,79 | 30,82 | 21,79 | 30,82 | 21,79 |
| $10,0 \times 600$ | 220 | 220 | 33,00 | 23,33 | 33,00 | 23,33 | 33,00 | 23,33 | 33,00 | 23,33 |

Calculation according to ETA- $1 \mathrm{l} / 0024$. Wood density $\mathrm{pk}=380 \mathrm{~kg} / \mathrm{m}^{3}$. All mechanical values provided should be viewed as subject to the assumptions that have been made and represent example calculations.
All values are calculated minimum values and are subject to typographical and printing errors.
a) The characteristic values of the load-bearing capacity Rk cannot be treated as equivalent to the max. possible lood (the max. force). Characterisicic values of the lood-bearing capacity Rk should be reduced to dimensioning values Rd
with regard to the usage class and class of the load duration: $R_{d}=R_{k} \cdot k_{m o d} / \gamma M$. The dimensioning values of the load-bearing capacity $R_{d}$ should be contrasted with the dimensioning values of the loads ( $\left.R_{d} \geq E_{d}\right)$.
Example:
Characteristic value for constant load (dead weight) $G \mathrm{~K}=2,00 \mathrm{kN}$ and variable load (e. g. snow load) $\mathrm{Qk}=3,00 \mathrm{kN} . \mathrm{kmod}=0,9 . \gamma \mathrm{M}=1,3$.
$\rightarrow$ Dimensioning value of the load $\mathrm{Ed}=2,00 \cdot 1,35+3,00 \cdot 1,5=7,20 \mathrm{kN}$.
The load-bearing capacity of the joint is therefore considered to have been demonstrated if $R_{d} \geq \mathrm{E}_{d} \rightarrow \min R_{k}=R_{d} \cdot \gamma M / k_{\text {mod }}$
i.e. the characterisicic minimum value is calculated based on: $\min \mathrm{Rk}=\mathrm{Rd}_{\mathrm{d}} \cdot \gamma \mathrm{M} / \mathrm{kmod} \rightarrow \mathrm{Rk}=7,20 \mathrm{kN} \cdot 1,3 / 0,9=10,40 \mathrm{kN} \rightarrow$ comparison with table values.

Please note: These are planning cids. Projects must only be calculated by authorised persons.
Please note: Verify the assumptions made. The stated values, and type and number of joining devices are based on preliminary meassurements. Projects are to be dimensioned exclusively by cuthorised persons in accordance with the State Building Code. As per LBauO, please contact a qualified structural engineer for a paid proof of stability. We will be happy to refer you to someone.

KONSTRUX WITH COUNTERSUNK HEAD AND DRILL POINT OR AG TIP 11,3 MM: TIMBER-TIMBER CONNECTION

| Dimensions |  |  | Extraction resistance | Shearing |  |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
|  |  |  | Characterisicic valve of the join's's loadbearing capacity Rax,k acc. to EAA-11/0024 | $V\left(\alpha=0^{\circ}\right)$ <br> $V\left(\alpha=0^{\circ}\right)$ <br> $\square$ <br> $V\left(\alpha=90^{\circ}\right)$ $\qquad$ $V\left(\alpha=90^{\circ}\right)$ | Characterersic <br> loadbearing capacit | $V\left(\alpha=0^{\circ}\right)$ $V\left(\alpha=90^{\circ}\right)$ $V\left(\alpha=90^{\circ}\right)$ $V\left(\alpha=0^{\circ}\right)$ <br> of the joint's . to ETA-II/0024 |  |
| $\mathrm{dl} \times \mathrm{L}$ [mm] | A [mm] | B [mm] | $\mathrm{Rax}, \mathrm{k}^{\text {a) }}$ [ $[\mathrm{kN}]$ | $R_{k}{ }^{(a)}-[k N]$ | $\mathrm{R}^{\text {a) }}$-[kN] | $R_{k}{ }^{\text {a) }}-[k N]$ | $R_{k}{ }^{\text {a) }}-[k N]$ |
|  |  |  |  | $\alpha=0^{\circ}$ | $\alpha=90^{\circ}$ | $\begin{aligned} & \alpha A=0^{\circ} \\ & \alpha B=90^{\circ} \end{aligned}$ | $\begin{aligned} & \alpha A=90^{\circ} \\ & \alpha B=0^{\circ} \end{aligned}$ |
| $11,3 \times 300$ | 160 | 160 | 18,25 | 12,17 | 10,73 | 10,73 | 12,17 |
| $11,3 \times 340$ | 180 | 180 | 20,85 | 12,82 | 11,38 | 11,38 | 12,82 |
| $11,3 \times 380$ | 200 | 200 | 23,46 | 13,47 | 12,03 | 12,03 | 13,47 |
| $11,3 \times 420$ | 220 | 220 | 26,07 | 14,12 | 12,34 | 12,34 | 14,12 |
| $11,3 \times 460$ | 240 | 240 | 26,67 | 14,77 | 12,34 | 12,34 | 14,77 |
| $11,3 \times 500$ | 260 | 260 | 31,28 | 15,21 | 12,34 | 12,34 | 15,21 |
| $11,3 \times 540$ | 280 | 280 | 33,89 | 15,21 | 12,34 | 12,34 | 15,21 |
| $11,3 \times 580$ | 300 | 300 | 36,49 | 15,21 | 12,34 | 12,34 | 15,21 |
| $11,3 \times 620$ | 320 | 320 | 39,10 | 15,21 | 12,34 | 12,34 | 15,21 |
| $11,3 \times 660$ | 340 | 340 | 41,71 | 15,21 | 12,34 | 12,34 | 15,21 |
| 11,3x700 | 360 | 360 | 44,32 | 15,21 | 12,34 | 12,34 | 15,21 |
| 11,3x750 | 380 | 380 | 48,23 | 15,21 | 12,34 | 12,34 | 15,21 |
| $11,3 \times 800$ | 400 | 420 | 50,00 | 15,21 | 12,34 | 15,21 | 12,34 |
| $11,3 \times 900$ | 460 | 460 | 50,00 | 15,21 | 12,34 | 12,34 | 15,21 |
| $11,3 \times 1000$ | 500 | 520 | 50,00 | 15,21 | 12,34 | 15,21 | 12,34 |

Calculation according to $\mathrm{ETA}-\mathrm{II} / 0024$. Wood density $\rho \mathrm{k}=380 \mathrm{~kg} / \mathrm{m}^{3}$. All mechanical values provided should be viewed as subject to the assumptions that have been made and represent example calculations.
All values are calculated minimum values and are subject to typographical and printing errors.
a) The characteristic values of the lood-bearing capacity Rk cannot be treated as equivalent to the max. possible load (the max. force). Characterisicic values of the load-bearing capacity Rk should be reduced to dimensioning valves $R d$ with regard to the usage class and class of the load duration: $\mathrm{Rd}=\mathrm{Rk} \cdot \mathrm{kmod} / \gamma M$. The dimensioning values of the load- bearing capacity $\mathrm{Rd}_{\mathrm{d}}$ should be contrasted with the dimensioning values of the loads ( $\mathrm{Rd} \geq \mathrm{Ed}$ ).

Example:
Characteristic value for constant load (dead weight) Gk= $2,00 \mathrm{kN}$ and variable load (e. g. snow load) $\mathrm{Qk}=3,00 \mathrm{kN} . \mathrm{kmod}=0,9 . \gamma \mathrm{M}=1,3$.
$\rightarrow$ Dimensioning value of the load $E d=2,00 \cdot 1,35+3,00 \cdot 1,5=7,20 \mathrm{kN}$.
The lood-bearing capacity of the joint is therefore considered to have been demonstrated if $\mathrm{Rd}_{\mathrm{d}} \geq \mathrm{Ed}_{\mathrm{d}} \rightarrow \mathrm{min} \mathrm{Rk}_{k}=\mathrm{Rd}_{d} \cdot \gamma \mathrm{M} / \mathrm{kmod}$
i.e. the characterisicic minimum value is calculated based on: $\min \mathrm{Rk}=\mathrm{R}_{\mathrm{d}} \cdot \gamma \mathrm{M} / \mathrm{kmod} \rightarrow \mathrm{Rk}=7,20 \mathrm{kN} \cdot 1,3 / 0,9=10,40 \mathrm{kN} \rightarrow$ comparison with table values.

Please note: These are planning aids. Projects must only be calculated by authorised persons.

KONSTRUX WITH COUNTERSUNK HEAD AND DRILL POINT OR AG TIP 11,3 MM: TIMBER-TIMBER CONNECTION


Calculation according to EA- $11 / 0024$. Wood density $\rho \mathrm{k}=380 \mathrm{~kg} / \mathrm{m}^{3}$. All mechanical values provided should be viewed as subject to the assumptions that have been made and represent example calculations.
All values are calculated minimum values and are subject to typographical and printing errors.
a) The characteristic valves of the load-bearing capacity Rk cannot be treated os equivalent to the max. possible load (the max. force). Characterisicic values of the load-bearing capacity Rk should be reduced to dimensioning values $R_{d}$
with regard to the usage class and dlass of the load duration: $R_{d}=R_{k} \cdot k_{m o d} / \gamma M$. The dimensioning values of the load-bearing capacity $R_{d}$ should be contrasted with the dimensioning values of the loads (Rd $\left.\geq E_{d}\right)$.
Example:
Characterisicic value for constant load (dead weight) Gk= $2,00 \mathrm{kN}$ and variable lood (e. g. snow load) $\mathrm{Qk}=3,00 \mathrm{kN} . \mathrm{kmod}=0,9 . \gamma \mathrm{M}=1,3$.
$\rightarrow$ Dimensioning value of the load $\mathrm{Ed}=2,00 \cdot 1,35+3,00 \cdot 1,5=7,20 \mathrm{kN}$.
The load-bearing capacity of the joint is therefore considered to have been demonstrated if $\mathrm{Rd}_{d} \geq \mathrm{Ed}_{\mathrm{d}} \rightarrow \min \mathrm{R}_{\mathrm{k}}=\mathrm{R}_{d} \cdot \gamma \mathrm{M} / \mathrm{kmod}$
i.e. the characterisitic minimum value is calculated based on: $\min \mathrm{R}_{\mathrm{k}}=\mathrm{R}_{\mathrm{d}} \cdot \gamma \mathrm{M} / \mathrm{kmod} \rightarrow \mathrm{R}_{\mathrm{k}}=7,20 \mathrm{kN} \cdot 1,3 / 0,9=10,40 \mathrm{kN} \rightarrow$ comparison with table values.

Please note: These are planning aids. Projects must only be calculated by authorised persons.

KONSTRUX ST WITH COUNTERSUNK HEAD AND DRILL POINT 6,5 TO 10,0 MM: STEEL-TIMBER JOINTS

| Dimensions |  |  |  | Extraction resistance | Tension connection |  |  |  | Shearing |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
|  |  |  |  |  <br> Charocterisicic value of the pion's loodhbering capacity Rax, a acc. 10 EAA-II/0024 |  |  | $\qquad$ <br> $R_{k}\left(\alpha=90^{\circ}\right)$ <br> int's loadbearing ca 0 ETA-II/0024 |  |  | $\square$ <br> - <br> ejoint's <br> to ETA-Il/0024 |
| dl xL[mm] | t[mm] | B [mm] | $\mathrm{B} 45^{\circ}$ [mm] | $\mathrm{Rax}, \mathrm{k}^{\text {(1) }}$.[kN] | $\mathrm{Rax}, \mathrm{k}^{\text {a }}$ ) $[\mathrm{FkN}]$ | $\mathrm{Rax}, 1^{1} \mathrm{l}^{\text {a) }}$ [ [kN] | $\mathrm{Rk}^{\text {a) }}$. [kN] | $\mathrm{Rk}^{\text {a) }}$. [kN] | $\mathrm{Rk}^{(0)} \cdot[\mathrm{kN}]$ | $\mathrm{Rk}^{\text {a) }}$-[kN] |
|  |  |  |  |  | $\alpha=45^{\circ}$ | $\alpha=90^{\circ}$ | $\alpha=45^{\circ}$ | $\alpha=90^{\circ}$ | $\alpha=0^{\circ}$ | $\alpha=90^{\circ}$ |
| 6,5x80 | 15 | 80 | 60 | 5,14 | 4,65 | 4,65 | 3,29 | 3,29 | 4,17 | 3,52 |
| 6,5 5100 | 15 | 100 | 80 | 6,73 | 6,24 | 6,24 | 4,41 | 4,41 | 4,17 | 3,52 |
| 6,5 120 | 15 | 120 | 80 | 8,31 | 1,82 | 7,82 | 5,53 | 5,53 | 4,17 | 3,52 |
| 6,5 140 | 15 | 140 | 100 | 9,89 | 9,40 | 9,40 | 6,65 | 6,65 | 4,17 | 3,52 |
| $8,0 \times 95$ | 15 | 100 | 80 | 1,59 | 1,00 | 1,00 | 4,95 | 4,95 | 6,18 | 5,22 |
| 8,0 125 | 15 | 120 | 100 | 10,43 | 9,84 | 9,84 | 6,\% | 6,96 | 6,18 | 5,22 |
| $8,0 \times 155$ | 15 | 160 | 120 | 13,28 | 12,69 | 12,69 | 8,97 | 8,97 | 6,18 | 5,22 |
| 8, 10195 | 15 | 200 | 140 | 17,07 | 16,48 | 16,48 | 11,65 | 11,65 | 6,18 | 5,22 |
| 8,0 220 | 15 | 220 | 160 | 19,44 | 18,85 | 18,85 | 13,33 | 13,33 | 6,18 | 5,22 |
| 8,0 245 | 15 | 240 | 180 | 21,81 | 21,22 | 21,22 | 15,01 | 15,01 | 6,18 | 5,22 |
| 8, $\times 270$ | 15 | 280 | 200 | 24,18 | 23,59 | 23,59 | 16,68 | 16,68 | 6,18 | 5,22 |
| 8,0 295 | 15 | 300 | 220 | 25,00 | 25,00 | 25,00 | 17,68 | 17,68 | 6,18 | 5,22 |
| 8,0x 330 | 15 | 340 | 240 | 25,00 | 25,00 | 25,00 | 17,68 | 17,68 | 6,18 | 5,22 |
| 8,0x 375 | 15 | 380 | 280 | 25,00 | 25,00 | 25,00 | 17,68 | 17,68 | 6,18 | 5,22 |
| 8,0x 400 | 15 | 400 | 280 | 25,00 | 25,00 | 25,00 | 17,68 | 17,68 | 6,18 | 5,22 |
| $8,0 \times 430$ | 15 | 440 | 300 | 25,00 | 25,00 | 25,00 | 17,68 | 17,68 | 6,18 | 5,22 |
| 8,0x 480 | 15 | 480 | 340 | 25,00 | 25,00 | 25,00 | 17,68 | 17,68 | 6,18 | 5,22 |
| 10,0 $\times 125$ | 15 | 120 | 100 | 12,69 | 11,97 | 11,97 | 8,46 | 8,46 | 8,72 | 1,30 |
| 10,0 $\times 155$ | 15 | 160 | 120 | 16,15 | 15,43 | 15,43 | 10,91 | 10,91 | 8,72 | 1,30 |
| 10,0 195 | 15 | 200 | 140 | 20,76 | 20,05 | 20,05 | 14,17 | 14,17 | 8,72 | 1,30 |
| $10,0 \times 220$ | 15 | 220 | 160 | 23,65 | 22,93 | 22,93 | 16,21 | 16,21 | 8,72 | 1,30 |
| 10,0 245 | 15 | 240 | 180 | 26,53 | 25,81 | 25,81 | 18,25 | 18,25 | 8,72 | 1,30 |
| $10,0 \times 270$ | 15 | 280 | 200 | 29,41 | 28,70 | 28,70 | 20,29 | 20,29 | 8,72 | 1,30 |
| $10,0 \times 300$ | 15 | 300 | 220 | 32,87 | 32,16 | 32,16 | 22,74 | 22,74 | 8,72 | 1,30 |
| 10,0 $\times 330$ | 15 | 340 | 240 | 33,00 | 33,00 | 33,00 | 23,33 | 23,33 | 8,72 | 1,30 |
| 10,0 $\times 360$ | 15 | 360 | 260 | 33,00 | 33,00 | 33,00 | 23,33 | 23,33 | 8,72 | 1,30 |
| $10,0 \times 400$ | 15 | 400 | 280 | 33,00 | 33,00 | 33,00 | 23,33 | 23,33 | 8,72 | 1,30 |
| 10,0x 450 | 15 | 460 | 320 | 33,00 | 33,00 | 33,00 | 23,33 | 23,33 | 8,72 | 1,30 |
| $10,0 \times 500$ | 15 | 500 | 360 | 33,00 | 33,00 | 33,00 | 23,33 | 23,33 | 8,72 | 1,30 |
| 10,0 $\times 550$ | 15 | 560 | 400 | 33,00 | 33,00 | 33,00 | 23,33 | 23,33 | 8,72 | 1,30 |
| 10,0 600 | 15 | 600 | 420 | 33,00 | 33,00 | 33,00 | 23,33 | 23,33 | 8,72 | 1,30 |

Calculation according to $E A-11 / 0024$. Wood density $\rho \mathrm{k}=380 \mathrm{~kg} / \mathrm{m}^{3}$. All mechanical values provided should be viewed as subject to the assumptions that have been made and represent example calculations.
All values are calculated minimum values and are subject to typographical and printing errors.
a) The characterisitic valves of the lood-bearing capacity Rk cannot be treated as equivalent to the max. possible lood (the max. force). Characterersisic values of the load-bearing capacity Rk should be reduced to dimensioning values Rd
with regard to the usage class and dlass of the load duration: $R_{d}=R_{k} \cdot \mathcal{k}_{\text {mod }} / \gamma M$. The dimensioning values of the load-bearing capacity $\mathbb{R}_{d}$ should be contrasted with the dimensioning values of the loads ( $\left.\mathbb{R}_{d} \geq \mathbb{E}_{d}\right)$.

## Example:

Characteristic valve for constant load (dead weight) $\mathrm{Gk}=2,00 \mathrm{kN}$ and variable load (e. g. snow load) $\mathrm{Qk}=3,00 \mathrm{kN}$. $\mathrm{kmod}=0,9 \cdot \gamma \mathrm{M}=1,3 . \rightarrow$ Dimensioning valve of the load $\mathrm{Ed}=2,00 \cdot 1,35+3,00 \cdot 1,5=7,20 \mathrm{kN}$. The load-bearing capacity of the joint is therefore considered to have been demonstrated if $\mathrm{R}_{d} \geq \mathrm{Ed}_{\mathrm{d}} \rightarrow \min \mathrm{R}_{\mathrm{k}}=\mathrm{R}_{d} \cdot \gamma \mathrm{M} / \mathrm{kmod}$. i.e. the characteristic minimum value is calculcated based on: $\min \mathrm{R}_{k}=\mathrm{R}_{d} \cdot \gamma \mathrm{M} / \mathrm{kmod} \rightarrow \mathrm{R}_{k}=7,20 \mathrm{kN} \cdot 1,3 / 0,9=10,40 \mathrm{kN} \rightarrow$ comparison with table values.
Please note: These are planning aids. Projects must only be calculated by authorised persons.

KONSTRUX WITH COUNTERSUNK HEAD AND DRILL POINT OR AG TIP 11,3 MM: STEEL/TIMBER CONNECTION

| Dimensions |  |  |  | Exiraction resistance | Tension connection |  |  |  | Shearing |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
|  |  |  |  |  <br> Characterisic value of the joint's loadbearing capacity Rax, a acc. to EAA-11/0024 |  | erisic value of the joint <br> sity Rax,k or Rk acc. to EI |  |  |  |  <br> of the join's <br> acc. to EA-11/0024 |
| $\mathrm{dl} \mathrm{xL}[\mathrm{mm}]$ | $\dagger$ [mm] | B [mm] | $\mathrm{B} 45^{\circ}[\mathrm{mm}]$ | ${\mathrm{Rax}, \mathrm{k}^{\text {( }} \text { ) }}^{\text {[kN] }}$ | ${\mathrm{Rax}, \mathrm{K}^{\text {a }}}^{\text {a) }} \cdot[\mathrm{kN}]$ | $\mathrm{Rax}, 1^{\text {a }}$ ( ${ }^{\text {a }}$ [kN] | $\mathrm{Rk}^{\text {a) }}$.[kN] | $\mathrm{Rk}^{\text {a) }}$-[kN] | $R_{k}{ }^{\text {a) }}$.[kN] | $\mathrm{Rk}^{\text {a) }}$. $[\mathrm{kN}]$ |
|  |  |  |  |  | $\alpha=45^{\circ}$ | $\alpha=90^{\circ}$ | $\alpha=45^{\circ}$ | $\alpha=90^{\circ}$ | $\alpha=0^{\circ}$ | $\alpha=90^{\circ}$ |
| 11,3x300 | 20 | 300 | 220 | 36,49 | 35,42 | 35,42 | 25,04 | 25,04 | 11,79 | 9,76 |
| $11,3 \times 340$ | 20 | 340 | 240 | 41,71 | 40,63 | 40,63 | 28,73 | 28,73 | 11,79 | 9,76 |
| 11,3x 380 | 20 | 380 | 260 | 46,92 | 45,84 | 45,84 | 32,42 | 32,42 | 11,79 | 9,76 |
| $11,3 \times 420$ | 20 | 420 | 300 | 50,00 | 50,0 | 50,00 | 35,36 | 35,36 | 11,79 | 9,76 |
| $11,3 \times 460$ | 20 | 460 | 320 | 50,00 | 50,00 | 50,00 | 35,36 | 35,36 | 11,79 | 9,76 |
| 11,3x500 | 20 | 500 | 360 | 50,0 | 50,0 | 50,0 | 35,36 | 35,36 | 11,79 | 9,76 |
| 11,3x540 | 20 | 540 | 380 | 50,0 | 50,00 | 50,0 | 35,36 | 35,36 | 11,79 | 9,76 |
| 11,3x580 | 20 | 580 | 420 | 50,0 | 50,0 | 50,0 | 35,36 | 35,36 | 11,79 | 9,76 |
| $11,3 \times 620$ | 20 | 620 | 440 | 50,00 | 50,00 | 50,00 | 35,36 | 35,36 | 11,79 | 9,76 |
| $11,3 \times 660$ | 20 | 660 | 460 | 50,00 | 50,00 | 50,00 | 35,36 | 35,36 | 11,79 | 9,76 |
| $11,3 \times 700$ | 20 | 700 | 500 | 50,00 | 50,00 | 50,00 | 35,36 | 35,36 | 11,79 | 9,76 |
| $11,3 \times 750$ | 20 | 740 | 540 | 50,00 | 50,00 | 50,00 | 35,36 | 35,36 | 11,79 | 9,76 |
| $11,3 \times 800$ | 20 | 800 | 560 | 50,00 | 50,00 | 50,00 | 35,36 | 35,36 | 11,79 | 9,76 |
| 11,3x900 | 20 | 900 | 640 | 50,00 | 50,00 | 50,00 | 35,36 | 35,36 | 11,79 | 9,76 |
| 11,3x 1000 | 20 | 1000 | 700 | 50,00 | 50,0 | 50,0 | 35,36 | 35,36 | 11,79 | 9,76 |

Calculation according to EAA- $1 / / 0024$. Wood density $\rho \mathrm{pk}=380 \mathrm{~kg} / \mathrm{m}^{3}$. All mechanical values provided should be viewed as subject to the assumptions that have been made and represent example calculations.
All values are calculated minimum values and are subject to typographical and printing errors.
a) The characteristic values of the lood-bearing capacity Rk cannot be treated as equivalent to the max. possible load (the max. force). Characteristic values of the load-bearing capacity Rk should be reduced to dimensioning valves

Rd with regard to the usage class and class of the load duration: $\mathrm{Rd}_{d}=\mathrm{Rk}_{k} \cdot \mathrm{kmod}_{\bmod } / \gamma \mathrm{M}$. The dimensioning values of the lood-bearing capacity $\mathrm{Rd}_{\mathrm{d}}$ should be contrasted with the dimensioning values of the loads ( $\left.\mathrm{R}_{\mathrm{d}} \geq \mathrm{Ed}_{\mathrm{d}}\right)$.
Example:
Characteristic value for constant load (dead weight) $G_{k}=2,00 \mathrm{kN}$ and variable load (e. g. snow load) $Q_{k}=3,00 \mathrm{kN} . \mathrm{kmod}=0,9 . \gamma \mathrm{M}=1,3$.
$\rightarrow$ Dimensioning value of the lood $\mathrm{E}_{\mathrm{d}}=2,00 \cdot 1,35+3,00 \cdot 1,5=7,20 \mathrm{kN}$.
The lood-bearing capacity of the joint is therefore considered to have been demonstrated if $\mathrm{Rd}_{\mathrm{d}} \geq \mathrm{Ed} . \rightarrow \min \mathrm{Rk}=\mathrm{Rd}_{d} \cdot \gamma \mathrm{M} / \mathrm{kmod}$
i.e. the characteristic minimum value is calculated based on: $\min \mathrm{R}_{\mathrm{k}}=\mathrm{R}_{\mathrm{d}} \cdot \gamma_{\mathrm{M}} / \mathrm{kmod}_{\bmod } \rightarrow \mathrm{R}_{\mathrm{k}}=7,20 \mathrm{kN} \cdot 1,3 / 0,9=10,40 \mathrm{kN} \rightarrow$ comparison with table values.

Please note: These are planning cids. Projects must only be calculated by authorised persons.

KONSTRUX ST WITH CYLINDER HEAD AND DRILL POINT 6,5 MM: MAIN/SECONDARY BEAM JOINTS


Calculation according to $\mathrm{ETA}-\mathrm{ll} / \mathrm{OO24}$. Wood density $\rho \mathrm{\rho k}=380 \mathrm{~kg} / \mathrm{m}^{3}$. All mechanical values provided should be viewed as subject to the assumptions that have been made and represent example calculations.
All values are calculated minimum values and are subject to typographical and printing errors.
a) The characteristic valves of the load-bearing capacity Rk cannot be treated as equivalent to the max. possible load (the max. force). Characterisicic values of the load bearing capacity Rk should be reduced to dimensioning values Rd with regard to the usage class and dlass of the load duration: $\mathrm{Rd}=\mathrm{Rk} \cdot \mathrm{Kmod} / \gamma \mathrm{m}$. The dimensioning values of the load- bearing capacity $\operatorname{Rd}$ should be contrasted with the dimensioning values of the loads ( $\mathbb{R d} \geq \mathrm{Ed}$ ).
Example:
Characterisic value for constant load (dead weight) $G_{k}=2,00 \mathrm{kN}$ and variable load (e. g. snow lood) $\mathrm{Qk}_{\mathrm{k}}=3,00 \mathrm{kN}$. $\mathrm{kmod}=0,9 . \gamma \mathrm{M}=1,3$.
$\rightarrow$ Dimensioning valve of the lood $E_{d}=2,00 \cdot 1,35+3,00 \cdot 1,5=7,20 \mathrm{kN}$.
$\rightarrow$ The load-bearing capacity of the joint is therefore considered to have been demonstrated if $\mathrm{Rd} \geq \mathrm{Ed} . \rightarrow \min \mathrm{Rk}=\mathrm{Rd} \cdot \gamma \mathrm{M} / \mathrm{kmod}$
i.e. the characteristic minimum value is calculated based on: $\min \mathrm{R}_{\mathrm{k}}=\mathrm{R}_{d} \cdot \gamma_{\mathrm{M}} / \mathrm{kmod} \rightarrow \mathrm{R}_{k}=7,20 \mathrm{kN} \cdot 1,3 / 0,9=10,40 \mathrm{kN} \rightarrow$ comparison with table values.
b) Estimated with an efficient quantity of pairs of screws: $n$,, .

Please note: These are planning aids. Projects must only be calculated by authorised persons.

## KONSTRUX ST WITH CYLINDER HEAD AND DRILL POINT 8,0 MM: MAIN/ SECONDARY BEAM JOINTS



Characterisic value of the joint's
$\mathrm{a}_{2}=\min .40 \mathrm{~mm}, \mathrm{a}_{2},=\min .24 \mathrm{~mm}, \mathrm{k}=\min .12 \mathrm{~mm}$ loadbearing capacity Ry,k acc. to ETA-II/0024

| $\mathrm{dl} \times \mathrm{L}[\mathrm{mm}]$ | min. WSB [mm] | min. HsB [mm] | $\min . W_{M B}[\mathrm{~mm}]$ | min. $\mathrm{H}_{M B}$ [mm] | $\mathrm{m}[\mathrm{mm}]$ | $\beta^{\circ}$ | $\left.R_{v,}, a^{\prime}, \mathrm{b}\right)$ - [kN] | Pair (n) |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| $8,0 \times 245$ | 80 | 200 | 100 | 200 | 87 | 45 | 16,43 | 1 |
|  | 100 |  |  |  |  |  | 30,66 | 2 |
|  | 140 |  |  |  |  |  | 44,16 | 3 |
|  | 180 |  |  |  |  |  | 57,21 | 4 |
| $8,0 \times 295$ | 80 | 220 | 120 | 220 | 104 | 45 | 17,44 | 1 |
|  | 100 |  |  |  |  |  | 32,55 | 2 |
|  | 140 |  |  |  |  |  | 46,88 | 3 |
|  | 180 |  |  |  |  |  | 60,74 | 4 |
| $8,0 \times 330$ | 80 | 260 | 140 | 260 | 117 | 45 | 17,44 | 1 |
|  | 100 |  |  |  |  |  | 32,55 | 2 |
|  | 140 |  |  |  |  |  | 46,88 | 3 |
|  | 180 |  |  |  |  |  | 60,74 | 4 |
| $8,0 \times 375$ | 80 | 280 | 160 | 280 | 133 | 45 | 17,44 | 1 |
|  | 100 |  |  |  |  |  | 32,55 | 2 |
|  | 140 |  |  |  |  |  | 46,88 | 3 |
|  | 180 |  |  |  |  |  | 60,74 | 4 |
| $8,0 \times 400$ | 80 | 300 | 160 | 300 | 141 | 45 | 17,44 | 1 |
|  | 100 |  |  |  |  |  | 32,55 | 2 |
|  | 140 |  |  |  |  |  | 46,88 | 3 |
|  | 180 |  |  |  |  |  | 60,74 | 4 |
| $8,0 \times 430$ | 80 | 320 | 180 | 320 | 152 | 45 | 17,44 | 1 |
|  | 100 |  |  |  |  |  | 32,55 | 2 |
|  | 140 |  |  |  |  |  | 46,88 | 3 |
|  | 180 |  |  |  |  |  | 60,74 | 4 |
| $8,0 \times 480$ | 80 | 360 | 180 | 360 | 170 | 45 | 17,44 | 1 |
|  | 100 |  |  |  |  |  | 32,55 | 2 |
|  | 140 |  |  |  |  |  | 46,88 | 3 |
|  | 180 |  |  |  |  |  | 60,74 | 4 |

Calculation according to $\mathrm{EAA}-\mathrm{II} / 0024$. Wood density $\mathrm{\rho k}=380 \mathrm{~kg} / \mathrm{m}^{3}$. All mechanical values provided should be viewed as subject to the assumptions that have been made and represent example calculations.
All values are calculated minimum values and are subject to typographical and printing errors.
a) The characteristic valves of the load-bearing capacity Rk cannot be treated as equivalent to the max. possible lood (the max. force). Characteristic values of the load-bearing capacity Rk should be reduced to dimensioning values Rd with regard to the usage class and class of the load duration: $\mathbb{R d}_{d}=\mathbb{R}_{k} \cdot \mathrm{kmod}_{\mathrm{mod}} / \gamma$. The dimensioning values of the load-bearing capacity $\mathbb{R}_{d}$ should be contrasted with the dimensioning values of the loads ( $\left.\mathbb{R}_{d} \geq \mathrm{F}_{d}\right)$.
Example:
Characteristic value for constant load (dead weight) $G_{k}=2,00 \mathrm{kN}$ and variabble load (e. g. snow load) $Q_{k}=3,00 \mathrm{kN} . \mathrm{kmod}=0,9 \cdot \gamma \mathrm{M}=1,3$.
$\rightarrow$ Dimensioning value of the lood $\mathrm{Ed}=2,00 \cdot 1,35+3,00 \cdot 1,5=7,20 \mathrm{kN}$.
The load-bearing capacity of the joint is therefore considered to have been demonstrated if $\mathrm{R}_{\mathrm{d}} \geq \mathrm{Ed}_{\mathrm{d}} \rightarrow \min \mathrm{R}_{\mathrm{k}}=\mathrm{R}_{\mathrm{d}} \cdot \gamma_{\mathrm{M}} / \mathrm{kmod}_{\bmod }$
i.e. the characteristic minimum value is calculated based on: $\min \mathrm{Rk}=\mathrm{Rd}_{\mathrm{d}} \cdot \gamma \mathrm{M} / \mathrm{kmod} \rightarrow \mathrm{Rk}=7,20 \mathrm{kN} \cdot 1,3 / 0,9=10,40 \mathrm{kN} \rightarrow$ comparison with table values.
b) Estimated with on efficient quantity of pairs of screws: $\mathrm{n}^{0}$, .

Please note: These are planning cids. Projects must only be calculated by authorised persons.

KONSTRUX ST WITH CYLINDER HEAD AND DRILL POINT 10,0 MM: MAIN/SECONDARY BEAM JOINTS


Calculation according to $\mathrm{EA}-\mathrm{II} / 0024$. Wood density $\rho_{\mathrm{k}}=380 \mathrm{~kg} / \mathrm{m}^{3}$. All mechanical values provided should be viewed as subject to the assumpions that have been made and represent example calculations.
All values are calculated minimum values and are subject to typoographical and printing errors.
a) The characteristic values of the load-bearing capacity Rk cannot be treated os equivalent to the max. possible load (the max. force). Characterisic values of the load-bearing capacity Rk should be reduced to dimensioning values Rd with regard to the usage class and class of the load duration: $R d=R_{k} \cdot K_{m o d} / \gamma \mathrm{m}$. The dimensioning values of the load-bearing capacity $\operatorname{Rd}$ should be contrasted with the dimensioning values of the loads ( $\left.R_{d} \geq E_{d}\right)$.
Example:
Characterisic value for constant load (dead weight) $G_{k}=2,00 \mathrm{kN}$ and variable load (e. g. snow load) $\mathrm{Qk}_{\mathrm{k}}=3,00 \mathrm{kN}$. $\mathrm{kmod}=0,9 \cdot \gamma \mathrm{M}=1,3$.
$\rightarrow$ Dimensioning value of the lood $\mathrm{Ed}=2,00 \cdot 1,35+3,00 \cdot 1,5=1,20 \mathrm{kN}$.
The load-bearing capacity of the joint is therefore considered to have been demonstrated if $\mathrm{Rd} \geq \mathrm{Ed} . \rightarrow \mathrm{min} \mathrm{Rk}=\mathrm{Rd} \cdot \gamma \mathrm{M} / \mathrm{kmod}$
i.e. the characterisicic minimum value is calculated based on: $\min \mathrm{R}_{k}=\mathrm{R}_{d} \cdot \gamma \mathrm{~m} / \mathrm{k}_{\mathrm{mod}} \rightarrow \mathrm{R}_{k}=7,20 \mathrm{kN} \cdot 1,3 / 0,9=10,40 \mathrm{kN} \rightarrow$ comparison with table values.
b) Estimated with an efficient quantity of pairs of screws: $\mathrm{n} 0,9$.

Please note: These are planning aids. Projects must only be calculated by authorised persons.

KONSTRUX ST WITH CYLINDER HEAD 6,5 MM

GEOMETRY AND MECHANICAL PROPERTIES


| KonstruX ST-CH 06, 5xL -TX30 |  |  |  |  |  |  |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Art. no. | $\begin{gathered} ⿺ \\ {[\mathrm{~mm}]} \end{gathered}$ | Lg, eff <br> [mm] | PU | Pre-drilling diameter $0 \mathrm{~d}_{v}$ [mm] | Characteristic pull-out resistance value $\mathrm{f}_{\mathrm{ax}, \mathrm{k}}\left[\mathrm{N} / \mathrm{mm}^{2}\right]$ | Characteristic tensile strength value fiens,,$[\mathrm{kN}]$ | Characteristic yield moment $M_{y, k}[\mathrm{Nmm}]$ | Characteristic yield strength $f_{y, k}\left[\mathrm{~N} / \mathrm{mm}^{2}\right]$ |
| 904808 | 80 | 71 | 100 | 4,5 | 11,4 | 17,0 | 1500 | 1000 |
| 908809 | 100 | 91 | 100 | 4,5 | 11,4 | 17,0 | 1500 | 1000 |
| 904810 | 120 | 11 | 100 | 4,5 | 11,4 | 17,0 | 1500 | 1000 |
| 908811 | 140 | 131 | 100 | 4,5 | 11,4 | 17,0 | 1500 | 1000 |
| 90812 | 160 | 151 | 100 | 4,5 | 11,4 | 17,0 | 1500 | 1000 |
| 904813 | 195 | 186 | 100 | 4,5 | 11,4 | 17,0 | 1500 | 1000 |

## Axial and edge distances

The minimum distances for KonstruX loaded exclusively in the axial direction in pre-drilled and non-pre-drilled holes in components measuring min. $t=65$ thick and min. 60 mm wide must be selected as follows

| Axial distance parallel to the direction of the grain | al | [mm] | 5.d | 33 |
| :---: | :---: | :---: | :---: | :---: |
| Axial distance perpendicular to the direction of the grain | a2 | [mm] | 5.d | 33 |
| Distance from the centre of gravity of the screw area driven into the wood from the end grain surface | al, 1 | [mm] | 5.d | 33 |
| Distance from the centre of gravity of the screw area driven into the wood from the side grain sufface | 02, 6 | [mm] | 3.d | 20 |
| Axill distance between a crossing pair of screws | a2,k | [mm] | 1,5•d | 10 |
| Reduced axial distance a2 perpendicular to the direction of the grain, if al $\cdot 02 \geq 25 \cdot \mathrm{~d}^{2}$ | a2, red | [mm] | 2,5•d | 16 |

The axial and edge distances are minimum distances according to DIN EN 1995:2014 (EC5) and generally apply to fasteners subjected to transverse loads
al
Distance from the fasteners within a row in the direction of the grain


02
Distance from the fasteners perpendicular to the direction of the grain
a3,
Distance between the fastener and the unloaded end of the end grain $90^{\circ} \leq \alpha \leq 270^{\circ}$


03,1
Distance between the fastener and the loaded end of the end grain $-90^{\circ} \leq \alpha \leq 90^{\circ}$


Eurotec | Constructive fastening

When analysed, the minimum distances for Konstrux screws in predrililed holes that are looded in a crosswise direction
ure as follows according to the position of the direction of the grain

Minimum distances for KonstruX screws in pre-drilled holes that ree loaded in a crosswise direction with a force/fibre angle of $0^{\circ}$ and $90^{\circ}$


|  |  |  | Force/fibre angle $\alpha=0^{\circ}$ |  | Force/fibre angle $\alpha=90^{\circ}$ |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Axial distance parallel to the direction of the grain | al | [mm] | 5.d | 33 | 4.d | 33 |
| Axial distance perpendicular to the direction of the grain | 02 | [mm] | 3.d | 20 | 4.d | 33 |
| Distance from the centre of gravity of the screw area driven into the wood from the unloaded end of the end grain | 03, 6 | [mm] | 7.d | 46 | 7.d | 46 |
| Distance from the centre of gravity of the screw area driven into the wood from the loaded end of the end groin | 03, ${ }^{\text {t }}$ | [mm] | 12.d | 78 | 7.d | 46 |
| Axial distance perpendicular to the unloaded edge | 04, | [mm] | 3.d | 20 | 3.d | 20 |
| Axial distance from the looded dedge | 04, ${ }^{\text {t }}$ | [mm] | 3.d | 20 | 7.d | 46 |

## When analysed, the minimum distances for KonstruX in non-pre-drilled holes, loaded in a crosswise direction, are as follows according to the position of the direction of the grain



KONSTRUX ST WITH CYLINDER HEAD AND DRILL POINT 6,5 MM: SHEARING STRENGTH RATIO WITHOUT PRE-DRILLING

| Dimensions |  |  | Axial pull-out load capacity | Shearing strength ratio without pre-drilling |  |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
|  |  |  |  | $\xrightarrow{V\left(a=0^{\circ}\right)}$ |  | $\xrightarrow{\mathrm{V}\left(\mathrm{a}=0^{\circ}\right)}$ | A |
|  |  |  |  | $V\left(\mathrm{a}=0^{\circ}\right)$ | - ${ }^{\text {® }}$ | $\underline{V\left(a=90^{\circ}\right)}$ | B |
|  |  |  |  | $\underbrace{V\left(a=90^{\circ}\right)}$ |  | $\xrightarrow{\mathrm{V}\left(\mathrm{a}=90^{\circ}\right)}$ | ${ }^{\text {A }}$ |
|  |  |  | Characterisicic value of the joint's loadbearing capacity Rax,k acc. to ETA-11/0024 |  | Characterisicic value of the joint's loadbearing capacity Rk occ. to ETA-II/0024 |  |  |
| Odl x L[mm] | A [mm] | B [mm] | $\mathrm{R}_{\mathrm{ax}, \mathrm{k}} \mathrm{l}^{\text {a) }}$ [ [KN] | $\mathrm{Rk}^{\text {a) }}$-[kN] | $\mathrm{R}_{\mathrm{k}}{ }^{\text {a) }}$-[kN] | $\mathrm{Rk}^{\text {a) }}$-[kN] | $\mathrm{Rk}^{\text {a) }}$-[kN] |
|  |  |  |  | $\alpha=0^{\circ}$ | $\alpha=90^{\circ}$ | $\alpha_{A}=0^{\circ}$ | $\alpha_{A}=90^{\circ}$ |
|  |  |  |  |  |  | $\alpha B=90^{\circ}$ | $\alpha B=0^{\circ}$ |
| 6,5x 120 | 60 | 80 | 4,35 | 3,83 | 3,37 | 3,83 | 3,37 |
| 6,5 140 | 80 | 80 | 4,43 | 3,85 | 3,39 | 3,39 | 3,85 |
| 6,5 5160 | 80 | 100 | 5,94 | 4,22 | 3,76 | 4,22 | 3,76 |
| 6,5× 195 | 100 | 100 | 7,20 | 4,54 | 4,08 | 4,8 | 4,54 |

[^4]
## Gurotec | Constructive fastening

## ANGLE-BRACKET SCREW (ABS)

For quick and easy screwing in

The Eurotec Angle-bracket screw (ABS) is made of hardened carbon steel and is specially designed for joints between steel sheet and wood. The spliting effect in the wood is reduced by the geometry of the screw tip. In addition, the screw is characterized, among other things, by the smooth shank under the head, which allows load transfer during shearing.


- Ensures quick and easy screwing in


Angle-bracket mounted in CLT system angle.

Angle-bracket screw
Blue, galvanised steel

| N | Art. no. | Dimensions [mm] | Drive | PU |
| :---: | :---: | :---: | :---: | :---: |
|  | 945343 | $5,0 \times 25$ | TX20 | 250 |
| [4.11/0084 | 945232 | $5,0 \times 35$ | TX20 | 250 |
|  | 945241 | $5,0 \times 40$ | TX20 | 250 |
|  | 945233 | $5,0 \times 50$ | TX20 | 250 |
|  | 945344 | $5,0 \times 60$ | TX20 | 250 |
|  | 945345 | $5,0 \times 70$ | TX20 | 250 |

## TECHNICAL INFORMATIONS

## ANGLE-BRACKET SCREW, STEEL BLUE GALVANISED

| Dimensions | Extraction resistance | Shearing Steel-Timber |
| :---: | :---: | :---: | :---: |




| $\begin{aligned} & \mathrm{dl} \times \mathrm{L} \\ & {[\mathrm{~mm}]} \end{aligned}$ | $\begin{gathered} d k \\ {[\mathrm{~mm}]} \end{gathered}$ | $\begin{gathered} \lg \\ {[\mathrm{mm}]} \end{gathered}$ | $F_{a x, 90, R k}$ <br> [kN] | $\begin{gathered} t \\ {[\mathrm{~mm}]} \end{gathered}$ | $\begin{gathered} R_{k} \\ {[k N]} \end{gathered}$ | $\begin{gathered} t \\ {[\mathrm{~mm}]} \end{gathered}$ | $R_{k}$ <br> [kN] | $\begin{gathered} \dagger \\ {[\mathrm{mm}]} \end{gathered}$ | $\begin{gathered} R_{k} \\ {[k N]} \end{gathered}$ | $\begin{gathered} t \\ {[\mathrm{~mm}]} \end{gathered}$ | $\begin{gathered} R_{k} \\ {[k N]} \end{gathered}$ | $\begin{gathered} \dagger \\ {[\mathrm{mm}]} \end{gathered}$ | $\begin{gathered} R_{k} \\ {[k N]} \end{gathered}$ |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
|  |  |  | $t \leq 9,0[\mathrm{~mm}]$ |  | $\alpha=0^{\circ}$ |  | $\alpha=0^{\circ}$ |  | $\alpha=0^{\circ}$ |  | $\alpha=0^{\circ}$ |  | $\alpha=0^{\circ}$ |
|  |  |  |  |  | $\alpha=90^{\circ}$ |  | $\alpha=90^{\circ}$ |  | $\alpha=90^{\circ}$ |  | $\alpha=90^{\circ}$ |  | $\alpha=90^{\circ}$ |
| 5, $0 \times 25$ | 7,2 | 16 | 0,97 | 1,5 | 0,89 | 2,0 | 0,87 | 2,5 | 0,85 | 3,0 | 0,\% | 4,0 | 1,18 |
| 5,0 $\times 35$ |  | 26 | 1,57 |  | 1,27 |  | 1,25 |  | 1,23 |  | 1,35 |  | 1,59 |
| 5,0 $\times 40$ |  | 31 | 1,88 |  | 1,46 |  | 1,44 |  | 1,42 |  | 1,55 |  | 1,81 |
| 5,0x50 |  | 41 | 2,48 |  | 1,84 |  | 1,82 |  | 1,80 |  | 1,89 |  | 2,10 |
| 5,0×60 |  | 51 | 3,09 |  | 1,99 |  | 1,99 |  | 1,99 |  | 2,09 |  | 2,29 |
| 5,0x70 |  | 61 | 3,69 |  | 2,14 |  | 2,14 |  | 2,14 |  | 2,24 |  | 2,4 |

[^5]Example:
Characterisicic value for constant load (dead load) $G_{k}=2,00 \mathrm{kN}$ and variable load (e.g. snow load) $Q_{k}=3,00 \mathrm{kN} . \mathrm{kmod}=0,9 \cdot \gamma \mathrm{M}=1,3$.
$\rightarrow$ Rated value of the load $\mathrm{E}_{\mathrm{d}}=2,00 \cdot 1,35+3,00 \cdot 1,5=7,20 \mathrm{kN}$.
Lood-bearing capacity of the connection is proved if $\mathrm{Rd} \geq \mathrm{Ed} . \rightarrow \mathrm{min} \mathrm{R}_{\mathrm{k}}=\mathrm{R}_{d} \cdot \gamma_{\mathrm{M}} / \mathrm{kmod}$
That is, the characteristic minimum valve of the lood-bearing capacity is calculated as: $\min R_{k}=R_{d} \cdot \gamma \mathrm{M} / \mathrm{kmod}_{\mathrm{mod}} \rightarrow \mathrm{R}_{\mathrm{k}}=7,20 \mathrm{kN} \cdot 1,3 / 0,9=10,40 \mathrm{kN} \rightarrow$ Aligned with table values.
Please note: These are planning aids. Projects must only be calculated by authorised persons.
Please note: Verify the assumptions made. The stated values, and type and number of joining devices are based on preliminary measurements. Projects are to be dimensioned exclusively by cuthorised persons in accordance with the State Building Code. As per LBuoO, please contact a qualified structural engineer for a poid proof of sability. We will be happy to refer you to someone.

## PANELTWISTEC

Paneltwistec wood construction screws may generally be installed in CLT without predrilling. The Paneltwistec is a wood construction screw with a special screw tip and milling ribs above the thread. The cutting notch on the screw tip ensures fast gripping and less splitting effect when screwing in. The Paneltwistec AG instead features a folded-down thread, which reduces the screw-in torque. Paneltwistec wood construction screws are available in both countersunk head and Washer head variants, as well as made of coated carbon steels and various stainless steels.


## PANELTWISTEC AG

Blue galvanised

Paneltwistec AG
Countersunk head, blue galvanised


## ADVANTAGES

- Quick and easy screwing-in
- Reduced spliting effect
- National and international approvals
- Free of chromium (VI) oxide
- No hammering of the screws when screwing in due to TX-Drive

| 空: | Art. no. | Dimensions [mm] | Drive | PU |
| :---: | :---: | :---: | :---: | :---: |
|  | 945436 | 3,5×30 | TX15 | 1000 |
|  | 945838 | 3,5×35 | TX15 | 1000 |
|  | 945437 | 3,5440 | TX15 | 1000 |
|  | 945490 | 3,5 $\times 50$ | TX15 | 500 |
|  | 945491 | 4,0 $\times 30$ | TX20 | 1000 |
|  | 945836 | 4,0×35 | TX20 | 1000 |
|  | 945492 | 4,0 40 | TX20 | 1000 |
|  | 945493 | 4,0x 45 | TX20 | 500 |
|  | 945494 | 4,0 $\times 50$ | TX20 | 500 |
|  | 945495 | 4,0x60 | TX20 | 200 |
|  | 945446 | 4,0×70 | TX20 | 200 |
|  | 945497 | 4,0x80 | TX20 | 200 |
|  | 945498 | 4,5 $\times 40$ | TX25 | 500 |
|  | 945588 | 4,5 45 | TX25 | 500 |
|  | 945499 | 4,5 $\times 50$ | TX25 | 500 |
|  | 945567 | 4,5660 | TX25 | 200 |
|  | 94568 | 4,5 $\times 70$ | TX25 | 200 |
|  | 94556 | 4,5 80 | TX25 | 200 |
|  | 945574 | $5,0 \times 40$ | TX25 | 200 |
|  | 945837 | $5,0 \times 45$ | TX25 | 200 |
|  | 94575 | 5,0x50 | TX25 | 200 |
|  | 945576 | $5,0 \times 60$ | TX25 | 200 |
|  | 945577 | 5,0×70 | TX25 | 200 |
|  | 945578 | 5,0x80 | TX25 | 200 |
|  | 945579 | 5,0×90 | TX25 | 200 |
|  | 945580 | 5,0× 100 | TX25 | 200 |
|  | 945581 | $5,0 \times 120$ | TX25 | 200 |
|  | 94583 | 6,0×60 | TX30 | 200 |
|  | 945584 | 6,0×70 | TX30 | 200 |
|  | 945632 | 6,0x80 | TX30 | 200 |
|  | 945633 | $6,0 \times 90$ | TX30 | 100 |
|  | 945634 | 6,0 $\times 100$ | TX30 | 100 |
|  | 945635 | $6,0 \times 110$ | TX30 | 100 |
|  | 945636 | $6,0 \times 120$ | TX30 | 100 |
|  | 945637 | 6,0x 130 | TX30 | 100 |
|  | 945638 | 6,0x 140 | TX30 | 100 |
|  | 945639 | 6,0x 150 | TX30 | 100 |
|  | 945640 | $6,0 \times 160$ | TX30 | 100 |
|  | 945641 | 6,0×180 | TX30 | 100 |
|  | 945642 | $6,0 \times 200$ | TX30 | 100 |
|  | 945643 | 6,0x 220 | TX30 | 100 |
|  | 945644 | 6,0x 240 | TX30 | 100 |
|  | 945645 | 6,0×260 | TX30 | 100 |
|  | 945646 | 6,0x 280 | TX30 | 100 |
|  | 945647 | 6,0x 300 | TX30 | 100 |

Paneltwistec AG
Countersunk head, blue galvanised

ADVANTAGES

- Quick and easy screwing-in
- Reduced splitting effect
- National and international approvals
- Free of chromium (VI) oxide
- No hammering of the screws when screwing
in due to TX-Drive

| Art. no. | Dimensions [mm] | Drive | PU |
| :---: | :---: | :---: | :---: |
| 944715 | $8,0 \times 80$ | TX40 • | 50 |
| 944716 | 8,0 $\times 100$ | TX40 - | 50 |
| 944717 | 8,0 $\times 120$ | TX40 - | 50 |
| 944718 | $8,0 \times 140$ | TX40 - | 50 |
| 944719 | $8,0 \times 160$ | TX40 - | 50 |
| 944720 | 8,0x 180 | TX40 - | 50 |
| 944721 | $8,0 \times 200$ | TX40 - | 50 |
| 944722 | 8,0 $\times 220$ | TX40 - | 50 |
| 944723 | 8,0 $\times 240$ | TX40 - | 50 |
| 944724 | 8,0 $\times 260$ | TX40 - | 50 |
| 944725 | 8,0 $\times 280$ | TX40 - | 50 |
| 944726 | 8,00300 | TX40 - | 50 |
| 944727 | 8,0x 320 | TX40 - | 50 |
| 944728 | 8,0x 340 | TX40 - | 50 |
| 944729 | 8,0 $\times 360$ | TX40 - | 50 |
| 944730 | 8,0 $\times 380$ | TX40 - | 50 |
| 944731 | 8,0 0400 | TX40 - | 50 |
| 944732 | 8,0x 420 | TX40 - | 25 |
| 944733 | 8,0x440 | TX40 - | 25 |
| 944734 | 8,0 $\times 460$ | TX40 - | 25 |
| 944735 | 8,0x 480 | TX40 - | 25 |
| 944736 | 8,0x 500 | TX40 - | 25 |
| 944737 | $8,0 \times 550$ | TX40 - | 25 |
| 944739 | 8,00600 | TX40 - | 25 |
| 945687 | $10 \times 100$ | TX50 • | 50 |
| 945688 | $10 \times 120$ | TX50 • | 50 |
| 945689 | $10 \times 140$ | TX50 • | 50 |
| 945690 | $10 \times 160$ | TX50 • | 50 |
| 945691 | $10 \times 180$ | TX50 • | 50 |
| 945692 | $10 \times 200$ | TX50 • | 50 |
| 945693 | $10 \times 220$ | TX50 • | 50 |
| 945694 | $10 \times 240$ | TX50 • | 50 |
| 945695 | $10 \times 260$ | TX50 • | 50 |
| 945696 | $10 \times 280$ | TX50 • | 50 |
| 945697 | $10 \times 300$ | TX50 • | 50 |
| 945698 | $10 \times 320$ | TX50 • | 50 |
| 945699 | $10 \times 340$ | TX50 • | 50 |
| 945703 | $10 \times 360$ | TX50 • | 50 |
| 945709 | $10 \times 380$ | TX50 • | 50 |
| 945711 | $10 \times 400$ | TX50 • | 50 |



## TECHNICAL INFORMATION

PANELTWISTEC AG, COUNTERSUNK-HEAD, BLUE GALVANISED


Calculation according to $\mathrm{ETA}-\mathrm{II} / 0024$. Wood density $\rho \mathrm{k}=350 \mathrm{~kg} / \mathrm{m}^{3}$. All mechanical values provided should be viewed as subject to the assumptions that have been made and represent example calculations.
All values are calculated minimum values and are subject to typographical and printing errors.
a) The characterisicic values of the lood-bearing capacity Rk cannot be trected as equivalent to the max. possible load (the max. force). Characterisisic values of the load-bearing capacity Rk should be reduced to dimensioning values
$R_{d}$ with regard to the usage class and class of the load duration: $R_{d}=R_{k} \cdot k_{m o d} / \gamma M$. The dimensioning values of the load-bearing capacity $R_{d}$ should be contrasted with the dimensioning values of the loads $\left(R_{d} \geq E_{d}\right)$.
Example:
Characterisic valve for constant load (dead weight) $G_{k}=2,00 \mathrm{kN}$ and variable load (e. g. snow lood) $Q_{k}=3,00 \mathrm{kN} . \mathrm{kmod}=0,9 \cdot \gamma \mathrm{M}=1,3$.
$\rightarrow$ Dimensioning value of the lood $\mathrm{E} d=2,00 \cdot 1,35+3,00 \cdot 1,5=7,20 \mathrm{kN}$.
The load-bearing capacity of the joint is therefore considered to have been demonstrated if $R_{d} \geq \mathrm{E}_{\mathrm{d}} \rightarrow \min \mathrm{R}_{k}=\mathrm{R}_{\mathrm{d}} \cdot \gamma_{M} / \mathrm{kmod}_{\text {mod }}$
I.e. the characteristic minimum value is calculated based on: $\min \mathrm{R}_{\mathrm{k}}=\mathrm{R}_{\mathrm{d}} \cdot \gamma \mathrm{M} / \mathrm{kmod}_{\bmod } \rightarrow \mathrm{R}_{\mathrm{k}}=7,20 \mathrm{kN} \cdot 1,3 / 0,9=10,40 \mathrm{kN} \rightarrow$ comparison with table values.

Please note: These are planning dids. Projects must only be calculated by authorised persons.
Please note: Verify the assumptions made. The stated values, and type and number of joining devices are based on preliminary measurements. Projects are to be dimensioned exclusively by authorised
persons in accordance with the State Building Code. As per LBauO, please contact a qualified structural engineer for a paid proof of stability. We will be happy to refer you to someone.


Calculation according to $\mathrm{EA}-11 / 0024$. Wood density $\rho_{\mathrm{k}}=350 \mathrm{~kg} / \mathrm{m}^{3}$. All mechanical values provided should be viewed as subject to the assumptions that have been made and represent example calculations.
All values are calculated minimum values and are subject to typographical and printing errors.
a) The characterisic values of the load-bearing capacity Rk cannot be treated as equivalent to the max. possible load (the max. force). Characteristic values of the load-bearing capacity Rk should be reduced to dimensioning values
$R_{d}$ with regard to the usage class and dlass of the load duration: $R_{d}=R_{k} \cdot k_{m o d} / \gamma M$. The dimensioning values of the load-bearing capacity $R_{d}$ should be contrasted with the dimensioning values of the loads $\left(R_{d} \geq E_{d}\right)$.
Example:
Characterisic value for constant load (dead weight) $G_{k}=2,00 \mathrm{kN}$ and variable load (e. g. snow lood) $Q_{k}=3,00 \mathrm{kN} . \mathrm{kmod}=0,9 . \gamma \mathrm{M}=1,3$.
$\rightarrow$ Dimensioning value of the lood $E d=2,00 \cdot 1,35+3,00 \cdot 1,5=7,20 \mathrm{kN}$.
The load-bearing capacity of the joint is therefore considered to have been demonstrated if $\mathrm{R}_{\mathrm{d}} \geq \mathrm{Ed} . \rightarrow \mathrm{min} \mathrm{Rk}_{\mathrm{k}}=\mathrm{Rd}_{d} \cdot \gamma_{\mathrm{M}} / \mathrm{kmod}$
I.e. the characteristic minimum value is calculcted based on: $\min \mathrm{R}_{\mathrm{k}}=\mathrm{R}_{d} \cdot \gamma_{\mathrm{M}} / \mathrm{kmod} \rightarrow \mathrm{R}_{\mathrm{k}}=7,20 \mathrm{kN} \cdot 1,3 / 0,9=10,40 \mathrm{kN} \rightarrow$ comparison with table values.

Please note: These are planning ciids. Proiects must only be calculated by authorised persons.


Calculation according to $\mathrm{EA}-\mathrm{II} / 0024$. Wood density $\rho_{\mathrm{k}}=350 \mathrm{~kg} / \mathrm{m}^{3}$. All mechanical values provided should be viewed as subject to the assumptions that have been made and represent example calculations.
All values are calculated minimum values and are subject to typographical and printing errors.
a) The characteristic values of the lood-bearing capacity Rk cannot be treated as equivalent to the max. possible load (the max. force). Characterisicic values of the lood-bearing capacity Pk should be reduced to dimensioning values
$R_{d}$ with regard to the usage class and class of the load duration: $R_{d}=R_{k} \cdot k_{m o d} / \gamma M$. The dimensioning values of the load-bearing capacity $R d$ should be contrasted with the dimensioning values of the loads ( $\left.R_{d} \geq E_{d}\right)$.
Example:
Characterisic value for constant load (dead weight) $G_{k}=2,00 \mathrm{kN}$ and variable load (e. g. snow load) $Q_{k}=3,00 \mathrm{kN} . \mathrm{kmod}=0,9 \cdot \gamma \mathrm{M}=1,3$.
$\rightarrow$ Dimensioning value of the load $\mathrm{E}_{\mathrm{d}}=2,00 \cdot 1,35+3,00 \cdot 1,5=7,20 \mathrm{kN}$.
The load-bearing capacity of the joint is therefore considered to have been demonstrated if $\mathrm{Rd} \geq \mathrm{Ed}_{\mathrm{d}} \rightarrow \mathrm{min} \mathrm{Rk}=\mathrm{Rd}_{d} \cdot \gamma \mathrm{M} / \mathrm{kmod}$
I.e. the characterisicic minimum value is calculated based on: $\min \mathrm{R}_{\mathrm{k}}=\mathrm{R}_{d} \cdot \gamma_{\mathrm{M}} / \mathrm{kmod}_{\bmod } \rightarrow \mathrm{R}_{\mathrm{k}}=7,20 \mathrm{kN} \cdot 1,9 / 0,9=10,40 \mathrm{kN} \rightarrow$ comparison with table values.

Please note: These are planning dids. Projects must only be calculated by authorised persons.
Please note: Verify the assumptions made. The stated values, and type and number of joining devices are based on preliminary measurements. Projects are to be dimensioned exclusively by authorised
persons in accordance with the State Building Code. As per LBuuO, please contact a qualified structural engineer for a paid proof of stability. We will be happy to refer you to someone.

## Gurotec <br> Constructive fastening



| Paneltwistec AG | Art. no. | Dimensions [mm] | Drive | PU |
| :---: | :---: | :---: | :---: | :---: |
| Washer head screw, blue galvanised | 945806 | $8,0 \times 60$ | TX40 - | 50 |
|  | 944588 | $8,0 \times 80$ | Tx40 - | 50 |
| T | 94458 | 8,0× 100 | TX40 | 50 |
|  | 94459 | $8,0 \times 120$ | TX40 - | 50 |
|  | 94459 | $8,0 \times 140$ | TX40 - | 50 |
|  | 94592 | $8,0 \times 160$ | TX40 - | 50 |
|  | 944593 | 8,0×180 | Tx40 | 50 |
|  | 94459 | 8,0×200 | TX40 | 50 |
|  | 944595 | 8,0×220 | TX40 | 50 |
| 0 | 94459\% | 8,0 240 | TX40 | 50 |
| 8 | 94459 | 8,0 $\times 260$ | TX40 | 50 |
| \% | 944598 | $8,0 \times 280$ | TX40 - | 50 |
| + | 94459 | $8,0 \times 300$ | TX40 - | 50 |
| H | 94460 | 8,0×320 | TX40 - | 50 |
| 者 | 944601 | 8,0×340 | TX40 | 50 |
| 早 | 94460 | 8,0x 360 | TX40 | 50 |
| \# | 94603 | 8,0×380 | TX40 | 50 |
| 7 | 94604 | 8,0x400 | TX40 | 50 |
| 1 | 94665 | $8,0 \times 420$ | TX40 - | 25 |
| 1 | 944606 | 8,0x 440 | TX40 | 25 |
|  | 944607 | 8,0x460 | TX40 | 25 |
| ADVANTAGES | 94608 | 8,0x480 | TX40 | 25 |
| - The larger head diameter allows for considerably higher torqu | 944609 | 8,0×500 | TX40 ${ }^{\circ}$ | 25 |
| and head pull-through capacity | 944610 | 8,0x550 | TX40 - | 25 |
| - This makes for better use of the screw's tensile load-bearing strength | 944611 | 8,0×600 | TX40 - | 25 |

Paneltwistec AG
Washer head screw, blue galvanised

| 㴍 |  |
| :---: | :---: |
| $\cdots$ | H/204 |



| Art. no. | Dimensions [mm] | Drive | PU |
| :---: | :---: | :---: | :---: |
| 945750 | 10x80 | TX50 - | 50 |
| 945751 | $10 \times 100$ | TX50 - | 50 |
| 945752 | $10 \times 120$ | TX50 | 50 |
| 945753 | $10 \times 140$ | TX50 - | 50 |
| 945754 | $10 \times 160$ | TX50 - | 50 |
| 945755 | $10 \times 180$ | TX50 - | 50 |
| 945756 | $10 \times 200$ | TX50 - | 50 |
| 945757 | $10 \times 220$ | TX50 - | 50 |
| 945758 | 10x 240 | TX50 - | 50 |
| 945759 | $10 \times 260$ | TX50 - | 50 |
| 945760 | $10 \times 280$ | TX50 - | 50 |
| 945761 | $10 \times 300$ | TX50 - | 50 |
| 945762 | $10 \times 320$ | TX50 - | 50 |
| 945763 | $10 \times 340$ | TX50 | 50 |
| 945764 | $10 \times 360$ | TX50 - | 50 |
| 945765 | $10 \times 380$ | TX50 - | 50 |
| 945766 | 10x 400 | TX50 - | 50 |

## ADVANTAGES

The larger head diameter allows for considerably higher
torque and head pull-hrough capacity
This makes for better use of the screw's tensile load-bearing strength


## PANELTWISTEC AG, WASHER HEAD, BLUE GALVANISED

 Eviop. Vodm BowerhugETA-11/0024

| Dimensions |  |  |  | Extraction resistance | Head pull-through resistance | Wood-Wood shearing |  |  |  | Steel-Wood shearing |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
|  |  |  |  |  |  | $V\left(\alpha=0^{\circ}\right)$ <br> $V\left(\alpha=0^{\circ}\right)$ <br> $V\left(\alpha=0^{\circ}\right)$ <br> $V\left(\alpha=90^{\circ}\right)$ |  |  | $\frac{\sqrt{7})}{\sqrt{5})}$ |  |  |
| $\begin{aligned} & \mathrm{dl} \times \mathrm{L} \\ & {[\mathrm{~mm}]} \end{aligned}$ | $\begin{gathered} \mathrm{dk} \\ {[\mathrm{~mm}]} \end{gathered}$ | $\begin{gathered} \text { AD } \\ {[\mathrm{mm}]} \end{gathered}$ | $\begin{gathered} \mathrm{El} \\ {[\mathrm{~mm}]} \end{gathered}$ | $\mathrm{Fax}_{, ~ 90, R k}$ <br> [kN] | $\begin{gathered} \text { Fax_head, Rk }_{\text {[kN] }} \end{gathered}$ | Fla,Rk <br> [kN] | $\begin{array}{ll}\text { Fla,Rk } & \text { Fla,Rk } \\ {[\mathrm{kN}]} & {[\mathrm{kN}]}\end{array}$ | Fla,Rk <br> [kN] | $\begin{gathered} \dagger \\ {[\mathrm{mm}]} \end{gathered}$ | $\begin{aligned} & \text { Fla,Rk } \\ & {[\mathrm{kN}]} \end{aligned}$ | $\begin{aligned} & \text { Fla,Rk } \\ & {[\mathrm{kN}]} \end{aligned}$ |
|  |  |  |  |  |  |  | $\alpha A D=0^{\circ}$ | $\begin{gathered} \alpha A D= \\ 90^{\circ} \end{gathered}$ |  |  |  |
|  |  |  |  |  |  | $\alpha=0^{\circ}$ | $\alpha=90^{\circ} \quad \alpha E T=90^{\circ}$ | $\begin{gathered} \alpha E I= \\ 0 \end{gathered}$ |  | $\alpha=0^{\circ}$ | $\alpha=90^{\circ}$ |
| $4,0 \times 40$ | 10,0 | 16 | 24 | 1,24 | 1,20 |  | 0,95 |  | 2 |  | 1,15 |
| $4,0 \times 50$ | 10,0 | 20 | 30 | 1,55 | 1,20 |  | 1,03 |  | 2 |  | 1,23 |
| $4,0 \times 60$ | 10,0 | 24 | 36 | 1,86 | 1,20 |  | 1,12 |  | 2 |  | 1,31 |
| $4,5 \times 50$ | 11,0 | 20 | 30 | 1,69 | 1,45 |  | 1,20 |  | 2 |  | 1,44 |
| $4,5 \times 60$ | 11,0 | 24 | 36 | 2,03 | 1,45 |  | 1,29 |  | 2 |  | 1,53 |
| $4,5 \times 70$ | 11,0 | 28 | 42 | 2,36 | 1,45 |  | 1,38 |  | 2 |  | 1,61 |
| 5,0×50 | 12,0 | 20 | 30 | 1,82 | 1,73 |  | 1,37 |  | 2 |  | 1,67 |
| $5,0 \times 60$ | 12,0 | 24 | 36 | 2,18 | 1,73 |  | 1,47 |  | 2 |  | 1,76 |
| $5,0 \times 70$ | 12,0 | 28 | 42 | 2,54 | 1,73 |  | 1,57 |  | 2 |  | 1,85 |
| $5,0 \times 80$ | 12,0 | 32 | 48 | 2,90 | 1,73 |  | 1,65 |  | 2 |  | 1,94 |
| 5,0 100 | 12,0 | 40 | 60 | 3,63 | 1,73 |  | 1,65 |  | 2 |  | 2,12 |
| $6,0 \times 30$ | 14,0 | 6 | 24 | 1,64 | 2,35 |  | 0,65 |  | 2 |  | 1,20 |
| $6,0 \times 40$ | 14,0 | 16 | 24 | 1,64 | 2,35 |  | 1,33 |  | 2 |  | 1,63 |
| $6,0 \times 50$ | 14,0 | 20 | 30 | 2,05 | 2,35 |  | 1,66 |  | 2 |  | 2,06 |
| 6,0×60 | 14,0 | 24 | 36 | 2,46 | 2,35 |  | 1,87 |  | 2 |  | 2,26 |
| $6,0 \times 70$ | 14,0 | 28 | 42 | 2,87 | 2,35 |  | 1,97 |  | 2 |  | 2,36 |
| 6,0×80 | 14,0 | 32 | 48 | 3,28 | 2,35 |  | 2,09 |  | 2 |  | 2,46 |
| 6,0×90 | 14,0 | 36 | 54 | 3,69 | 2,35 |  | 2,21 |  | 2 |  | 2,57 |
| $6,0 \times 100$ | 14,0 | 40 | 60 | 4,10 | 2,35 |  | 2,23 |  | 2 |  | 2,67 |
| $6,0 \times 110$ | 14,0 | 44 | 66 | 4,79 | 2,35 |  | 2,23 |  | 2 |  | 2,71 |
| $6,0 \times 120$ | 14,0 | 50 | 70 | 4,79 | 2,35 |  | 2,23 |  | 2 |  | 2,84 |
| $6,0 \times 130$ | 14,0 | 60 | 70 | 4,79 | 2,35 |  | 2,23 |  | 2 |  | 2,84 |
| $6,0 \times 140$ | 14,0 | 70 | 70 | 4,79 | 2,35 |  | 2,23 |  | 2 |  | 2,84 |
| $6,0 \times 150$ | 14,0 | 80 | 70 | 4,79 | 2,35 |  | 2,23 |  | 2 |  | 2,84 |
| $6,0 \times 160$ | 14,0 | 90 | 70 | 4,79 | 2,35 |  | 2,23 |  | 2 |  | 2,84 |
| 6,0 0180 | 14,0 | 110 | 70 | 4,79 | 2,35 |  | 2,23 |  | 2 |  | 2,84 |
| $6,0 \times 200$ | 14,0 | 130 | 70 | 4,79 | 2,35 |  | 2,23 |  | 2 |  | 2,84 |
| $6,0 \times 220$ | 14,0 | 150 | 70 | 4,79 | 2,35 |  | 2,23 |  | 2 |  | 2,84 |
| 6,0 $\times 240$ | 14,0 | 170 | 70 | 4,79 | 2,35 |  | 2,23 |  | 2 |  | 2,84 |
| $6,0 \times 260$ | 14,0 | 190 | 70 | 4,79 | 2,35 |  | 2,23 |  | 2 |  | 2,84 |
| $6,0 \times 280$ | 14,0 | 210 | 70 | 4,79 | 2,35 |  | 2,23 |  | 2 |  | 2,84 |
| $6,0 \times 300$ | 14,0 | 230 | 70 | 4,79 | 2,35 |  | 2,23 |  | 2 |  | 2,84 |

Calculation according to $\mathrm{ETA}-\mathrm{II} / 0024$. Wood density $\rho_{\mathrm{k}}=350 \mathrm{~kg} / \mathrm{m}^{3}$. All mechanical values provided should be viewed as subject to the assumptions that have been made and represent example calculations.
All values are calculated minimum values and are subject to typographical and printing errors.
a) The characteristic values of the load-bearing capacity Rk cannot be treated as equivalent to the max. possible load (the max. force). Characterisicic values of the lood-bearing capacity Rk should be reduced to dimensioning values Rd
with regard to the usage class and class of the load duration: $\mathrm{Rd}=\mathrm{Rk} \cdot \mathrm{Kmod} / \gamma \mathrm{m}$. The dimensioning values of the load-bearing capacity Rd should be contrasted with the dimensioning values of the loads (Rd $\geq \mathrm{Ed})$.
Example:
Characterisic value for constant lood (dead weight) $G_{k}=2,00 \mathrm{kN}$ and variable load (e. g. snow lood) $Q_{k}=3,00 \mathrm{kN} . \mathrm{kmod}^{2}=0,9 . \gamma \mathrm{m}=1,3$.
$\rightarrow$ Dimensioning value of the lood $E d=2,00 \cdot 1,35+3,00 \cdot 1,5=7,20 \mathrm{kN}$.
The load-bearing capacity of the joint is therefore considered to have been demonstrated if $\mathrm{R}_{\mathrm{d}} \geq \mathrm{Ed}_{\mathrm{d}} \rightarrow \min \mathrm{R}_{\mathrm{k}}=\mathrm{Rd}_{\mathrm{d}} \cdot \gamma \mathrm{M} / \mathrm{kmod}$
I.e. the characteristic minimum value is calculcted based on: $\min \mathrm{R}_{\mathrm{k}}=\mathrm{R}_{d} \cdot \gamma_{\mathrm{M}} / \mathrm{kmod}_{\bmod } \rightarrow \mathrm{R}_{\mathrm{k}}=7,20 \mathrm{kN} \cdot 1,3 / 0,9=10,40 \mathrm{kN} \rightarrow$ comparison with table values.

Please note: These are planning aids. Projects must only be calculated by authorised persons.


Calculation according to EA- $\mathrm{II} / 0024$. Wood density $\rho \mathrm{pk}=350 \mathrm{~kg} / \mathrm{m}^{3}$. All mechanical values provided should be viewed as subject to the assumpions that have been made and represent example calculations.
All values are calculated minimum values and are subject to typographical and printing errors.
a) The characterisic values of the lood-bearing capacity Rk cannot be treated as equivalent to the max. possible lood (the max. force). Characterisicic values of the load-bearing capacity Rk should be reduced to dimensioning values Rd with regard to the usage class and class of the load duration: $R_{d}=R_{k} \cdot k_{m o d} / \gamma$. The dimensioning values of the load-bearing capacity $R_{d}$ should be contrasted with the dimensioning values of the loods $\left(R_{d} \geq E_{d}\right)$.

## Example:

Characterisic value for constant lood (dead weight) $G_{k}=2,00 \mathrm{kN}$ and variabbe lood (e. g. snow load) $Q_{k}=3,00 \mathrm{kN} . \mathrm{kmod}=0,9 . \gamma_{\mathrm{M}}=1,3$.
$\rightarrow$ Dimensioning value of the load $\mathrm{Ed}=2,00 \cdot 1,35+3,00 \cdot 1,5=7,20 \mathrm{kN}$.
The lood-bearing capacity of the joint is therefore considered to have been demonstrated if $R_{d} \geq E_{d} . \rightarrow \min R_{k}=R_{d} \cdot \gamma_{M} / k_{\text {mod }}$
l.e. the characteristic minimum value is calculated based on: $\min \mathrm{Rk}=\mathrm{Rd} \cdot \gamma_{\mathrm{M}} / \mathrm{kmod} \rightarrow \mathrm{Rk}_{\mathrm{k}}=7,20 \mathrm{kN} \cdot 1,3 / 0,9=10,40 \mathrm{kN} \rightarrow$ comparison with table values.

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Please note: Verify the assumptions made. The stated values, and type and number of joining devices are based on preliminary meassurements. Projects are to be dimensioned exclusively by authorised persons in accordance with the State Building Code. As per LBauO, please contact a qualified structural engineer for a paid proof of stability. We will be happy to refer you to someone.


Calculation according to $\mathrm{EA}-\mathrm{II} / \mathrm{OO24}$. Wood density $\rho_{\mathrm{k}}=350 \mathrm{~kg} / \mathrm{m}^{3}$. All mechanical values provided should be viewed as subject to the assumptions that have been made and represent example calculations.
All values are calculated minimum values and are subject to typographical and printing errors.
a) The characteristic values of the lood-bearing capacity Rk cannot be treated as equivalent to the max. possible load (the max. force). Characteristic values of the load-bearing capacity Rk should be reduced to dimensioning values Rd
with regard to the usage class and class of the lood duration: $R_{d}=R_{k} \cdot \mathrm{~K}_{\text {mod }} / \gamma \mathrm{M}$. The dimensioning values of the load-bearing capacity $\mathbb{R}_{d}$ should be contrassed with the dimensioning values of the loads ( $\left.\mathbb{R}_{d} \geq \mathrm{E}_{\mathrm{d}}\right)$.
Example:
Characterisic value for constant load (dead weight) $G_{k}=2,00 \mathrm{kN}$ and variable lood (e. g. snow load) $Q_{k}=3,00 \mathrm{kN} . \mathrm{kmod}=0,9 . \gamma \mathrm{M}=1,3$.
$\rightarrow$ Dimensioning value of the load $\mathrm{E}_{\mathrm{d}}=2,00 \cdot 1,35+3,00 \cdot 1,5=7,20 \mathrm{kN}$.
The load-bearing capacity of the joint is therefore considered to have been demonstrated if $\mathrm{R}_{\mathrm{d}} \geq \mathrm{E}_{\mathrm{d}} \rightarrow \min \mathrm{R}_{\mathrm{k}}=\mathrm{R}_{\mathrm{d}} \cdot \gamma \mathrm{M} / \mathrm{kmod}_{\mathrm{mod}}$
I.e. the characteristic minimum valve is calculated based on: $\min \mathrm{R}_{k}=\mathrm{R}_{d} \cdot \gamma_{\mathrm{M}} / \mathrm{kmod}_{\bmod } \rightarrow \mathrm{R}_{\mathrm{k}}=7,20 \mathrm{kN} \cdot 1,3 / 0,9=10,40 \mathrm{kN} \rightarrow$ comparison with table values.

Please note: These are planning aids. Projects must only be calculcted by authorised persons.

Please note: Verify the assumptions made. The stated values, and type and number of joining devices are based on preliminary measurements. Projects are to be dimensioned exclusively by authorised persons in accordance with the State Building Code. As per LBauO, please contact a qualified structural engineer for a paid proof of stability. We will be happy to refer you to someone.

## PANELTWISTEC

Hardened stainless steel


| Paneltwistec | Art．no． | Dimensions［mm］ | Drive | PU |
| :---: | :---: | :---: | :---: | :---: |
| Countersunk－head，scrape point， | 90474 | 4，0 40 | TX20 | 500 |
| hardened stainless steel | 904475 | 4，0 45 | TX20 | 500 |
|  | 904776 | 4，0 050 | TX20 | 500 |
|  | 90477 | $4,0 \times 60$ | TX20 | 500 |
| fret | 904478 | 4，5 45 | TX20 | 200 |
| Nrer | 904479 | 4，5 5 50 | TX20 | 200 |
|  | 904480 | 4，5 560 | TX20 | 200 |
|  | 90481 | 4，5x70 | TX20 | 200 |
| ， | 100981 | 4，5 580 | T220 | 200 |
| 1 | 904882 | 5，0 50 | TX25 | 200 |
| 最 | 90488 | 5，0×60 | TX25 | 200 |
| f | 90488 | $5,0 \times 70$ | TX25 | 200 |
| d | 90485 | 5，0 $\times 80$ | TX25 | 200 |
| 厚 | 90488 | $5,0 \times 90$ | TX25 | 100 |
| C | 904011 | 5，0×100 | TX25 | 100 |
| F | 904012 | 6，0×60 | TX30 | 100 |
| 复 | 904013 | 6，0×70 | TX30 | 100 |
| $v$ | 900014 | 6，0×80 | TX30 | 100 |
| ADVANTAGES | 900015 | 6，0×90 | TX30－ | 100 |
| －Limited resistance to acid | 904016 | $6,0 \times 100$ | TX30－ | 100 |
| Not suitable for use with woods containing tanning agents | 904017 | $6,0 \times 120$ | TX30 | 100 |
| such as cumarú，oak，merbau，robinia，etc． | 904018 | 6，0×140 | TX30 | 100 |
| －Magnetised | 904019 | $6,0 \times 160$ | TX30 | 100 |

Paneltwistec
Washer head，scrape poin， hardened stainless steel



| Art．no． | Dimensions［mm］ | Drive | PU |
| :---: | :---: | :---: | :---: |
| 94578 | 8，0×80 | TX40－ | 50 |
| 94570 | $8,0 \times 100$ | TX40 | 50 |
| 945271 | $8,0 \times 120$ | TX40－ | 50 |
| 94572 | $8,0 \times 140$ | TX40－ | 50 |
| 945364 | $8,0 \times 160$ | Tx40 | 50 |
| 945365 | $8,0 \times 180$ | TX40 | 50 |
| 945366 | $8,0 \times 200$ | TX40－ | 50 |
| 945367 | 8，0 $\times 220$ | Tx40 | 50 |
| 945368 | 8，0 240 | TX40 | 50 |
| 945369 | $8,0 \times 260$ | TX40－ | 50 |
| 945370 | 8，0×280 | Tx40－ | 50 |
| 945371 | 8，0×300 | Tx40 | 50 |
| 945372 | 8，0×320 | TX40－ | 50 |
| 94573 | 8，0× 340 | Tx40 | 50 |
| 945374 | 8，0×360 | TX40－ | 50 |
| 945375 | 8，0×380 | TX40－ | 50 |
| 945376 | 8，0x 400 | TX40－ | 50 |

## ADVANTAGES

Also suitable for fastening over－rafter insulation
－The larger head diameter allows for considerably higher torque and head pull－through capacity
This makes for better use of the screw＇s tensile load－bearing strength

Gurotec | Constructive fastening

## PANELTWISTEC AG

Washer head, hardened stainless steel

| Paneltwistec AG | \%CE | Art. no. | Dimensions [mm] | Drive | PU |
| :---: | :---: | :---: | :---: | :---: | :---: |
| Washer head, screw tip AG, hardened stainless steel | 边 | 975772 | 6,0×60 | TX30 ${ }^{\text {c }}$ | 100 |
|  |  | 975773 | 6,0×80 | TX30 - | 100 |
|  |  | 97574 | $6,0 \times 100$ | TX30 - | 100 |
|  |  | 975775 | $6,0 \times 120$ | TX30 - | 100 |
|  |  | 97576 | 6,0x 140 | TX30 - | 100 |
|  |  | 97577 | $6,0 \times 160$ | TX30 - | 100 |

## PANELTWISTEC A2

Stainless steel A2

Paneltwistec A2
Countersunk head, Stainless steel A2


| Art. no. | Dimensions [mm] | Drive | PU |
| :---: | :---: | :---: | :---: |
| 903330 | 8,0x80 | TX40 - | 50 |
| 903231 | $8,0 \times 100$ | TX40 - | 50 |
| 903332 | $8,0 \times 120$ | TX40 - | 50 |
| 903233 | $8,0 \times 140$ | TX40 - | 50 |
| 903234 | $8,0 \times 160$ | TX40 - | 50 |
| 903235 | $8,0 \times 180$ | TX40 - | 50 |
| 903236 | $8,0 \times 200$ | TX40 - | 50 |
| 903237 | 8,0x 220 | TX40 - | 50 |
| 903338 | 8,0×240 | TX40 - | 50 |
| 903239 | $8,0 \times 260$ | TX40 - | 50 |
| 903240 | 8,0 280 | TX40 - | 50 |
| 90324 | $8,0 \times 300$ | TX40 - | 50 |
| 903242 | 8,0x320 | TX40 - | 50 |
| 903243 | 8,0 $\times 340$ | TX40 - | 50 |
| 90324 | 8,0× 360 | TX40 - | 50 |
| 903245 | 8,0x 380 | TX40 - | 50 |
| 90324 | $8,0 \times 400$ | TX40 - | 50 |

ADVANTAGES
Limited resistance to acid
Not suitable for atmospheres containing chlorine

## Panelłwistec A2

Washer head, Stainless steel A2


| Art. no. | Dimensions [mm] | Drive | PU |
| :---: | :---: | :---: | :---: |
| 903211 | $8,0 \times 80$ | TX40 • | 50 |
| 903212 | $8,0 \times 100$ | TX40 - | 50 |
| 903213 | 8,0 $\times 120$ | TX40 - | 50 |
| 903214 | 8,0x 140 | TX40 - | 50 |
| 903215 | 8,0 $\times 160$ | TX40 - | 50 |
| 903216 | $8,0 \times 180$ | TX40 - | 50 |
| 903217 | $8,0 \times 200$ | TX40 - | 50 |
| 903218 | 8,0 $\times 220$ | TX40 - | 50 |
| 903219 | 8,0 240 | TX40 - | 50 |
| 903220 | 8,0 $\times 260$ | TX40 - | 50 |
| 903221 | 8,0 $\times 280$ | TX40 - | 50 |
| 903222 | 8,0 $\times 300$ | TX40 - | 50 |
| 903223 | 8,0 $\times 320$ | TX40 - | 50 |
| 903224 | 8,0x 340 | TX40 - | 50 |
| 903225 | 8,0 $\times 360$ | TX40 - | 50 |
| 903226 | 8,0 $\times 380$ | TX40 - | 50 |
| 903227 | 8,0x400 | TX40 - | 50 |
| ADVANTAGES |  |  |  |
| - Limited resistance to acid |  |  |  |
| - Not suitable for atmospheres containing chlorine |  |  |  |

Eurotec | Constructive fastening

## PANELTWISTEC A4

Stainless steel A4

| Paneltwistec | \% ( Art. no. | Dimensions [mm] | Drive | PU |
| :---: | :---: | :---: | :---: | :---: |
| Countersunk-head, Stainless steel A4 | 901476 | 4,0×25 | TX20 | 500 |
|  | - 111442 | 4,0×35 | TX20 | 500 |
|  | 90302 | 4,0x40 | TX20 | 500 |
|  | 111443 | 4,0 $\times 45$ | TX20 | 500 |
|  | 901109 | 4,0x55 | T 220 - | 500 |
|  | 111444 | $4,0 \times 60$ | TX20 | 500 |
|  | 111445 | 4,0x70 | T 220 - | 200 |
|  | 111446 | 4,0x80 | TX20 | 200 |
|  | 111447 | 4,5 45 | TX25 | 200 |
|  | 111448 | $4,5 \times 60$ | TX25 | 200 |
|  | 111449 | 4,5×70 | TX25 | 200 |
|  | 111450 | 4,5 $\times 80$ | TX25 | 200 |
|  | 903990 | $5,0 \times 40$ | TX25 | 200 |
|  | 111451 | 5, $\times 50$ | TX25 | 200 |
|  | 111452 | 5,0×60 | TX25 | 200 |
|  | 111453 | $5,0 \times 70$ | TX25 | 200 |
|  | 111454 | 5,0x80 | TX25 | 200 |
|  | 903580 | 5,0× 100 | TX25 | 200 |
|  | 111459 | 6,0×60 | TX30 | 100 |
|  | 948885 | 6,0×70 | TX30 | 100 |
|  | 111460 | 6,0×80 | TX30 | 100 |
|  | 111458 | $6,0 \times 100$ | TX30 | 100 |
|  | 901478 | 6,0 $\times 120$ | TX30 | 100 |
|  | 903380 | $8,0 \times 80$ | TX40 - | 50 |
|  | 903881 | $8,0 \times 100$ | TX40 - | 50 |
|  | 903782 | $8,0 \times 120$ | TX40 - | 50 |
|  | 90383 | $8,0 \times 140$ | TX40 - | 50 |
|  | 903284 | $8,0 \times 160$ | TX40 | 50 |
|  | 90328 | 8,0x 180 | TX40 - | 50 |
|  | 903286 | $8,0 \times 200$ | TX40 - | 50 |
|  | 90388 | $8,0 \times 220$ | TX40 | 50 |
|  | 903788 | $8,0 \times 240$ | TX40 - | 50 |
|  | 903889 | 8,0 2260 | TX40 - | 50 |
|  | 903290 | $8,0 \times 280$ | TX40 - | 50 |
|  | 903291 | 8,0 300 | TX40 - | 50 |
|  | 903292 | 8,0 320 | TX40 - | 50 |
|  | 903293 | 8,0 340 | TX40 - | 50 |
|  | 903294 | $8,0 \times 360$ | TX40 | 50 |
|  | 903295 | 8,0 380 | TX40 - | 50 |
|  | 903296 | $8,0 \times 400$ | TX40 - | 50 |


| Paneltwistec A4 |  |  | Art. no. | Dimensions [mm] | Drive | PU |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Washer head, Stainless steel A4 |  | 新 | 903260 | $8,0 \times 80$ | TX40 | 50 |
|  |  |  | 903261 | 8,0 $\times 100$ | TX40 - | 50 |
|  |  |  | 903262 | $8,0 \times 120$ | TX40 - | 50 |
|  |  |  | 903263 | 8,0 $\times 140$ | TX40 - | 50 |
|  |  |  | 903264 | $8,0 \times 160$ | TX40 - | 50 |
|  |  |  | 903265 | $8,0 \times 180$ | TX40 - | 50 |
|  |  |  | 903266 | $8,0 \times 200$ | TX40 - | 50 |
|  |  |  | 903267 | 8,0 $\times 220$ | TX40 - | 50 |
|  |  |  | 903268 | $8,0 \times 240$ | TX40 - | 50 |
|  |  |  | 903269 | $8,0 \times 260$ | TX40 - | 50 |
|  |  |  | 903270 | $8,0 \times 280$ | TX40 - | 50 |
|  |  |  | 903271 | $8,0 \times 300$ | TX40 - | 50 |
|  |  |  | 903272 | $8,0 \times 320$ | TX40 - | 50 |
|  |  |  | 903273 | $8,0 \times 340$ | TX40 - | 50 |
|  |  |  | 903274 | $8,0 \times 360$ | TX40 - | 50 |
|  |  |  | 903275 | $8,0 \times 380$ | TX40 - | 50 |
|  |  |  | 903276 | $8,0 \times 400$ | TX40 • | 50 |
|  |  |  | ADVANTAGES |  |  |  |
|  |  |  | - Limited resistance to acid |  |  |  |
|  |  |  | - Suitable for use with woods containing tanning agents such as cumarú, oak, merbau, robinia, etc. |  |  |  |
|  |  |  | - Suitable for saline atmospheres |  |  |  |
|  |  |  | - Not suitable for atmospheres containing chlorine |  |  |  |
|  |  |  | - The screw is suitable for use in timber / timber joints in outdoor installations and is used in garden, façade and balcony construction |  |  |  |

## SAWTEC

Wood construction screw made of hardened carbon steel

The SawTec is a wood construction screw with a special screw tip and saw teeth below the head. The screw has a double-stage cylinder head. The special geometry of the screw tip reduces the screwing torque and also leads to a lower spliting effect when screwing in.


Friction part

- Friction part creates space for the shank, thereby reduces the insertion resistance

Coarse thread

- Speeds up the screwing-in process

DAG screw tip

- The special geometry of the DAG screw tip ensures a reduction of the screwing torque and also leads to a lower splitting effect when screwing-in


| SawTec | Art. no. | Dimensions [mm] | Drive | PU |
| :---: | :---: | :---: | :---: | :---: |
| Cylinder head, blue galvanised | 954115 | $5,0 \times 40$ | TX25 | 200 |
|  | 954117 | 5,0×50 | TX25 | 200 |
| T | 954118 | $5,0 \times 60$ | TX25 | 200 |
|  | 954119 | 5,0×70 | TX25 | 200 |
|  | 954120 | 5,0×80 | TX25 | 200 |
|  | 954121 | 5,0×90 | TX25 | 200 |
|  | 954122 | $5,0 \times 100$ | TX25 | 200 |
|  | 954124 | 5,0× 120 | Tx25 | 200 |
|  | 954128 | 6,0×60 | TX30 | 100 |
|  | 954129 | $6,0 \times 70$ | TX30- | 100 |
|  | 954130 | 6,0 $\times 80$ | TX30- | 100 |
|  | 954131 | 6,0× 100 | TX30 - | 100 |
|  | 954133 | 6,0x 120 | TX30- | 100 |
|  | 954135 | 6,0x 140 | TX30- | 100 |
|  | 954137 | 6,0×160 | TX30- | 100 |
|  | 954138 | $6,0 \times 180$ | TX30- | 100 |
|  | 954145 | 8,0 80 | TX40 - | 50 |
|  | 954146 | $8,0 \times 100$ | Tx40 - | 50 |
|  | 954147 | $8,0 \times 120$ | TX40 - | 50 |
|  | 954148 | $8,0 \times 140$ | TX40 - | 50 |
|  | 954149 | $8,0 \times 160$ | TX40 - | 50 |
| 1 | 954150 | $8,0 \times 180$ | TX40 | 50 |
|  | 954151 | 8,0 200 | TX40 - | 50 |
|  | 954152 | 8,0x 220 | TX40 - | 50 |
| \% | 954153 | 8,0 240 | TX40 - | 50 |
| ADVANTAGES | 954154 | $8,0 \times 260$ | TX40 - | 50 |
| - Faster and easier screwing-in due to the DAG tip | 954155 | $8,0 \times 280$ | TX40 | 50 |
| . The DAG tip reduces the screw-in torque | 954156 | 8,0x 300 | TX40 | 50 |
| - The DAG tip reduces the screw-in torque | 954157 | 8,0x 320 | TX40 - | 50 |
| - Reduced spliting effect | 954158 | $8,0 \times 340$ | TX40 - | 50 |
| . Screws do not hit one another when screwed in using the TX drive | 954159 | 8,0x 360 | TX40 - | 50 |
|  | 954160 | 8,0x 380 | TX40 - | 50 |
|  | 954161 |  | TX40 - | 50 |
| APPLLCATION INFORMATION | 954181 | 8,0x 420 | TX40 - | 50 |
| Can be used in service classes 1 and 2 according to | 954182 | $8,0 \times 440$ | TX40 | 50 |
| DIN EN 1995 - Eurocode 5 | 954183 | $8,0 \times 460$ | TX40 - | 50 |
|  | 954184 | 8,0x 480 | TX40 - | 50 |
|  | 954185 | 8,0x 500 | TX40 - | 50 |
|  | 954186 | 8,0x 550 | TX40 - | 50 |
|  | 954187 | $8,0 \times 600$ | Tx40 - | 50 |
|  | 954162 | $10,0 \times 100$ | TX50 - | 50 |
|  | 954163 | 10,0 $\times 120$ | TX50 - | 50 |
|  | 954164 | 10,0 $\times 140$ | Tx50 - | 50 |
|  | 954165 | $10,0 \times 160$ | TX50 - | 50 |
|  | 954166 | $10,0 \times 180$ | TX50 - | 50 |
|  | 954167 | $10,0 \times 200$ | TX50 - | 50 |
|  | 954168 | 10,0 $\times 220$ | Tx50 - | 50 |
|  | 954169 | 10,0 $\times 240$ | TX50 - | 50 |
|  | 954170 | 10,0 $\times 260$ | Tx50 - | 50 |
|  | 954171 | 10,0 $\times 280$ | TX50 - | 50 |
|  | 954172 | 10,0 $\times 300$ | TX50 - | 50 |
|  | 954173 | 10,0 $\times 320$ | TX50 | 50 |
|  | 954174 | 10,0 $\times 340$ | TX50 - | 50 |
|  | 954175 | 10,0 $\times 360$ | TX50 | 25 |
|  | 954176 | 10,0×380 | TX50 | 25 |
|  | 954177 | 10,0x400 | TX50 - | 25 |



| $\begin{aligned} & \mathrm{dl} \times \mathrm{L} \\ & {[\mathrm{~mm}]} \end{aligned}$ | $\begin{gathered} d k \\ {[\mathrm{~mm}]} \end{gathered}$ | $\begin{gathered} \mathrm{AD} \\ {[\mathrm{~mm}]} \end{gathered}$ | $\begin{gathered} \mathrm{It} \\ {[\mathrm{~mm}]} \end{gathered}$ | $\mathrm{Fax}_{\mathrm{ax}}, 90$,Rk <br> [kN] | Fox,head,Rk [kN] | Fla,Rk <br> [kN] | Fla,Rk <br> [kN] | Fla,Rk <br> [kN] | Fla,Rk [kN] | $\begin{gathered} \dagger \\ {[\mathrm{mm}]} \end{gathered}$ | Fla,Rk <br> [kN] | Fla,Rk <br> [kN] |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
|  |  |  |  |  |  |  |  | $\alpha_{A D}=0^{\circ}$ | $\alpha A D=90^{\circ}$ |  |  |  |
|  |  |  |  |  |  | $\alpha=0^{\circ}$ | $\alpha=90^{\circ}$ | $\alpha_{\mathrm{E}}=90^{\circ}$ | $\alpha \in I=0^{\circ}$ |  | $\alpha=0^{\circ}$ | $\alpha=90^{\circ}$ |
| 5,0×40 | 10,5 | 16 | 24 | 1,45 | 1,10 |  |  |  |  | 2 |  |  |
| 5,0×50 | 10,5 | 20 | 30 | 1,82 | 1,10 |  |  |  |  | 2 |  |  |
| 5,0×60 | 10,5 | 24 | 36 | 2,18 | 1,10 |  |  |  |  | 2 |  |  |
| 5,0×70 | 10,5 | 28 | 42 | 2,54 | 1,10 |  |  |  |  | 2 |  |  |
| 5,0×80 | 10,5 | 32 | 48 | 2,90 | 1,10 |  |  |  |  | 2 |  |  |
| 5,0×90 | 10,5 | 36 | 54 | 3,27 | 1,10 |  |  |  |  | 2 |  |  |
| 5,0× 100 | 10,5 | 40 | 60 | 3,63 | 1,10 |  |  |  |  | 2 |  |  |
| 5,0× 120 | 10,5 | 60 | 60 | 3,63 | 1,10 |  |  |  |  |  |  |  |
| 6,0×60 | 13,0 | 24 | 36 | 2,46 | 1,69 |  |  |  |  | 2 |  |  |
| 6,0×70 | 13,0 | 28 | 42 | 2,87 | 1,69 |  |  |  |  | 2 |  |  |
| 6,0×80 | 13,0 | 32 | 48 | 3,28 | 1,69 |  |  |  |  | 2 |  |  |
| 6,0×90 | 13,0 | 36 | 54 | 3,69 | 1,69 |  |  |  |  | 2 |  |  |
| 6,0 $\times 100$ | 13,0 | 40 | 60 | 4,10 | 1,69 |  |  |  |  | , |  |  |
| 6,0×110 | 13,0 | 50 | 60 | 4,10 | 1,69 |  |  |  |  | 2 |  |  |
| $6,0 \times 120$ | 13,0 | 60 | 60 | 4,10 | 1,69 |  |  |  |  | 2 |  |  |
| 6,0×130 | 13,0 | 60 | 70 | 4,79 | 1,69 |  |  |  |  | 2 |  |  |
| 6,0×140 | 13,0 | 70 | 70 | 4,79 | 1,69 |  |  |  |  | 2 |  |  |
| $6,0 \times 150$ | 13,0 | 80 | 70 | 4,79 | 1,69 |  |  |  |  | 2 |  |  |
| $6,0 \times 160$ | 13,0 | 90 | 70 | 4,79 | 1,69 |  |  |  |  | 2 |  |  |
| $6,0 \times 180$ | 13,0 | 110 | 70 | 4,79 | 1,69 |  |  |  |  | 2 |  |  |
| 8,0x80 | 18,0 | 30 | 50 | 4,26 | 3,24 | 3,89 | 3,08 | 3,89 | 3,08 | 3 | 4,61 | 3,94 |
| $8,0 \times 100$ | 18,0 | 40 | 60 | 5,33 | 3,24 | 4,31 | 3,48 | 4,31 | 3,48 | , | 4,83 | 4,20 |
| $8,0 \times 120$ | 18,0 | 60 | 60 | 5,33 | 3,24 | 4,31 | 3,68 | 4,31 | 3,68 | 3 | 4,83 | 4,20 |
| $8,0 \times 140$ | 18,0 | 40 | 100 | 8,44 | 3,24 | 4,31 | 3,48 | 4,31 | 3,48 |  | 5,60 | 4,98 |
| $8,0 \times 160$ | 18,0 | 60 | 100 | 8,44 | 3,24 | 4,31 | 3,68 | 4,31 | 3,68 | 3 | 5,60 | 4,98 |
| $8,0 \times 180$ | 18,0 | 80 | 100 | 8,44 | 3,24 | 4,31 | 3,68 | 4,31 | 3,68 | 3 | 5,60 | 4,98 |
| 8,0 200 | 18,0 | 100 | 100 | 8,44 | 3,24 | 4,31 | 3,68 | 3,68 | 4,31 | 3 | 5,60 | 4,98 |

[^6]with regard to the usage class and dlass of the load duration: $R_{d}=R_{k} \cdot k_{\bmod } / \gamma \mathrm{m}$. The dimensioning values of the load-bearing capacity $\mathrm{Rd}_{d}$ should be contrasted with the dimensioning values of the loads ( $\left.\mathrm{Rd}_{\mathrm{d}} \geq \mathrm{E}_{\mathrm{d}}\right)$.

## Example:

Characterisicic value for constant lood (dead weight) $G_{k}=2,00 \mathrm{kN}$ and variable load (e. g. snow load) $Q_{k}=3,00 \mathrm{kN}$. $\mathrm{kmod}=0,9 . \gamma \mathrm{m}=1,3$.
$\rightarrow$ Dimensioning value of the load $\mathrm{E}_{\mathrm{d}}=2,00 \cdot 1,35+3,00 \cdot 1,5=7,20 \mathrm{kN}$.
The load-bearing capacity of the joint is therefore considered to have been demonstrated if $\mathrm{Rd} \geq \mathrm{Ed} . \rightarrow \mathrm{min} \mathrm{Rk}=\mathrm{Rd} \cdot \gamma \mathrm{M} / \mathrm{kmod}$
I.e. the characterisicic minimum value is calculated based on: $\min \mathrm{R}_{\mathrm{k}}=\mathrm{R}_{d} \cdot \gamma_{\mathrm{M}} / \mathrm{kmod} \rightarrow \mathrm{R}_{\mathrm{k}}=7,20 \mathrm{kN} \cdot 1,3 / 0,9=10,40 \mathrm{kN} \rightarrow$ comparison with table values.

Please note: These are planning ciids. Projects must only be calculated by authorised persons.
Please note: Verify the assumptions made. The stated values, and type and number of joining devices are based on preliminary measurements. Projects are to be dimensioned exclusively by outhorised persons in accordance with the State Building Code. As per LBuoO, please contact a qualified structural engineer for a paid proof of stability. We will be happy to refer you to someone.

| Dimensions |  |  |  | Extraction resistance | Head pull-through resistance |  | Wood-Wood | d shearing |  |  | Wood shearing |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
|  |  |  |  |  |  | $\xrightarrow{V\left(\alpha=0^{\circ}\right)}$ $V\left(a=0^{\circ}\right)$ $V\left(\alpha=0^{\circ}\right)$ $V\left(\alpha=90^{\circ}\right)$ | $A D$ | $\begin{aligned} & \text { AD } \quad \stackrel{V\left(a=90^{\circ}\right)}{\square} \\ & \text { ET } \quad \begin{array}{l} V\left(a=90^{\circ}\right) \\ \text { AD } \\ \text { ET } \quad \underline{\left(a=90^{\circ}\right)} \\ V\left(a=0^{\circ}\right) \end{array} \end{aligned}$ |  |  |  |  |
| $\begin{aligned} & d l \times l \\ & {[\mathrm{~mm}]} \end{aligned}$ | $\begin{gathered} \mathrm{dk} \\ {[\mathrm{~mm}]} \end{gathered}$ | $\begin{gathered} A D \\ {[\mathrm{~mm}]} \end{gathered}$ | $\begin{gathered} \mathrm{EI} \\ {[\mathrm{~mm}]} \end{gathered}$ | $F_{a x}, 90, R k$ <br> [kN] | Fax,head, Rk <br> [kN] | Fla,Rk <br> [kN] | Fla,Rk <br> [kN] | Fla,Rk <br> [kN] | Fla,Rk <br> [kN] | $\begin{gathered} \dagger \\ {[\mathrm{mm}]} \end{gathered}$ | Fla,Rk <br> [kN] | Fla,Rk <br> [kN] |
|  |  |  |  |  |  |  |  | $a_{A D}=0^{\circ}$ | $\alpha_{A D}=90^{\circ}$ |  |  |  |
|  |  |  |  |  |  | $\alpha=0^{\circ}$ | $\alpha=90^{\circ}$ | $\alpha_{\mathrm{E}}=90^{\circ}$ | $\alpha_{E T}=0^{\circ}$ |  | $\alpha=0^{\circ}$ | $\alpha=90^{\circ}$ |
| 8,0×220 | 18,0 | 120 | 100 | 8,44 | 3,24 | 4,31 | 3,68 | 3,68 | 4,31 | 3 | 5,60 | 4,98 |
| 8,0 240 | 18,0 | 140 | 100 | 8,44 | 3,24 | 4,31 | 3,68 | 3,68 | 4,31 | 3 | 5,60 | 4,98 |
| 8,0 260 | 18,0 | 160 | 100 | 8,44 | 3,24 | 4,31 | 3,68 | 3,68 | 4,31 | 3 | 5,60 | 4,98 |
| 8,0 280 | 18,0 | 180 | 100 | 8,44 | 3,24 | 4,31 | 3,68 | 3,68 | 4,31 | 3 | 5,60 | 4,98 |
| 8,0x 300 | 18,0 | 200 | 100 | 8,44 | 3,24 | 4,31 | 3,68 | 3,68 | 4,31 | 3 | 5,60 | 4,98 |
| 8,0x 320 | 18,0 | 220 | 100 | 8,44 | 3,24 | 4,31 | 3,68 | 3,68 | 4,31 | 3 | 5,60 | 4,98 |
| 8,0x 340 | 18,0 | 240 | 100 | 8,44 | 3,24 | 4,31 | 3,68 | 3,68 | 4,31 | 3 | 5,60 | 4,98 |
| 8,0x 360 | 18,0 | 260 | 100 | 8,44 | 3,24 | 4,31 | 3,68 | 3,68 | 4,31 | 3 | 5,60 | 4,98 |
| 8,0x 380 | 18,0 | 280 | 100 | 8,44 | 3,24 | 4,31 | 3,68 | 3,68 | 4,31 | 3 | 5,60 | 4,98 |
| 8,0x 400 | 18,0 | 300 | 100 | 8,44 | 3,24 | 4,31 | 3,68 | 3,68 | 4,31 | 3 | 5,60 | 4,98 |
| 8,0x 420 | 18,0 | 320 | 100 | 8,44 | 3,24 | 4,31 | 3,68 | 3,68 | 4,31 | 3 | 5,60 | 4,98 |
| 8,0x 440 | 18,0 | 340 | 100 | 8,44 | 3,24 | 4,31 | 3,68 | 3,68 | 4,31 | 3 | 5,60 | 4,98 |
| 8,0x 460 | 18,0 | 360 | 100 | 8,44 | 3,24 | 4,31 | 3,68 | 3,68 | 4,31 | 3 | 5,60 | 4,98 |
| 8,0x 480 | 18,0 | 380 | 100 | 8,44 | 3,24 | 4,31 | 3,68 | 3,68 | 4,31 | 3 | 5,60 | 4,98 |
| 8,0x 500 | 18,0 | 400 | 100 | 8,44 | 3,24 | 4,31 | 3,68 | 3,68 | 4,31 | 3 | 5,60 | 4,98 |
| 8,0x 550 | 18,0 | 450 | 100 | 8,44 | 3,24 | 4,31 | 3,68 | 3,68 | 4,31 | 3 | 5,60 | 4,98 |
| 8,0x600 | 18,0 | 500 | 100 | 8,44 | 3,24 | 4,31 | 3,68 | 3,68 | 4,31 | 3 | 5,60 | 4,98 |
| 10,0 100 | 22,0 | 40 | 60 | 6,48 | 4,84 | 6,03 | 4,67 | 6,03 | 4,67 | 3 | 6,78 | 5,81 |
| $10,0 \times 120$ | 22,0 | 60 | 60 | 6,48 | 4,84 | 6,37 | 5,40 | 6,37 | 5,40 | 3 | 6,78 | 5,81 |
| $10,0 \times 140$ | 22,0 | 40 | 100 | 10,26 | 4,84 | 6,03 | 4,67 | 6,03 | 4,67 | 3 | 7,72 | 6,76 |
| $10,0 \times 160$ | 22,0 | 60 | 100 | 10,26 | 4,84 | 6,37 | 5,40 | 6,37 | 5,40 | 3 | 7,72 | 6,76 |
| $10,0 \times 180$ | 22,0 | 80 | 100 | 10,26 | 4,84 | 6,37 | 5,40 | 6,37 | 5,40 | 3 | 7,72 | 6,76 |
| 10,0 200 | 22,0 | 100 | 100 | 10,26 | 4,84 | 6,37 | 5,40 | 5,40 | 6,37 | 3 | 7,72 | 6,76 |
| $10,0 \times 220$ | 22,0 | 120 | 100 | 10,26 | 4,84 | 6,37 | 5,40 | 5,40 | 6,37 | 3 | 7,72 | 6,76 |
| $10,0 \times 240$ | 22,0 | 140 | 100 | 10,26 | 4,84 | 6,37 | 5,40 | 5,40 | 6,37 | 3 | 7,72 | 6,76 |
| $10,0 \times 260$ | 22,0 | 160 | 100 | 10,26 | 4,84 | 6,37 | 5,40 | 5,40 | 6,37 | 3 | 7,72 | 6,76 |
| $10,0 \times 280$ | 22,0 | 180 | 100 | 10,26 | 4,84 | 6,37 | 5,40 | 5,40 | 6,37 | 3 | 7,72 | 6,76 |
| $10,0 \times 300$ | 22,0 | 200 | 100 | 10,26 | 4,84 | 6,37 | 5,40 | 5,40 | 6,37 | 3 | 7,72 | 6,76 |
| $10,0 \times 320$ | 22,0 | 220 | 100 | 10,26 | 4,84 | 6,37 | 5,40 | 5,40 | 6,37 | 3 | 7,72 | 6,76 |
| $10,0 \times 340$ | 22,0 | 240 | 100 | 10,26 | 4,84 | 6,37 | 5,40 | 5,40 | 6,37 | 3 | 7,72 | 6,76 |
| $10,0 \times 360$ | 22,0 | 260 | 100 | 10,26 | 4,84 | 6,37 | 5,40 | 5,40 | 6,37 | 3 | 7,72 | 6,76 |
| $10,0 \times 380$ | 22,0 | 280 | 100 | 10,26 | 4,84 | 6,37 | 5,40 | 5,40 | 6,37 | 3 | 7,72 | 6,76 |
| $10,0 \times 400$ | 22,0 | 300 | 100 | 10,26 | 4,84 | 6,37 | 5,40 | 5,40 | 6,37 | 3 | 7,72 | 6,76 |

Calculation according to $\mathrm{EA}-\mathrm{-l} / 0024$. Wood density $\rho_{\mathrm{k}}=350 \mathrm{~kg} / \mathrm{m}^{3}$. All mechanical values provided should be viewed as subject to the assumptions that have been made and represent example calculations.
All values are calculated minimum values and are subiect to typoographical and printing errors.
a) The characterisisic values of the lood-bearing capacity $R_{k}$ cannot be treated os equivalent to the max. possible load (the max. force). Characteristic values of the lood-bearing capacity $R_{k}$ should be reduced to dimensioning values $R_{d}$ with regard to the usage class and class of the lood duration: $R_{d}=R_{k} \cdot k_{m o d} / \gamma$ M. The dimensioning values of the lood- bearing capacity $R d$ should be contrassed with the dimensioning values of the loads (Rd $\geq$ Ed).

## Example:.:

Characteristic value for constant lood (dead weight) $G_{k}=2,00 \mathrm{kN}$ and variable load (e. g. snow load) $Q_{k}=3,00 \mathrm{kN} . \mathrm{kmod}^{2}=0,9 . \gamma \mathrm{M}=1,3$.
$\rightarrow$ Dimensioning value of the lood $\mathrm{E}_{\mathrm{d}}=2,00 \cdot 1,35+3,00 \cdot 1,5=7,20 \mathrm{kN}$.
The load-bearing capacity of the joint is therefore considered to have been demonstrated if $\mathrm{Rd}_{\mathrm{d}} \geq \mathrm{Ed} . \rightarrow \min \mathrm{Rk}_{\mathrm{k}}=\mathrm{Rd}_{d} \cdot \gamma \mathrm{YM} / \mathrm{kmod}$
I.e. the characteristic minimum value is calculated based on: $\min \mathrm{R}_{\mathrm{k}}=\mathrm{R}_{\mathrm{d}} \cdot \gamma_{\mathrm{M}} / \mathrm{kmod} \rightarrow \mathrm{Rk}_{\mathrm{k}}=7,20 \mathrm{kN} \cdot 1,3 / 0,9=10,40 \mathrm{kN} \rightarrow$ comparison with table values.

Please note: These are planning cids. Projects must only be calculated by authorised persons.

## TOPDUO ROOFING SCREW

The wood-construction screw for all over-rafter insulation systems

The Topduo roofing screw can be used to fasten both compression-resistant and non-compression-resistant above-rafter insulation. The high pull-out resistance in both connecting timbers also makes the TopDuo roofing screw suitable for many other applications in timber construction. The screw has a double thread and is available with a flanged buttonhead and cylinder head.

Cylinder head

- Virtually disappears in wood
- Speeds up the screwingin process

Underhead thread with cutting notches



- Reamer creates space for the shank, reducing the screw-in resistance

DAG screw tip

- The special geometry of the DAG screw tip ensures a reduction of the screwing torque and also leads to a lower splitting effect when
screwing-in


## FASTENING OPTIONS:

Topduo is suitable for pressure resistant ( $\geq \mathbf{5 0} \mathbf{~ k P a}$ ) and non-pressure resistant insulations.
The compressive strength $010 \%$ can be found in the product data sheet issued by the insulating material manufacturer.

Solely $90^{\circ}$ screw connection
(absorbtion of wind suction)


Combined $65^{\circ}$ and $90^{\circ}$ screw connection labsorbtion of shearing forces and wind suction)


Topduo cylinder head for fastening insulation material.

## TOPDUO ROOFING SCREW

The wood-construction screw for all over-rafter insulation systems

Topduo roofing screw
Washer head, hardened carbon steel, electrogalvanised

|  | Art. no. | Dimensions [mm] | Length [mm] ${ }^{\text {a) }}$ | Drive | PU |
| :---: | :---: | :---: | :---: | :---: | :---: |
| $5$ | 945870 | 8,0 $\times 165$ | 60/80 | TX40 - | 50 |
|  | 945871 | 8,0 $\times 195$ | 60/100 | TX40 - | 50 |
|  | 945813 | 8,0 $\times 225$ | 60/100 | TX40 - | 50 |
|  | 945814 | 8,00235 | 60/100 | TX40 - | 50 |
|  | 945815 | 8,00255 | 60/100 | TX40 - | 50 |
|  | 945816 | 8,0 $\times 275$ | 60/100 | TX40 - | 50 |
|  | 945817 | 8,00302 | 60/100 | TX40 - | 50 |
|  | 945818 | 8,00335 | 60/100 | TX40 - | 50 |
|  | 945819 | 8,0 $\times 365$ | 60/100 | TX40 - | 50 |
|  | 945820 | 8,0x 397 | 60/100 | TX40 - | 50 |
|  | 945821 | 8,0x 435 | 60/100 | TX40 - | 50 |
|  | 945843 | $8,0 \times 472$ | 60/100 | TX40 - | 50 |
|  | a) Under-head thread/drive thread |  |  |  |  |

Topduo roofing screw
Cylinder head, hardened carbon steel, electrogalvanised


| Art. no. | Dimensions [mm] | Length [mm] ${ }^{\text {a) }}$ | Drive | PU |
| :---: | :---: | :---: | :---: | :---: |
| 945956 | 8,00 225 | 60/100 | TX40 - | 50 |
| 945965 | 8,0 $\times 235$ | 60/100 | TX40 - | 50 |
| 945957 | $8,0 \times 255$ | 60/100 | TX40 - | 50 |
| 945958 | 8,0 $\times 275$ | 60/100 | TX40 - | 50 |
| 945960 | 8,0x 302 | 60/100 | TX40 - | 50 |
| 945961 | 8,0x 335 | 60/100 | TX40 - | 50 |
| 945962 | 8,0 $\times 365$ | 60/100 | TX40 • | 50 |
| 945963 | 8,0x 397 | 60/100 | TX40 - | 50 |
| 945964 | 8,0x 435 | 60/100 | TX40 • | 50 |

a) Under-head thread/drive thread


Roof construction with Topduo.


Façade construction with the Topduo roofing screw.


Topduo washer head for fastening insulation material.

## CALCULATING QUANTITIES FOR TOPDUO ROOFING SCREW

STATICALLY NON-PRESSURE-RESISTANT INSULATING MATERIALS AT $\sigma_{10} \%<50$ KPA

| Design sample for specified assumptions, project-related design may yied significantly more favourable results |  |  |  |  |  |  |  |  |  |  |  |  |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Number of Topduo screws per m${ }^{2}$ |  |  |  |  |  |  |  |  |  |  |  |  |  |  |
| Insulction thickness | 40 | 60 | 80 | 100 | 120 | 140 | 140 | 160 | 180 | 200 | 220 | 240 | 260 | 280 |
| Boarding thickness (on raters) | 24 | 24 | 24 | 24 | 24 | - | 24 | 24 | 24 | 24 | 24 | 24 | 24 | 24 |
| Dimensions Topduo Waster head | $8 \times 165^{\text {b }}$ | $\left.8 \times 195^{6}\right)$ | $8 \times 225$ | $8 \times 235$ | $8 \times 255$ | $8 \times 275$ | $8 \times 302$ | $8 \times 335$ | $8 \times 335$ | $8 \times 365$ | $8 \times 365$ | $8 \times 397$ | $8 \times 435$ | $8 \times 435$ |
| acc. Cyinder heodal | [mm] | [mm] | [mm] | [mm] | [mm] | [mm] | [mm] | [mm] | [mm] | [mm] | [mm] | [mm] | [mm] | [mm] |
| Snowload fone $0^{\circ} \leq \mathrm{DN} \leq 10^{\circ}$ | 2,20 | 2,20 | 2,38 | 2,38 | 2,38 | 2,38 | 2,38 | 2,29 | 2,29 | 2,48 | 3,01 | 3,57 | 4,08 | 4,76 |
| $\text { Wind zone } 4^{(0)} 10^{\circ}<\mathrm{DN} \leq 25^{\circ}$ | 2,38 | 2,38 | 2,60 | 2,60 | 2,60 | 2,60 | 2,60 | 2,60 | 2,60 | 3,17 | 3,81 | 4,40 | e) | e) |
| Altitude $\mathrm{NW} 25^{\circ}<\mathrm{DN} \leq 40^{\circ}$ | 2,72 | 2,72 | 3,01 | 3,01 | 3,01 | 3,01 | 3,01 | 3,01 | 3,01 | 3,57 | 4,40 | 5,19 | e) | e) |
| $\leq 285 \mathrm{~m} \quad 40^{\circ}<\mathrm{DN} \leq 60^{\circ}$ | 2,86 | 3,01 | 3,17 | 3,17 | 3,36 | 3,36 | 3,36 | 3,36 | 3,36 | 3,57 | 4,40 | 5,19 | e) | e) |
| Snowload $0^{\circ} \leq$ DN $\leq 10^{\circ}$ | 1,79 | 1,79 | 1,97 | 2,04 | 2,04 | 2,04 | 2,04 | 2,12 | 2,60 | 3,81 | 4,40 | 5,19 | e) | e) |
|  | 2,29 | 2,29 | 2,48 | 2,60 | 2,60 | 2,60 | 2,60 | 2,72 | 3,36 | 4,76 | e) | e) | e) | e) |
| $\begin{aligned} & \begin{array}{l} \text { Wind } 20 n e \\ \text { Alituve NNI } \\ \text { 25 } \end{array} 25^{\circ}<\mathrm{DN} \leq 40^{\circ} \end{aligned}$ | 2,38 | 2,48 | 2,72 | 2,72 | 2,72 | 2,86 | 2,86 | 2,86 | 3,57 | 5,19 | e) | e) | e) | e) |
| $\leq 600 \mathrm{~m} 40^{\circ}<\mathrm{DN} \leq 60^{\circ}$ | 2,60 | 2,60 | 2,86 | 2,86 | 2,86 | 2,86 | 2,86 | 3,01 | 3,57 | 5,19 | e) | e) | e) | e) |

a) Quantity always refers to the less favourable valve from Topdvo Washer head and Cylinder-head
b) Topduo Washer head only, () Includes snow load zones 1, 2 and 2*, d) Includes all wind zones apart from North Sea istands
e) Use of our project assessment sevvice is recommended. The design examples isted here represent unfavourable, i.e. statically safe, instances.
f) Includes snow lood zones 1,2 und $3, \mathrm{~g}$ ) Includes wind zones 1 and 2 (inland)

Further assumptions:
Design with ECS design software in accordance with ETA-II/0024; screw-in angle $65^{\circ}$; gabled roof; ridge height above ground max. 18 m ; gross density insulation $1,50 \mathrm{kN} / \mathrm{m}^{3}$; raffers $\mathrm{C} 248 / \geq 12 \mathrm{~cm}$; counter batten $\mathrm{C} 244 / 6 \mathrm{~cm}$; raffer centre distance $0,70 \mathrm{~m}$; roofing dead weight $0,55 \mathrm{kN} / \mathrm{m}^{2}$; snow guard available; quantity calculation regarding wind pressure after the most unfavourable roof area.
All listed values should be viewed as subject to the assumptions that have been made. They therefore represent example calculations and are subject to typographical and printing errors.
Please note: These are planning aids. Projects must only be calculated by authorised persons.

## CALCULATING QUANTITIES FOR TOPDUO ROOFING SCREW

STATICALLY PRESSURE-RESISTANT INSULATING MATERIALS AT $\sigma 10 \% \geq 50 \mathrm{KPA}$
Design sample for specified assumptions, project-related design may yield significantly more favourable results
Number of Topduo screws per m²

|  | Insulation thickness | 40 | 60 | 80 | 100 | 120 | 140 | 160 | 180 | 200 | 220 | 240 | 260 | 280 | 300 |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Boarding thich | ickness (on rafters) | 24 | 24 | 24 | 24 | 24 | 24 | 24 | 24 | 24 | 24 | 24 | 24 | 24 | 24 |
| Dimensions Topduo Washer head acc. (ylinder head ${ }^{(0)}$ |  | $8 \times 195^{\text {b }}$ | $8 \times 225$ | $8 \times 235$ | $8 \times 255$ | $8 \times 275$ | $8 \times 302$ | $8 \times 335$ | $8 \times 335$ | $8 \times 365$ | $8 \times 365$ | $8 \times 397$ | $8 \times 435$ | $8 \times 435$ | $8 \times 472^{\text {b }}$ |
|  |  | [mm] | [mm] | [mm] | [mm] | [mm] | [mm] | [mm] | [mm] | [mm] | [mm] | [mm] | [mm] | [mm] | [mm] |
| $\begin{aligned} & \text { Snow lood zone } \\ & 2^{*}(\text { c) } \\ & \text { Wind zone 4d) } \\ & \text { Altitude NN } \\ & \leq 285 \mathrm{~m} \end{aligned}$ | $0^{\circ} \leq \mathrm{DN} \leq 10^{\circ}$ | 1,96 | 2,06 | 2,06 | 2,06 | 2,06 | 2,06 | 2,06 | 2,06 | 2,06 | 2,06 | 2,12 | 1,80 | 2,40 | 2,32 |
|  | $10^{\circ}<\mathrm{DN} \leq 25^{\circ}$ | 2,11 | 2,05 | 1,97 | 1,94 | 1,97 | 1,90 | 1,85 | 2,14 | 2,01 | 2,74 | 2,57 | 2,38 | 3,23 | 2,93 |
|  | $25^{\circ}<\mathrm{DN} \leq 40^{\circ}$ | 2,48 | 2,41 | 2,28 | 2,35 | 2,41 | 2,35 | 2,18 | 2,67 | 2,49 | 3,48 | 3,22 | 2,96 | 4,42 | 3,79 |
|  | $40^{\circ}<\mathrm{DN} \leq 60^{\circ}$ | 2,31 | 2,30 | 2,56 | 2,65 | 2,74 | 2,65 | 2,42 | 2,96 | 2,74 | 4,00 | 3,70 | 3,48 | 4,87 | 4,47 |
| Snow load zone $3^{\text {f) }}$ Wind zone 2g) Altitude NN $\leq 400 \mathrm{~m}$ | $0^{\circ} \leq \mathrm{DN} \leq 10^{\circ}$ | 2,65 | 2,54 | 2,39 | 2,34 | 2,26 | 2,23 | 2,34 | 2,34 | 2,16 | 2,46 | 2,32 | 2,19 | 2,86 | 2,65 |
|  | $10^{\circ}<\mathrm{DN} \leq 25^{\circ}$ | 4,04 | 3,81 | 3,55 | 3,33 | 3,33 | 3,15 | 3,15 | 2,99 | 2,99 | 3,66 | 3,37 | 3,06 | 4,37 | 3,74 |
|  | $25^{\circ}<\mathrm{DN} \leq 40^{\circ}$ | 4,46 | 4,16 | 3,84 | 3,58 | 3,58 | 3,58 | 3,37 | 3,37 | 3,37 | 4,67 | 4,20 | 3,92 | e) | e) |
|  | $40^{\circ}<\mathrm{DN} \leq 60^{\circ}$ | 3,55 | 3,26 | 3,26 | 3,26 | 3,44 | 3,26 | 2,96 | 3,66 | 3,44 | e) | 4,67 | 4,27 | e) | e) |

[^7]Further assumptions:
Design with ECS design software in accordance with ETA-II/0024; screw-in angle roof thrusiscrew $65^{\circ} /$ wind pressure $\operatorname{screw} 90^{\circ}$; gabled roof; ridge height above ground max. 18 m ; gross density insulation $1,50 \mathrm{kN} / \mathrm{m}^{3}$; raffers $(248 / \geq 12 \mathrm{~cm} ;$ counter batten C 24
$4 / 6 \mathrm{~cm}$; rafter centre distance $0,70 \mathrm{~m}$; roofing dead weight $0,55 \mathrm{kN} / \mathrm{m}^{2}$; snow guard available; quantity calculation with respect to wind pressure affer the most unfavourable roof area.
All listed values should be viewed as subject to the assumptions that have been made. They therefore represent example calculations and are subject to typographical and printing errors.
Please note: These are planning aids. Projects must only be calculated by authorised persons.
Please note: Verify the assumptions made. The stated values, and type and number of poining devices are based on preliminary measurements. Projects are to be dimensioned exclusively by authorised
persons in accordance with the State Building Code. As per LBauO, please contact a qualified structural engineer for a paid proof of stability. We will be happy to refer you to someone.

## EuroTec calculation service

## On-rafter insulation according to ETA-1 1 /0024

by phone 02331 6245-444•by fax 02331 6245-200 • by e-mail technik@eurotec.team

Please contact our technical department or use the free calculation services in the service section of our website.
Contact

| Trader: | Contractor: |
| :---: | :---: |
| Contact person: | Contact person: |
| e-mail: | Phone: |
| Project: | e-mail: |
| Project details |  |



[^8]
## FURTHER PRODUCTS

| Liffing anchor und ball supporting bolt | $128-139$ |
| :--- | :--- |
| Ideefix | $140-147$ |
| Transport anchor system | $148-149$ |
| SonoTec | $150-161$ |
| Bolt anchor | $162-165$ |
| Silent EPDM decoupling profile | $166-167$ |
| Ecktec | $168-169$ |

## LIFTING ANCHOR AND BALL SUPPORTING BOLT

For the transport of prefabricated wall modules

The Liffing anchor is specifically designed for use with a ball supporting bolt. The liffing anchor can be used to transport prefabricated wall modules. The fact that it is sed with screws means the anchor can be used several times. 8 screws are included in delivery.

The product only works in combination with the ball supporting bolt ( $\varnothing$ : $20 \mathrm{~mm}, \mathrm{l}: 50 \mathrm{~mm}$ ) provided for this purpose. The specifications of the product data sheet must be observed! Please consult with our technical department and download the product data sheet from our website www.eurotec.team/en.

Liffing anchor


| Art. no. | Designation | Dimensions $[\mathrm{mm}]^{\text {a) }}$ | Material | PU $^{*}$ |
| :--- | :--- | :--- | :--- | ---: |
| 944892 | Lifting Anchor | $60 \times 40$ | $\mathrm{SJ235}$ | 4 |
| a) Height x Diameter  <br>   <br> *Comes supplied with screwsn  |  |  |  |  |


| Art. no. | Designation | Dimensions [mm] ${ }^{\text {a/ }}$ | Material | Fl [kN] | F2 [kN] | F3 [kN] | PU |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| 948893 | Ball supporing bolt | $50 \times 20$ | S/235 | 10 | 8,5 | 6,5 | 1 |
| a) Height X Diameter |  |  |  |  |  |  |  |

## Please note <br> This product is subject to important conditions! Please also watch the Application video and follow the instructions for use.

LIFTING ANCHOR
APPLICATION VIDEO




## LIFTING ANCHOR

## TECHNICAL INFORMATION

Horizontal wall or beam: Set upright, then lift

| CII- wall or heam |  |  |  |
| :---: | :---: | :---: | :---: |
| Connection in the | Connector | Stop bracket $\beta$ | Total weight [kg] wihh 2 strands |
| End grain area | Lifing anchor $940 \mathrm{~mm}+8 \times$ VSS $6 \times 60$ | $30^{\circ}$ | 44 |
|  |  | $45^{\circ}$ | 528 |
|  |  | $60^{\circ}$ | 569 |
|  |  | $75^{\circ}$ | 588 |
|  |  | $\beta$ | with nstruns |
|  |  | $90^{\circ}$ | nx 297 |



Note
The tables illustrate the 'Setting upright and subsequently lifting a horizontal wall or horizontal beam' load case (lifting from a horizontal position leading to vertical suspension). The connectors are to be screwed flush, as well as at right angles to the surfaces of the narrow sides and side or end grain surfaces, into the centre plane of the components.

## TECHNICAL INFORMATION

Vertical wall or beam: Lift

| CLI - wall or beam |  |  |  |
| :---: | :---: | :---: | :---: |
| Connection in the | Connector | Stop bracket $\beta$ | Total weight [kg] with 2 strands |
|  |  | $30^{\circ}$ | 601 |
|  |  | $45^{\circ}$ | 886 |
| Norrow surface | Lifting Anchor $040 \mathrm{~mm}+8 \times 1 / 5 S 6 \times 60$ | $60^{\circ}$ | 1135 |
| Narrow surface | Lifing Anchor $040 \mathrm{~mm}+8 \times 1556 \times 60$ | $75^{\circ}$ | 1311 |
|  |  | $\beta$ | with nstrands |
|  |  | $90^{\circ}$ | nx 688 |

## Note

The tables illustrate an example of "Liffing a standing wall or beam". Lliffing from the horizontal to vertical suspension). The table values are only valid for lifting or assembly states.

## Ceiling lying: Liffing


(Table on the next pages)
Please note: Verify the assumptions made. The stated values, and type and number of joining devices are based on preliminary measurements. Projects are to be dimensioned exclusively by authorised persons in accordance with the State Building Code. As per LBauO, please contact a qualified structural engineer for a paid proof of stability. We will be happy to refer you to someone.

Eurotec | Further products


## Notes

The tables illustrates an example of "lifting of horizontal ceiling elements". (Liffing from the horizontal to vertical suspension). The connectors must be screwed in flush with the surface, plus perpendicular to the component surface.

## OPERATING INSTRUCTIONS FOR THE BALL SUPPORTING BOLT

## Warning!

Ball supporting bolts are designed for lifting and holding individual loads (not people!). In addition, they are not suitable for continuous load rotation. Contamination (e.g. grinding sludge, oil and emulsion deposits, dust, etc.) can impair the function of ball supporting bolts.

Damaged ball supporting bolts can put people's lives at risk. Before each use, ball supporting bolts must be inspected for visible defects (e.g. deformations, fractures, cracks, damage, missing balls, corrosion, function of the unlocking mechanism). Damaged ball supporting bolts must be mitdrawn from further use.


## Handling and loading

Press the button (A) to release the balls. The balls are locked again by releasing the button (A).
Please note: The button $(A)$ is locked when the spring force has caused it to spring back to its original position. Do not press the button when loaded!
The load values F1 / F2 / F3 (see page 2) apply to lifting in a steel receptacle and x min.
$=1,5 \mathrm{~mm}$.

## Maintenance

Ball supporting bolts must be subjected to a safety inspection by a competent person at least once a year.

## Visual inspection

Deformations, fractures, cracks, missing / damaged balls, corrosion, screw connection damage on the shackle.

## Functional test

The balls' locking and unlocking mechanism must close automatically by spring force.
Full shackle mobility is guaranteed.


| $d_{1}$ | 1 | $d_{2}$ | $d_{3}$ | $d_{4} \mathrm{~min}$. | $I_{2}$ | 13 | 14 | 15 | 16 | 17 | 18 | x min.* | x max.* | D HII | $\mathrm{F}_{1} \mathrm{kN} *$ | $\mathrm{F}_{2} \mathrm{kN} *$ | $\mathrm{F}_{3} \mathrm{kN}{ }^{*}$ |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| 20,0 | 50 | 24,50 | 30,0 | 25,00 | 19,70 | 36,5 | 52,0 | 32,6 | 36 | 56 | 114,0 | 1,5 | 25 | 20,0 | 10,0 | 8,5 | 6,5 |
| *with five-fold protection against breakage |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |

## Original EC conformity mark

The product complies with the regulations set down in the EC Directive 2006/42/EC.

| Make: | Ball supporting bolt |
| :--- | :--- |
| Type: | EH 22350 |
| Applied standards: | DIN EN 13155 |

[^9]
## LIFTING ANCHOR MINI AND BALL SUPPORTING BOLT

For transporting small elements

The Lifting Anchor Mini is especially suitable for transporting smaller loads such as beam girders or struts. Since the inner diameter has been reduced from $\varnothing 20 \mathrm{~mm}$ (Lifting Anchor) to $\varnothing 16 \mathrm{~mm}$ (Liffing Anchor Mini), there is also a new smaller ball supporting bolt.
A special feature of the lifting anchor mini is a stop on the upper edge, which simplifies installation if the hole is drilled through.

Liffing Anchor Mini


| Art. no. | Designation | Dimensions $[\mathrm{mm}]^{\text {a) }}$ | Material | Number of screws* | PU |
| :--- | :--- | :--- | :--- | :--- | :--- |
| 944901 | Lifing Anchor Mini | $49 \times 45$ | S235JR | 8 | 4 |

a) Height $x$ Diameter
*Incl. 8 TX25 fully threaded screws TX25 6,0 x 60

| Art. no. | Designation | Dimensions [mm] ${ }^{\text {a/ }}$ | Material | Fl <br> [kN] | F2 <br> [kN] | F3 <br> [kN] | PU |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| 944905 <br> a) Height x Diameter | Boll supporting bolt | $25 \times 16$ | SJ235 | 4,8 | 4,5 | 4,1 | 1 |



## LIFTING ANCHOR MINI

## TECHNICAL INFORMATION

Horizontal wall or beam: Set upright, then lift

| CII-Wall or beam |  |  |  |
| :---: | :---: | :---: | :---: |
| Connection in the | Connetor | Stop bracket | Total weight [kg] |
|  |  | $\beta$ | with 2 strands |
| End grain area | Lifting anchor mini $1040 \mathrm{~mm}+8 \times$ VSS $6 \times 60$ | $30^{\circ}$ | 248 |
|  |  | $45^{\circ}$ | 295 |
|  |  | $60^{\circ}$ | 318 |
|  |  | $75^{\circ}$ | ${ }^{328}$ |
|  |  | $\beta$ | with nstrons |
|  |  | $90^{\circ}$ | nx166 |




## Note

The tables illustrate the 'Setting upright and subsequently lifting a horizontal wall or horizontal beam' load case (liffing from a horizontal position leading to vertical suspension). The connectors are to be screwed flush, as well as at right angles to the surfaces of the narrow sides and side or end grain surfaces, into the centre plane of the components.

## TECHNICAL INFORMATION

Wand oder Träger stehend: Anheben

| CII - Wall or beam |  |  |  |
| :---: | :---: | :---: | :---: |
| Connection in the | Connetor | Stop bracket | Total weight [kg] |
|  |  | - | with 2 strands |
| Narrow sufface | Lifting anchor mini $1040 \mathrm{~mm}+8 \times$ VSS $6 \times 60$ | $30^{\circ}$ | 360 |
|  |  | $45^{\circ}$ | 585 |
|  |  | $60^{\circ}$ | 869 |
|  |  | $75^{\circ}$ | $11 \%$ |
|  |  | $\beta$ | with ntrund |
|  |  | $90^{\circ}$ | nx688 |

Ceiling lying: Liffing

(Table on the next page)
Please note: Verify the assumptions made. The stated values, and type and number of joining devices are based on preliminary meassurements. Projects are to be dimensioned exclusively by cuthorised persons in accordance with the State Building Code. As per LBauO, please contact a qualified structural engineer for a paid proof of stability. We will be happy to refer you to someone.

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## Note

The tables illustrate an example of "lifing of horizontal ceiling elements". (lifing from the horizontal to vertical suspension). The connectors must be screwed in flush with the surface, plus perpendicular to the component surface.

## OPERATING INSTRUCTIONS FOR THE BALL SUPPORTING BOLT

## Warning!

Ball supporting bolts are designed for lifting and holding individual loads (not people!). In addition, they are not suitable for continuous load rotation. Contamination (e.g. grinding sludge, oil and emulsion deposits, dust, etc.) can impair the function of ball supporting bolts.

Damaged ball supporting bolts can put people's lives at risk. Before each use, ball supporting bolts must be inspected for visible defects (e.g. deformations, fractures, cracks, damage, missing balls, corrosion, function of the unlocking mechanism).
Damaged ball supporting bolts must be withdrawn from further use.

## Handling and loading

Press the button (A) to release the balls. The balls are locked again by releasing the button (A).
Please note: The button $(A)$ is locked when the spring force has caused it to spring back to its original position. Do not press the button when loaded!
The load values F1 / F2 / F3 (see page 2) apply to liffing in a steel receptacle and x min. $=1.5 \mathrm{~mm}$

## Maintenance

Ball supporting bolts must be subjected to a safety inspection by a competent person at
least once a year.

## Visual inspection

Deformations, fractures, cracks, missing / damaged balls, corrosion, screw connection damage on the shackle.

## Functional test

The balls' locking and unlocking mechanism must close automatically by spring force.
Full shackle mobility is guaranteed.


| $d_{1}$ | 1 | $\mathrm{d}_{2}$ | $d_{3}$ | $d_{4}$ min. | $\mathrm{I}_{2}$ | 13 | $I_{4}$ | $\mathrm{I}_{5}$ | 16 | $I_{7}$ | 18 | x min.* | x max.* | DHII | $\mathrm{F}_{1} \mathrm{kN}{ }^{*}$ | $F_{2} k N^{*}$ | $\mathrm{F}_{3} \mathrm{kN}{ }^{*}$ |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
|  | 50 | 24,50 | $30,0$ | 25,00 | 19,70 | 36,5 | 52,0 | 32,6 | 36 | 56 | 114,0 | 1,5 | 25 | 20,0 | 10,0 | 8,5 | 6,5 |

*with five-fold protection against breakage

Original EC conformity mark
The product complies with the regulations set down in the EC Directive 2006/42/EG.

| Make: | Ball supporting bolt |
| :--- | :--- |
| Type: | EH 22350 |
| Applied standards: | DIN EN 13155 |

Please note: Verify the assumptions made. The stated values, and type and number of joining devices are based on preliminary measurements. Projects are to be dimensioned exclusively by outhorised persons in accordance wiht the State Building Code. As per LBauO, please contact a qualified structural engineer for a paid proof of stability. We will be happy to refer you to someone.

## IDEEFIX

Hidden wood connector

The IdeeFix wood connector is used to create hidden wood connections for single- or multiple-row serial connections in wood-wood connections. It ensures high load-bearing capacity for tensile and transverse forces, is designed for universal use and is quick and easy to mount.


## INSTRUCTIONS FOR USE

The wood is predrilled for the IdeeFix. Then the IdeeFix is first inserted into the drill hole without screws. Then, thanks to its low splitting effect, the screws can be inserted without further predrilling. In the middle of the IdeeFix is a thread into which another screw can be inserted.



2
Insert and install supplied screws
3 Fix construction in place with construction screws - Done!



Ideefix application for connecting column and beam girder

## IDEEFIX 30/40/50

Technical information


| IdeeFix |  |  | Timber <br> Dimensions <br> Min. cross section post |  | Tension connection with anti--wist element |  | Mortise joint with anti--wist element |  | Tensile lood with threaded bolt |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
|  | sions |  |  |  | Drilling depth for post | Drilling depth for cross-piece | Drilling depth for post | Drilling depth for cross-piece | Perm. Values | Char. Values | Screw pattern |
| $d_{c}$ | $\mathrm{ag}_{\mathrm{g}}$ | $V_{c}$ | [mm] | [mm] | [mm] | [mm] | [mm] | [mm] | $\mathrm{Nze}_{\text {e }}$ [ [kN] | $\mathrm{Rl}_{1,1 \mathrm{k}}$ [kN] | pc. |
| 30 | M12 | 3 | 80 | 80 | 27 | . | 20 | 7 | 7,62 | 17,33 |  |
| 40 | M16 | 5 | 120 | 120 | 35 | - | 25 | 10 | 12,65 | 28,79 | $\bigcirc$ |
| 50 | M20 | 5 | 160 | 160 | 45 | - | 30 | 15 | 20,81 | 47,35 |  |
| 30 | M12 | 3 | 60 | 80 | 27 | . | 20 | 7 | 5,71 | 13,00 |  |
| 40 | M16 | 5 | 80 | 120 | 35 | . | 25 | 10 | 9,49 | 21,59 | $\infty$ |
| 50 | M20 | 5 | 120 | 160 | 45 | . | 30 | 15 | 15,61 | 35,51 |  |
| 30 | M12 | 3 | 40 | 80 | 27 | . | 20 | 7 | 3,81 | 8,67 |  |
| 40 | M16 | 5 | 60 | 120 | 35 | . | 25 | 10 | 6,33 | 14,39 | 88 |
| 50 | M20 | 5 | 80 | 160 | 45 | - | 30 | 15 | 10,41 | 23,67 |  |
| 30 | M12 | 3 | 60 | 60 | 27 | . | 20 | 7 | 3,81 | 8,67 |  |
| 40 | M16 | 5 | 80 | 80 | 35 | - | 25 | 10 | 6,33 | 14,39 | c |
| 50 | M20 | 5 | 120 | 120 | 45 | - | 30 | 15 | 10,41 | 23,67 |  |

dc is the diameter and the total heightit of the connector
ag is the metric connection thread of the connector
vc is the height of the integrated anti-wwis system
Fully threaded screw, GoFix ${ }^{\mathbb{}}$ FK IF $305,0 \times 40 \mathrm{~mm}$ - IF $406,0 \times 60 \mathrm{~mm}$ - IF $508,0 \times 90 \mathrm{~mm}$
The connection is drawn together using a threaded rod or construction screw with a DIN 440 R washer
Tension connection as a mortise joint with simultaneous absorption of transverse forces
Rk characteristic value calculated according to DIN $1052: 2004$-08 Timber pk $380 \mathrm{~kg} / \mathrm{m}^{3} \mathrm{Nze}$. recommended permissible load $\mathrm{R}, \mathrm{k} \times 0,8 \mathrm{kmod}: 1,3 \mathrm{ym}: 1,4$. Factor 1,4 average load safety factor
Please note: The stated values are planning aids. Projects must only be calculated by authorised persons.

## MAIN-SECONDARY BEAM


de is the diameter and the total height of the connector
ag is the metric connection thread of the connector
Vc s s the height of the integrated anti--wist system
System - Fully threaded screw, GoFix ${ }^{(2)}$ FK IF $305,0 \times 40 \mathrm{~mm}$ - IF $406,0 \times 60 \mathrm{~mm}$ - IF $508,0 \times 90 \mathrm{~mm}$
The connection is drown together using a threaded rod or construction screw with a DIN 440 R washer
MB-SB connection as a mortise joint with simultaneous absorption of tensile forces
Rk characteristic value calculated according to DIN $1052: 2004-08$ Timber pk $380 \mathrm{~kg} / \mathrm{m}^{3} \mathrm{Nze}$. recommended permissible load $\mathrm{R}, \mathrm{k} \times 0,8 \mathrm{kmod}: 1,3 \mathrm{ym}: 1,4$. Factor 1,4 average lood safety factor
Please note: The stated values are planning aids. Projects must only be calculated by authorised persons.

MAIN-SECONDARY BEAM,
DOUBLE-SIDED CONNECTION, WITH FIXING SCREW


| IdeeFix |  |  | Timber <br> Dimensions <br> Min. cross section <br> of secondary beam |  | Timber <br> Dimensions <br> Min. cross section of main beam |  | Main-secondary beam with anti-twist element |  | Lood-bearing capacity with threaded bolt |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
|  | sions |  |  |  | Drilling depth for SB | Drilling depth for MB | Perm. Values | Char. Values | Screw pattern |
| $d_{c}$ | $a_{g}$ | $V_{c}$ | w [mm] | h [mm] |  |  | w [mm] | $\mathrm{h}[\mathrm{mm}]$ | [mm] | [mm] | $V_{z e}$. [kN] | R23,k[ ${ }^{\text {cNW }}$ ] | pc. |
| 30 | M12 | 3 | 80 | 80 | 80 | 80 | 20 | 10 | 2,34 | 5,32 | 1 |
| 40 | M16 | 5 | 120 | 120 | 120 | 120 | 25 | 15 | 3,60 | 8,19 |  |
| 50 | M20 | 5 | 160 | 160 | 160 | 160 | 30 | 20 | 5,03 | 11,44 |  |
| 30 | M12 | 3 | 60 | 80 | 60 | 80 | 20 | 10 | 2,34 | 5,32 | 1 |
| 40 | M16 | 5 | 80 | 120 | 80 | 120 | 25 | 15 | 3,60 | 8,19 |  |
| 50 | M20 | 5 | 120 | 160 | 120 | 160 | 30 | 20 | 5,03 | 11,44 |  |
| 30 | M12 | 3 | 40 | 80 | 40 | 80 | 20 | 10 | 2,34 | 5,32 |  |
| 40 | M16 | 5 | 60 | 120 | 60 | 120 | 25 | 15 | 3,60 | 8,19 | 88 |
| 50 | M20 | 5 | 80 | 160 | 80 | 160 | 30 | 20 | 5,03 | 11,44 |  |
| 30 | M12 | 3 | 60 | 60 | 60 | 60 | 20 | 10 | 2,34 | 5,32 |  |
| 40 | M16 | 5 | 80 | 80 | 80 | 80 | 25 | 15 | 3,60 | 8,19 | (8) |
| 50 | M20 | 5 | 120 | 120 | 120 | 120 | 30 | 20 | 5,03 | 11,44 |  |

## de is the diameter and the total height of the connectior

ag is the metric connection thread of the connector
Vcis the height of the integrated anti-wwist system
System - Fully threaded screw, Gofix ${ }^{\circledR}$ FK IF $305,0 \times 40 \mathrm{~mm}$ - IF $406,0 \times 60 \mathrm{~mm}$ - IF $508,0 \times 90 \mathrm{~mm}$
Position retention using Gofix ${ }^{\text {® }}$ SK IF $305,0 \times 100 \mathrm{~mm}$, IF $406,0 \times 140 \mathrm{~mm}$, IF $508,0 \times 160 \mathrm{~mm}$
MB-SB connection as mortise joint for double-sided connection of secondary beam
Rk characteristic value calculated according to DIN $1052: 2004-08$ Timber pk $380 \mathrm{~kg} / \mathrm{m}^{3} \mathrm{Nze}$. recommended permissible load $\mathrm{R}, \mathrm{k} \times 0,8 \mathrm{kmod}: 1,3 \mathrm{ym}: 1,4$. Favtor 1,4 average lood safety factor
Please note: The stated values are planning aids. Projects must only be calculated by authorised persons.

## MAIN-SECONDARY BEAM MULTIPLE CONNECTION, SINGLE-ROW



| IdeeFix |  |  | Timber Dimensions <br> Min. cross section of secondary beam |  | Edge and centre distance |  | Main-secondary beam Multiple connection |  | Load-bearing capacity Single-row |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
|  | ns |  |  |  | Edge <br> distance | Centre distance | Drilling depth for SB | Drilling depth for MB | Perm. Values | Char. Values | Number of Connectors |
| $d_{c}$ | $a_{g}$ | $V_{C}$ | W [mm] | h [mm] | [mm] | [mm] | [mm] | [mm] | Ve. [kN] | $\mathrm{R}_{23, \mathrm{k}}[\mathrm{kN}]$ | pc. |
| 30 | M12 | 3 | 80 | 80 | 50 | 50 | 20 | 7 | 4,32 | 8,94 | 1 |
| 40 | M16 | 5 | 120 | 120 | 60 | 60 | 25 | 10 | 6,98 | 14,66 | 1 |
| 50 | M20 | 5 | 160 | 160 | 80 | 80 | 30 | 15 | 10,88 | 21,09 | 1 |
| 30 | M12 | 3 | 80 | 150 | 50 | 50 | 20 | 10 | 8,64 | 17,88 | 2 |
| 40 | M16 | 5 | 120 | 180 | 60 | 60 | 25 | 15 | 13,96 | 29,32 | 2 |
| 50 | M20 | 5 | 160 | 240 | 80 | 80 | 30 | 20 | 21,76 | 42,18 | 2 |
| 30 | M12 | 3 | 80 | 200 | 50 | 50 | 20 | 10 | 12,96 | 26,82 | 3 |
| 40 | M16 | 5 | 120 | 240 | 60 | 60 | 25 | 15 | 20,94 | 43,98 | 3 |
| 50 | M20 | 5 | 160 | 320 | 80 | 80 | 30 | 20 | 32,64 | 63,27 | 3 |
| 30 | M12 | 3 | 80 | 250 | 50 | 50 | 20 | 10 | 17,28 | 35,76 | 4 |
| 40 | M16 | 5 | 120 | 300 | 60 | 60 | 25 | 15 | 27,92 | 58,64 | 4 |
| 50 | M20 | 5 | 160 | 400 | 80 | 80 | 30 | 20 | 43,52 | 84,36 | 4 |
| 30 | M12 | 3 | 80 | 300 | 50 | 50 | 20 | 10 | 21,60 | 44,70 | 5 |
| 40 | M16 | 5 | 120 | 360 | 60 | 60 | 25 | 15 | 34,90 | 73,30 | 5 |
| 50 | M20 | 5 | 160 | 480 | 80 | 80 | 30 | 20 | 54,40 | 105,45 | 5 |
| 30 | M12 | 3 | 80 | 350 | 50 | 50 | 20 | 10 | 25,92 | 53,64 | 6 |
| 40 | M16 | 5 | 120 | 420 | 60 | 60 | 25 | 15 | 41,88 | 87,96 | 6 |
| 50 | M20 | 5 | 160 | 560 | 80 | 80 | 30 | 20 | 65,28 | 126,54 | 6 |
| 30 | M12 | 3 | 80 | 400 | 50 | 50 | 20 | 10 | 30,24 | 62,58 | 7 |
| 40 | M16 | 5 | 120 | 480 | 60 | 60 | 25 | 15 | 48,86 | 102,62 | 7 |
| 50 | M20 | 5 | 160 | 640 | 80 | 80 | 30 | 20 | 76,16 | 117,63 | 7 |
| 30 | M12 | 3 | 80 | 450 | 50 | 50 | 20 | 10 | 34,56 | 71,52 | 8 |
| 40 | M16 | 5 | 120 | 540 | 60 | 60 | 25 | 15 | 55,84 | 117,28 | 8 |
| 50 | M20 | 5 | 160 | 720 | 80 | 80 | 30 | 20 | 87,04 | 168,72 | 8 |

[^10]

| IdeeFix |  |  | Timber <br> Dimensions <br> Min. cross section of secondary beam |  | Edge and centre distance |  | Main-secondary beam Multiple connection |  | Load-bearing capacity Single-row |  | $\theta 0$ |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
|  | sions [ |  |  |  | Edge distance | Centre distance | Drilling depth for SB | Drilling depth for MB | Perm. Values | Char. Values | Number of connectors |
| $d_{c}$ | $\mathrm{ag}_{\mathrm{g}}$ | $V_{c}$ | w [mm] | h [mm] | [mm] | [mm] | [mm] | [mm] | $V_{z e}$. [kN] | R23,k[kN] | pc. |
| 30 | M12 | 3 | 150 | 80 | 50 | 50 | 20 | 10 | 8,64 | 17,88 | 2 |
| 40 | M16 | 5 | 180 | 120 | 60 | 60 | 25 | 15 | 13,6 | 29,32 | 2 |
| 50 | M20 | 5 | 240 | 160 | 80 | 80 | 30 | 20 | 21,76 | 42,18 | 2 |
| 30 | M12 | 3 | 150 | 150 | 50 | 50 | 20 | 10 | 17,28 | 35,76 | 4 |
| 40 | M16 | 5 | 180 | 180 | 60 | 60 | 25 | 15 | 27,92 | 58,64 | 4 |
| 50 | M20 | 5 | 240 | 240 | 80 | 80 | 30 | 20 | 43,52 | 84,36 | 4 |
| 30 | M12 | 3 | 150 | 200 | 50 | 50 | 20 | 10 | 25,92 | 53,64 | 6 |
| 40 | M16 | 5 | 180 | 240 | 60 | 60 | 25 | 15 | 41,88 | 87,96 | 6 |
| 50 | M20 | 5 | 240 | 320 | 80 | 80 | 30 | 20 | 65,28 | 126,54 | 6 |
| 30 | M12 | 3 | 150 | 250 | 50 | 50 | 20 | 10 | 34,56 | 71,52 | 8 |
| 40 | M16 | 5 | 180 | 300 | 60 | 60 | 25 | 15 | 55,84 | 117,28 | 8 |
| 50 | M20 | 5 | 240 | 400 | 80 | 80 | 30 | 20 | 87,04 | 168,72 | 8 |


| 30 | M12 | 3 | 150 | 300 | 50 | 50 | 20 | 10 | 43,20 | 89,40 |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| 40 | M16 | 5 | 180 | 360 | 60 | 60 | 25 | 15 | 69,80 | 146,60 |
| 50 | M20 | 5 | 240 | 480 | 80 | 80 | 30 | 20 | 108,80 | 210,90 |
| 30 | M12 | 3 | 150 | 350 | 50 | 50 | 20 | 10 | 51,84 | 1007,28 |
| 40 | M16 | 5 | 180 | 420 | 60 | 60 | 25 | 15 | 83,76 | 175,92 |
| 50 | M20 | 5 | 240 | 560 | 80 | 80 | 30 | 20 | 130,56 | 253,08 |


| 30 | M12 | 3 | 150 | 400 | 50 | 50 | 20 | 10 | 60,48 | 125,16 |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| 40 | M16 | 5 | 180 | 480 | 60 | 60 | 25 | 15 | 97,72 | 205,24 |
| 50 | M20 | 5 | 240 | 640 | 80 | 80 | 30 | 20 | 152,32 | 295,26 |
|  | M12 | 3 | 150 | 450 | 50 | 50 | 20 | 10 | 14 |  |
| 30 | 3 | 69,12 | 143,04 | 16 |  |  |  |  |  |  |
| 40 | M16 | 5 | 180 | 540 | 60 | 60 | 25 | 15 | 111,68 | 234,56 |
| 50 | M20 | 5 | 240 | 720 | 80 | 80 | 30 | 20 | 174,08 | 337,44 |

## de is the diameter and the tota height of the connector

## ag is the metric connection thread of the connector <br> vcis the height of the integrated anti--wwiss system

Fully threaded screw, GoFix ${ }^{\circledR}$ FK IF $305,0 \times 40 \mathrm{~mm}$ - IF $406,0 \times 60 \mathrm{~mm}$ - IF $508,0 \times 90 \mathrm{~mm}$
The connection is drown together using a threaded rod or constructionscrew with a DIN 440 R washer
MB-SB connection as a mortise joint with simultaneous absorption of tensile forces
Rk characteristic value calculated according to DIN $1052: 2004-08$ Timber pk $380 \mathrm{~kg} / \mathrm{m}^{3} \mathrm{Nze}$. recommended permissible lood R,k $\times 0,8 \mathrm{kmod}: 1,3 \mathrm{ym}$ : 1,4 . Factor 1,4 average load safety factor
Please note: The stated values are planning cids. Projects must only be calculated by cuthorised persons.


## TRANSPORT ANCHOR SYSTEM

## Transport anchor and transport anchor screws - The secure liffing system

Made of high-grade steel, this liffing attachment is used to lift all kinds of timber parts safely and easily. The transport anchors of the load group up to 1,3 tonnes are strictly to be used only in conjunction with the $\varnothing 11 \times 125 \mathrm{~mm}$ and $\varnothing 11 \times 160 \mathrm{~mm}$ Eurotec transport anchorscrews. The Eurotec transport anchor screws must only be used once. They are to be screwed into solid wood (soffwood), laminated veneer timber, glued laminated timber, cross laminated timber, stacked planks and laminated joists without pilot-drilling. Use in hardwoods is not permitted. The possible, or rather permissible, assembly positions can be found in our operating instructions, of which we will be delighted to provide you with a copy.

## Transport anchor

High-quality steel


| Art. no. | Dimensions $[\mathrm{mm}]^{\text {a) }}$ | Load group | PU* $^{*}$ |
| :--- | :--- | :--- | ---: |
| 110361 | $190 \times 70$ | up to 1,3 to | 2 |
| a) Length x width <br> $*$ Screws must be ordered separately (see below) |  |  |  |

## PLEASE NOTE

- Transport anchor screws must only be used once
- Insert the screws without pilot-drilling
- Read the operating instructions in detail before use
- Users are to be trained before beginning use for the first time
- The transport anchor is to be examined for damage before each use and rejected if necessary
- The weight of the component to be lifted must not exceed the permissible value
- At least two altachment points per component to be lifted

| Permissible liffing lood ${ }^{(a)}$ per attachment point ${ }^{\text {b/ }}$ |  |  |  |  |
| :---: | :---: | :---: | :---: | :---: |
|  | $\gamma^{\prime \prime}$ | $\alpha^{\text {d) }}$ | $11 \times 125 \mathrm{~mm}$ | $11 \times 160 \mathrm{~mm}$ |
| Avil tenson | $60^{\circ}$ | $60^{\circ}$ | 533 kg | 603 kg |
| Axxil fension | $60^{\circ}$ | $30^{\circ}$ | 409 kg | 462 kg |
| yonal tension | $60^{\circ}$ | $90^{\circ}$ | 462 kg | 522 kg |
| Sugar | $60^{\circ}$ | $0^{\circ}$ | 139 kg | 157 kg |

[^11]| \% C | Art. no. | Dimensions [mm] | Head | PU |
| :---: | :---: | :---: | :---: | :---: |
| 为 | 110359 | $11 \times 125$ | SW17 | 20 |
|  | 110360 | $11 \times 160$ | SW17 | 20 |



Transport anchor system for safe transport.

## SONOTEC SOUND INSULATION CORK

The perfect solution for sound insulation


| Art. no. | Designation | Dimensions [mm] | Material thickness [mm] | PU |
| :--- | :--- | :--- | :--- | :--- |
| 945305 | SK02 | $80 \times 1100$ | 6 | 20 |
| 945306 | SK02 | $100 \times 1100$ | 6 | 20 |

SonoTec sound insulation cork


| Art. no. | Designation | Dimensions [mm] | Material thickness [mm] | PU |
| :--- | :--- | :--- | :--- | :--- |
| 945307 | SKO3 | $80 \times 1100$ | 6 | 20 |
| 945308 | SK03 | $100 \times 1100$ | 6 | 20 |

SonoTec sound insulation cork
Material: SK04



## ADVANTAGES

Sustainable material
High load bearing capacity
Hidden installation
Easy to use
Impermeable to water and gas due to component-
specific requirements

## LOAD ABSORPTION

Different loads have to be absorbed when decoupling the timber vertical truss from the concrete. These are located in the $0,1 \mathrm{~N} / \mathrm{mm}^{2}-3 \mathrm{~N} / \mathrm{mm}^{2}$ stat. permanent load range. A timber beam (C24 softwood) may only be loaded up to $2,5 \mathrm{~N} / \mathrm{mm}^{2}$ (characteristic) perpendicular to the grain. Our products cover load cases from $0,1 \mathrm{~N} / \mathrm{mm}^{2}-3 \mathrm{~N} / \mathrm{mm}^{2}$ ab. The cork can thus be used both in lightweight and solid construction with cross-laminated timber (CLT).

## NOISE REDUCTION

The SonoTec sound insulation cork can reduce noise by up to 40 dB .

## MATERIAL

The SonoTec sound insulation cork is a combination of the components cork and natural rubber. This product is suitable for the application of vibration damping where very high isolation values are required and can be used as invisible insulators (pads/strips) with a low resonant frequency and medium to low load.

## SONOTEC SOUND INSULATION CORK FOR VARIOUS APPLICATIONS

The perfect solution for sound insulation


Different SonoTec decoupling profiles variations for shearing angles
CLT system angle

| Art. no. | Dimensions [mm] | Material | Can be combined with |  | PU |
| :---: | :---: | :---: | :---: | :---: | :---: |
|  |  |  | Art-No. | Name |  |
| 945311 | $230 \times 70 \times 6$ | SkO4 | 954088 | HH flat shearing ongle | 5 |
| 945312 | $230 \times 80 \times 6$ | SKO4 | 954180 | CII sysiem angle | 5 |
| 945314 | $230 \times 100 \times 6$ | SKO4 | 954087 | HB flat Shearing ongle | 5 |
| 945313 | $230 \times 120 \times 6$ | SKO4 | 954112 | Shearing angle $120 \times 230$ | 5 |

TECHNICAL DATA

|  | SKO2 | SKO3 | SKOO |
| :---: | :---: | :---: | :---: |
| Temperature $\left[{ }^{\circ} \mathrm{C}\right] /$ span width | $10 /+100$ | Load ranges $\left[\mathrm{N} / \mathrm{mm}^{2}\right]$ | $-10 /+100$ |
| Density $\left[\mathrm{kg} / \mathrm{m}^{3}\right]$ | 700 | 1100 | 1125 |
| Shore hardness $[\mathrm{shore} \mathrm{A}]$ | $35-50$ | $45-60$ | $60-80$ |
| Break rotatio $[\%]$ | $>200$ | $>300$ | $>100$ |
| Tensile strength $\left[\mathrm{N} / \mathrm{mm}^{2}\right]$ | $>2,0$ | $>5,0$ | $>6,0$ |
| compression $23^{\circ} \mathrm{C} / 70 \mathrm{~h}[\%]$ | $<15$ | $<15$ | $<15$ |



## IDENTIFYING THE CORRECT MATERIAL: AN EXAMPLE

We precisely identify the right material for you. So you still get an idea of how the right material is identified, we have outlined a sample identification process for you below.

First of all, we need the static continuous load that the sound insulation cork is to absorb. This is specified by the architect, structural engineer or stress analyst in question.

One of three different materials is selected depending on the static continuous load:


Please note: Verify the assumptions made. The stated values, and type and number of joining devices are based on preliminary measurements. Projects are to be dimensioned exclusively by outhorised persons in accordance with the State Building Code. As per LBauO, please contact a qualified structural engineer for a paid proof of stability. We will be happy to refer you to someone.

| Static continuous load $\mathrm{N} / \mathrm{mm}^{2}$ | Product | Dimensions [mm] | Art. no. |
| :---: | :---: | :---: | :---: |
| $0,10-0,39$ | SKO2 | $80 \times 1100$ | 945305 |
| $0,10-0,39$ | SKO2 | $100 \times 1100$ | 945306 |
| $0,40-1,40$ | SKO3 | $80 \times 1100$ | 945307 |
| $0,40-1,40$ | SKO3 | $100 \times 1100$ | 945308 |
| $1,50-3,10$ | SKO4 | $80 \times 1100$ | 945309 |
| $1,50-3,10$ | SKO4 | $100 \times 1100$ | 945310 |

In the second step, the material's natural frequency is determined; this depends on the occurring load. The values are approximately taken from the following table.

|  |  | 6 mm |  |  | 12 mm |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
|  | Continuous load [ $\mathrm{N} / \mathrm{mm}^{2}$ ] | Natural frequency <br> [ Hz ] | Deflection [mm] | Modulus of elasicity $10 \mathrm{~Hz}\left[\mathrm{~N} / \mathrm{mm}^{2}\right]$ | Natural frequency <br> [ $\mathrm{Hz}_{z}$ ] | Deflection [mm] | Modulus of elasicity $10 \mathrm{~Hz}\left[\mathrm{~N} / \mathrm{mm}^{2}\right]$ |
| SK02 | 0,1 | 44 | 0,2 | 4,0 | 27 | 0,5 | 3,7 |
|  | 0,2 | 33 | 0,5 | 4,5 | 19 | 1,3 | 4,0 |
|  | 0,3 | 27 | 0,8 | 5,6 | 17 | 1,9 | 5,1 |
|  | 0,4 | 27 | 1,1 | 6,9 | 17 | 2,6 | 6,5 |
| SKO3 | 0,5 | 50 | 0,2 | 11,5 | 31 | 0,4 | 10,5 |
|  | 0,8 | 38 | 0,4 | 15,75 | 22 | 1,0 | 14,0 |
|  | 1,1 | 31 | 0,7 | 19,5 | 20 | 1,6 | 18,0 |
|  | 1,5 | 31 | 0,9 | 28,5 | 20 | 2,2 | 27,0 |
| SKO4 | 1,6 | 58 | 0,3 | 18,5 | 36 | 0,6 | 17,0 |
|  | 2,4 | 44 | 0,6 | 24,5 | 25 | 1,3 | 22,0 |
|  | 3,2 | 35 | 1,0 | 30,5 | 23 | 2,0 | 28,0 |
|  | 4,0 | 35 | 1,5 | 43,0 | 23 | 2,7 | 41,0 |

*Values for SKO2 are based on test results provided by the University of Coimbra / Institute for Research and Technological Development in Construction Sciences. The values for SK03 and SKO4 are generalised. The ongoing tests confirm the values. The results will replace the described values.
As an example, the following sample calculation assumes a load of $0,3 \mathrm{~N} / \mathrm{mm}^{2}$. Our SKO2 material was chosen due to the specified load. From the above table, we can see that the natural frequency must therefore be 27 Hz . We can illustrate this as follows in the graphs below.

## SK02 Natural frequency [Hz]



## Eurotec | Further products

In the next step, we take a closer look at the interference frequency.
To this end, we look at the graphs below and can thus conclude that the sound reduction in the low frequency range has deteriorated.
Low frequencies (basses) can only be isolated by mass. The frequencies to be isolated for building acoustics start in the 80 Hz range, so this is negligible. 80 Hz can be assumed if no interference frequencies are specified.

The sound reduction in dB can be determined in two ways:
1:
Based on an interference frequency of 80 Hz , a sound reduction of approx.
17 dB can be read off the following graph. These values are achieved under ideal conditions (optimum room temperature, room humidity, etc.).


2:
A sound insulation factor can be calculated from the natural frequency identified previously ( 27 Hz ) and the specified interference frequency $(80 \mathrm{~Hz})$.

## Sound insulation factor $\mathrm{f} / \mathrm{fO}$ : Interference frequency / natural frequency $\rightarrow 80 \mathrm{~Hz} / 27 \mathrm{~Hz} \approx 2,96$

The sound reduction can then be read off based on the factor calculated previously. This is 17 dB under ideal conditions.

Please note: Verify the assumptions made. The stated values, and type and number of joining devices are based on preliminary measurements. Projects are to be dimensioned exclusively by outhorised persons in accordance with the State Building Code. As per LBuuO, please contact a qualified structural engineer for a paid proof of stability. We will be happy to refer you to someone.


In the last step, the material's deflection is identified.
This step is particularly important for the building's designers. The deflection is also identified using the continuous load, and there is a separate graph for each material. For the sample calculation with SKO 2 and $0,3 \mathrm{~N} / \mathrm{mm}^{2}$, the following graph shows a deflection of $0,8 \mathrm{~mm}$.
The graphs shown here are naturally adapted to the factors
identified previously.

## SK02 Deflection [mm]



## Gurotec <br> Further products

For our SK03 and SKO4 materials, the following graphs apply to the deflection:

## SK03 Deflection [mm]




Please note: Verify the assumptions made. The stated values, and type and number of joining devices are based on preliminary measurements. Projects are to be dimensioned excusively by outhorised persons in accordance with the State Building Code. As per LBauO, please contact a qualified structural engineer for a paid proof of sability. We will be happy to refer you to someone.


## PROPERTIES OF CORK

The cork bark has a honeycomb-shaped cell structure with over 40 million cells per $\mathrm{cm}^{3}$. The cells have a high proportion of an air-like gas mixture, which results in the cork's low weight on the one hand and the high compression capacity and elasticity on the other. The cork can therefore be compressed by up to half its size and can return to its original shape after compression.

Almost half of the cork bark is made up of suberin, a non-combustible biopolymer. The substance lines the individual cells and makes them impermeable to liquids and gases. The bark's structure and thickness protect the cork oak from heat, drying out and infections. This natural protective insulation makes cork oak an ideal insulating and sealing material for technical purposes.


Very good sound and thermal insulation
Impermeable to liquids and gases
Good resistance to fire and high temperatures
High frictional resistance
Compressible and elastic
Good wear resistance
Low weight - floats on water
Hypoallergenic and anti-static - does not absorb dust High flexibility - comfortable and soft

## ENVIRONMENT

Cork is one of the most natural and environmentally friendly raw materials in the world. Cork oak is also the only tree that can completely regenerate itself after each harvest. The fact that cork can be recycled and ressed in new products makes it an ideal raw material with regard to sustainability.

## NATURAL RUBBER

Alongside cork, natural rubber is another natural and renewable raw material. Natural rubber is a rubber-like substance and is extracted from the milky sap (also known as latex) of the rubber tree. The rubber tree grows in the tropics of Africa, South America and Asia.
Natural rubber accounts for around $40 \%$ of global rubber production. In contrast, synthetic rubber is made using crude oil as a basis and consumes far more energy during the manufacturing and transport processes.

Natural rubber is made into various products, most of them are used in tyre production. Other applications include seals, binders and mattresses.

## PROPERTIES OF NATURAL RUBBER

High level of elasticity
Good mechanical resistance
High tear strength
Water repellent
Poor electrical and thermal performance
Weighs less than water on, we would be delighted to send you our brochure. Alternatively, you will find out more online.

## SONOTEC ANGULAR DECOUPLER

## Perfect complement to the eurotec shearing angles and the CLT system angle

SonoTec angular decoupler


| Art. no. | Dimensions $[\mathrm{mm}]$ | Material | Can be combined with | PU |  |
| :--- | :--- | :--- | :--- | :--- | :--- |
|  |  |  | Art-No. | Name |  |
| 945311 | $230 \times 70 \times 6$ | SKO4 | 954088 | HH flat shearing angle | 5 |
| 945312 | $230 \times 80 \times 6$ | SKO4 | 954180 | CII system angle | 5 |
| 945314 | $230 \times 100 \times 6$ | SKO4 | 954087 | HB flat shearing angle | 5 |
| 945313 | $230 \times 120 \times 6$ | SKO4 | 954112 | Shearing angle $120 \times 230$ | 5 |

The Eurotec SonoTec Angular Decoupler forms the perfect complement to the Eurotec shearing angles and the CLT System Angle. The underlay is made from SKO4, which is a compound formed from cork and natural rubber. The product is suitable for vibration damping applications in which very high insulation values are required. SonoTec angular decouplers are used as invisible insulators (pads/strips) with a low resonance frequency and a medium-low load.

ADVANTAGES

- Underlay enables straightforward assembly
- Sustainable material
- Invisible
- High load-bearing capacity
- REACH-compliant


## DESCRIPTION

SonoTec angular decouplers feature cut-outs for concrete screws, making them suitable for use in concrete. The double layer allows an increase in the separation layer to $\mathbf{1 2 ~ m m}$. The specifications for Sonotec SKO4 Sound Insulation Cork apply. The material can be screwed through when used in wood. The application must be determined in advance by a structural engineer. No statement can be made regarding noise reduction since this is dependent on the construction.


Shearing angle for fixing a wall to the concrete foundation.


[^12]
## BOLT ANCHOR

## For fastening in concrete

ETA-18/0533

CE
Eves loch brontra tuh-22/0631

The Eurotec bolt anchor is a force-controlled expanding anchor for pushthrough installations. The galvanized steel bolt anchor is approved for use in non-cracked concrete, the stainless steel A4 bolt anchor as well as the bolt anchor C3 for both non-cracked and cracked concrete. Despite the high loadbearing capacity, small axial and edge distances can be maintained. Different anchoring depths and dimensions allow a wide range of applications for connecting attachments of various materials to concrete. The A4 bolt anchor can be used both indoors and outdoors, while the galvanized steel and C 3 bolt anchor can only be used indoors. Each bolt anchor is equipped with an expansion clip, which ensures high load-bearing capacity and reduces the number of fastening points required.


| Art. no. | Dimensions [mm] | Spanner gap | PU |
| :--- | :--- | :--- | :--- |
| 946142 | $8,0 \times 75$ | SW13 | 100 |
| 9666143 | $8,0 \times 100$ | SW13 | 100 |
| 946644 | $10,0 \times 100$ | SWW | 50 |
| 946145 | $10,0 \times 120$ | SW17 | 50 |
| 946146 | $10,0 \times 140$ | SW17 | 50 |
| 9466148 | $12,0 \times 140$ | SW19 | 25 |

Bolt anchor
With washer, electrogalvanised, for non-cracked concrete


| Art. no. | Dimensions [mm] | Spanner gap | PU |
| :---: | :---: | :---: | :---: |
| 946170* | 6,0 $\times 55$ | SWIO | 200 |
| 946171** | 6,0 85 | Swio | 100 |
| 946172* | 8,0 $\times 50$ | SWI3 | 100 |
| 946173 | $8,0 \times 75$ | SWI3 | 100 |
| 946174 | $8,0 \times 95$ | SW13 | 100 |
| 946175 | $8,0 \times 115$ | SWI3 | 100 |
| 946176 | $8,0 \times 135$ | SWI3 | 50 |
| 946177* | $10,0 \times 60$ | SW17 | 100 |
| 946178 | 10,0x 80 | SW17 | 50 |
| 946179 | $10,0 \times 100$ | SW17 | 50 |
| 946180 | $10,0 \times 120$ | SW17 | 50 |
| 946181 | $10,0 \times 140$ | SW17 | 50 |
| 946182* | 12,0×80 | SW19 | 50 |
| 946183 | 12,0×95 | SW19 | 50 |
| 946184 | 12,0x 110 | SW19 | 50 |
| 946185 | $12,0 \times 130$ | SW19 | 25 |
| 946186 | $12,0 \times 160$ | SW19 | 25 |
| 946187 | 12,0 $\times 180$ | SW19 | 25 |
| 946188 | $16,0 \times 125$ | SW24 | 20 |
| 946189 | $16,0 \times 140$ | SW24 | 20 |
| 946190 | $16,0 \times 180$ | SW24 | 10 |
| acc. to DN 440: |  |  |  |
| 946191 | 12,0 $\times 200$ | SW19 | 20 |
| 946192 | $12,0 \times 220$ | SW19 | 20 |
| 946193 | $12,0 \times 240$ | Sw19 | 15 |
| 946194 | $12,0 \times 260$ | SW19 | 15 |
| 946195 | $16,0 \times 220$ | SW24 | 10 |
| 946196 | $16,0 \times 240$ | SW24 | 10 |
| 94619 | $16,0 \times 260$ | SW24 | 10 |


| Bolt anchor, galvanised steel C3 | N $=17$ | Art. no. | Dimensions [mm] | Spanner gap | PU |
| :---: | :---: | :---: | :---: | :---: | :---: |
| With washer, galvanised steel C 3 , for cracked concrete and non-cracked concrete | to our product range | 946227* | 8,0x50 | SW13 | 100 |
|  |  | 94622 | 8, $\times 75$ | SWI3 | 100 |
|  |  | 94629 | 8,0×80 | SW13 | 100 |
|  | 为 | 946230 | 8,0×95 | SWI3 | 100 |
|  |  | 946331 | 8,0x 115 | SWl3 | 100 |
|  |  | 946332 | $10,0 \times 90$ | SW17 | 100 |
|  |  | 94623 | 10,0 $\times 105$ | SW17 | 50 |
|  |  | 946234 | 10,0x 115 | SW17 | 50 |
|  |  | 946235 | 10,0 $\times 135$ | SW17 | 50 |
|  |  | 946236 | $10,0 \times 165$ | SW17 | 50 |
|  |  | 94623 | 10,0× 185 | SW17 | 50 |
|  |  | 946238* | $12,0 \times 80$ | SW19 | 50 |
|  |  | 946239 | $12,0 \times 100$ | SW19 | 50 |
|  |  | 94624 | 12,0x 110 | SW19 | 50 |
|  |  | 946241 | 12,0 $\times 120$ | SW19 | 50 |
|  |  | 946242 | $12,0 \times 130$ | SW19 | 50 |
|  |  | 94623 | $12,0 \times 150$ | SW19 | 50 |
|  |  | 94624 | $12,0 \times 180$ | SW19 | 50 |
|  |  | 94624 | $12,0 \times 200$ | SW19 | 50 |
|  |  | 946246 | $12,0 \times 220$ | SW19 | 25 |
|  |  | 94624 | 12,0 $\times 255$ | SW19 | 25 |
|  |  | 94628 | $16,0 \times 145$ | SW24 | 25 |
|  |  | 946249 | $16,0 \times 175$ | SW24 | 25 |
|  |  | 946250 | $16,0 \times 220$ | SW24 | 25 |
|  |  | 946251 | $16,0 \times 250$ | SW24 | 25 |
|  |  | 94625 | 20,0 $\times 170$ | SW30 | 20 |
|  |  | 94625 | 20,0 $\times 200$ | SW30 | 20 |
|  |  | *Screws not regulated by ETA-22/0451 |  |  |  |



## TECHNICAL INFORMATION



| $\begin{gathered} \begin{array}{c} \text { Dimensions } \\ {[\mathrm{mm}]} \end{array} \\ \emptyset \times \text { Length } \end{gathered}$ | Min. Subsurface thickness $\mathrm{h}_{\text {min }}$ [mm] | Drill diameter $d_{0}[\mathrm{~mm}]$ | Min. Depth of drill hole $h_{1}$ [mm] | Min. Depth of drill hole $h_{\text {ef }}$ [mm] | Max. Drill diameter in attached part $d_{f}$ [mm] | $\begin{gathered} \text { Max. } \\ \text { altachment thickness } \\ \mathrm{t}_{\text {fix }}[\mathrm{mm}] \end{gathered}$ | Installation torque $\mathrm{T}_{\text {inst }}[\mathrm{Nm}]$ |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Bolt anchor with washer according to DIN 125A |  |  |  |  |  |  |  |
| 6,0× $55^{*}$ | 100 | 6 | 50 | 35 | 7 | 5 | 11 |
| $6,0 \times 85^{*}$ | 100 | 6 | 50 | 35 | 7 | 35 | 11 |
| $8,0 \times 50 *$ | 100 | 8 | 55 | 30 | 9 | 5 | 15 |
| 8,0x75 | 100 | 8 | 55 | 40 | 9 | 15 | 15 |
| $8,0 \times 95$ | 100 | 8 | 55 | 40 | 9 | 35 | 15 |
| $8,0 \times 115$ | 100 | 8 | 55 | 40 | 9 | 55 | 15 |
| $8,0 \times 135$ | 100 | 8 | 55 | 40 | 9 | 75 | 15 |
| $10,0 \times 60^{*}$ | 100 | 10 | 65 | 30 | 12 | 5 | 25 |
| $10,0 \times 80$ | 100 | 10 | 65 | 50 | 12 | 5 | 25 |
| $10,0 \times 100$ | 100 | 10 | 65 | 50 | 12 | 25 | 25 |
| $10,0 \times 120$ | 100 | 10 | 65 | 50 | 12 | 45 | 25 |
| $10,0 \times 140$ | 100 | 10 | 65 | 50 | 12 | 65 | 25 |
| $12,0 \times 80 *$ | 110 | 12 | 80 | 50 | 14 | 5 | 40 |
| 12,0× 95 | 110 | 12 | 80 | 65 | 14 | 5 | 40 |
| $12,0 \times 110$ | 110 | 12 | 80 | 65 | 14 | 20 | 40 |
| $12,0 \times 130$ | 110 | 12 | 80 | 65 | 14 | 40 | 40 |
| $12,0 \times 160$ | 110 | 12 | 80 | 65 | 14 | 70 | 40 |
| $12,0 \times 180$ | 110 | 12 | 80 | 65 | 14 | 90 | 40 |
| $16,0 \times 125$ | 120 | 16 | 90 | 80 | 18 | 15 | 80 |
| $16,0 \times 140$ | 120 | 16 | 90 | 80 | 18 | 30 | 80 |
| $16,0 \times 180$ | 120 | 16 | 90 | 80 | 18 | 70 | 80 |
| Bolt anchor with washer according to DIN 440 |  |  |  |  |  |  |  |
| $12,0 \times 200$ | 110 | 12 | 80 | 65 | 14 | 110 | 40 |
| $12,0 \times 220$ | 110 | 12 | 80 | 65 | 14 | 130 | 40 |
| $12,0 \times 240$ | 110 | 12 | 80 | 65 | 14 | 150 | 40 |
| $12,0 \times 260$ | 110 | 12 | 80 | 65 | 14 | 170 | 40 |
| $16,0 \times 220$ | 120 | 16 | 90 | 80 | 18 | 110 | 80 |
| $16,0 \times 240$ | 120 | 16 | 90 | 80 | 18 | 130 | 80 |
| $16,0 \times 260$ | 120 | 16 | 90 | 80 | 18 | 150 | 80 |
| Bolt anchor A4 |  |  |  |  |  |  |  |
| $8,0 \times 75$ | 100 | 8 | 60 | 45 | 9 | 15 | 20 |
| $8,0 \times 100$ | 100 | 8 | 60 | 45 | 9 | 40 | 20 |
| $10,0 \times 100$ | 120 | 10 | 75 | 60 | 12 | 25 | 45 |
| $10,0 \times 120$ | 120 | 10 | 75 | 60 | 12 | 45 | 45 |
| $10,0 \times 140$ | 120 | 10 | 75 | 60 | 12 | 65 | 45 |
| $12,0 \times 140$ | 140 | 12 | 85 | 70 | 14 | 50 | 60 |

[^13]| Dimensions [mm] | Min. Subsurface thickness $h_{\text {min }}$ [mm] | Drill diameter $d_{0}[\mathrm{~mm}]$ | Min. Depth of drill hole $h_{1}$ [mm] | Min. Depth of drill hole hef [mm] | Max. Drill diameter in attached part df [mm] | $\begin{gathered} \text { Max. } \\ \text { attachment thickness } \\ \mathrm{t}_{\text {fix }}[\mathrm{mm}] \end{gathered}$ | Installation torque $\mathrm{T}_{\text {inst }}[\mathrm{Nm}]$ |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| $0 \times$ Lenght |  |  |  |  |  |  |  |
| Bolt antho, gavanaised steel ${ }^{\text {3 }}$ |  |  |  |  |  |  |  |
| $8,0 \times 50^{*}$ | 100 | 8 | 40 | 30 | 9 | 2 | 15 |
| $8,0 \times 75$ | 100 | 8 | 60 | 48 | 9 | 9 | 15 |
| $8,0 \times 80$ | 100 | 8 | 60 | 48 | 9 | 14 | 15 |
| $8,0 \times 95$ | 100 | 8 | 60 | 48 | 9 | 29 | 15 |
| $8,0 \times 115$ | 100 | 8 | 60 | 48 | 9 | 49 | 15 |
| $10,0 \times 90$ | 120 | 10 | 75 | 60 | 12 | 10 | 40 |
| $10,0 \times 105$ | 120 | 10 | 75 | 60 | 12 | 25 | 40 |
| $10,0 \times 115$ | 120 | 10 | 75 | 60 | 12 | 35 | 40 |
| 10,0x 135 | 120 | 10 | 75 | 60 | 12 | 55 | 40 |
| $10,0 \times 165$ | 120 | 10 | 75 | 60 | 12 | 85 | 40 |
| $10,0 \times 185$ | 120 | 10 | 75 | 60 | 12 | 105 | 40 |
| $12,0 \times 80^{*}$ | 140 | 12 | 65 | 50 | 14 | 4 | 60 |
| 12,0x 100 | 140 | 12 | 85 | 70 | 14 | 4 | 60 |
| $12,0 \times 110$ | 140 | 12 | 85 | 70 | 14 | 14 | 60 |
| $12,0 \times 120$ | 140 | 12 | 85 | 70 | 14 | 24 | 60 |
| $12,0 \times 130$ | 140 | 12 | 85 | 70 | 14 | 34 | 60 |
| $12,0 \times 150$ | 140 | 12 | 85 | 70 | 14 | 54 | 60 |
| $12,0 \times 180$ | 140 | 12 | 85 | 70 | 14 | 84 | 60 |
| $12,0 \times 200$ | 140 | 12 | 85 | 70 | 14 | 104 | 60 |
| 12,0 $\times 220$ | 140 | 12 | 85 | 70 | 14 | 124 | 60 |
| $12,0 \times 255$ | 140 | 12 | 85 | 70 | 14 | 159 | 60 |
| $16,0 \times 145$ | 170 | 14 | 105 | 85 | 18 | 28 | 100 |
| $16,0 \times 175$ | 170 | 14 | 105 | 85 | 18 | 58 | 100 |
| $16,0 \times 220$ | 170 | 14 | 105 | 85 | 18 | 103 | 100 |
| $16,0 \times 250$ | 170 | 14 | 105 | 85 | 18 | 133 | 100 |
| $20,0 \times 170$ | 200 | 20 | 125 | 100 | 22 | 32 | 200 |
| $20,0 \times 200$ | 200 | 20 | 125 | 100 | 22 | 62 | 200 |
| *Not reglucted by EA-22/0451 |  |  |  |  |  |  |  |

## SILENT EPDM DECOUPLING PROFILE

For sound insulation and material separation

The decoupling profile is used for sound insulation and material separation in timber and solid timber construction. The decoupling strip serves as a sound-insulating profile striip between timber parts and ensures physical and mechanical separation of adioining components. As a result, it prevents the transfer of vibration from fooffall/structure-borne sound.

Silent EPDM decoupling profile
Material: SK02


| Art. no. | Thickness $[\mathrm{mm}]$ | Width $[\mathrm{mm}]$ | Lenght $[\mathrm{mm}]$ | Color | Material | PU |
| :--- | :--- | :--- | :--- | :--- | :--- | ---: |
| 945382 | 5 | 95 | 20 | Black | EPDM | 1 |

ADVANTAGES

- Versatile applications
- Can be individually cut to size (supplied as a roll)
- Ageing-resistant
- UV-stable
- Ozone-resistant
- Free of conflict materials


## PROPERTIES

- Density: approx. $1,4 \mathrm{~g} / \mathrm{cm}^{3}$
- Usage temperature: $-30^{\circ} \mathrm{C}-+90^{\circ} \mathrm{C}$
- Shore hardness $48=0,500 \mathrm{~N} / \mathrm{mm}^{2}=0,05 \mathrm{kN} / \mathrm{m}^{2}$


## INSTRUCTIONS FOR USE

Cut the decoupling profile to the desired length and place it in the chosen position, then fasten it in place at intervals of approx. $40-60 \mathrm{~cm}$, for example using the Eurotec Hammer tacker.


| Material properties |  |  |  |
| :---: | :---: | :---: | :---: |
| Property | Measurement method | Unit | Value |
| Hardness | DIW 1507619-1 | Shore A | 48 |
| Density | DII 53479 | $\mathrm{g} / \mathrm{m}^{3}$ | 1,23 |
| Tearstrengh | DIIN5354 | MPa | 8,5 |
| Elongation of brak | DIIN5354 | \% | 510 |
| Compression set | DIW 150815-1 | \% | $\leq 40$ |
| Temperature resistance |  | ${ }^{\circ} \mathrm{C}$ | $30 / 100^{\circ} \mathrm{C}$ |

[^14]

## ECKTEC

The space-saving alternative to the conventional brace

The EckTec connector can replace the conventional brace. This allows a better look without disruptive braces, especially
 at low installation heights.

Ecktec


| Art. no. | Dimensions $[\mathrm{mm}]^{\text {a) }}$ | PU* |
| :--- | :--- | ---: |
| 975664 | $50 \times 50 \times 100$ | 1 |
| a) Width $\times$ Height $\times$ Depth <br> *Delivery incl. screws |  |  |
| ADVANTAGES |  |  |
| - Supports load absorption with horizontal forces |  |  |
| - Pre-assembly at the factory optional |  |  |
| - Many different areas of use |  |  |

## INSTRUCTIONS FOR USE

The EckTec connector is fixed with two $4 \times 40$ Paneltwistecs. The first KonstruX ST $8 \times 155$ fully-threaded screws are then set at $25^{\circ}$ in the posts. After mounting the cross beam, the other $8 \times 95$ KonstruX ST fully threaded screws can be set at $90^{\circ}$. Min. cross-section of beam: $120 \times 120 \mathrm{~mm}$




SPECIAL COMPONENTS

| Individual solutions for complex constructions | 170 |
| :--- | :--- |
| Special components | 171 |
| Module connectors | 172 |

## Gurotec

## INDIVIDUAL SOLUTIONS FOR COMPLEX CONSTRUCTIONS

Your construction site is a bit more complex and you are missing the perfect connector for special tasks? NO PROBLEM!

On request, we manufacrute individual components, adapted to your needs, so that you can build worry-free!
Due to the ever-increasing popularity of wood as a building material in terms of environmental protection and cross laminated timber explicitly in building construction, we have increasingly focused on the topic of fastening and anchoring of prefabricated timber elements.

In this context, the efficiency as well as the quality of the products from the complex field of timber engineering is in the foreground. The core of this demanding architecture consists of complicated shapes, enormous spans of the structures as well as high static challenges.


For our customers we are able to develop and manufacture unique solutions in these areas of modular construction. These include hall structures for industry, trade and agriculture; but also bridges or more complex roof structures.


## SPECIAL COMPONENTS

We offer customized solutions for your projects. From floor anchor plates with cross bracing in heavy timber construction connected by steel cables to cross flat connectors for heavily loaded timber connections with individual hole patterns.

Optimal load distribution thanks to individual adaptations to your projects
Better utilization of the individual connectors, for highly stressed junctions in engineered timber construction


## Gurotec

## EUROTEC MODULE CONNECTORS

Our products include shearing angles, shearing plates, tie rods and tension straps. These are used for anchoring walls, columns and ceilings.

The special features of shearing angles are the different installation heights and the type of perforation, depending on the application.

In order to secure aligned components against shear forces, we also developed the shearing plate, which can be used in a variety of ways to cover all possible anchoring cases.

In our product range you will find several variants of the tension straps. They can be used to create timber-timber, timber-concrete and steel-steel connections. Special holes for bolting at an angle of $45^{\circ}$ make the tension straps particularly efficient and unique.

The Eurotec tie bar is used to absorb tensile forces to enable simple and fast base point anchoring of timber elements in timber, steel or concrete substrates.


## CONDITIONS OF SALE AND DELIVERY

All sales to buyers, customers and contract partners, hereinafter referred to as customers, are made exclusively subject to the following terms and conditions unless other agreements are made in writing in the individual case:

## 1. SCOPE, GENERAL PROVISIONS

Our terms and conditions shall apply exclusively! We will not accept contradictory terms and conditions of our customers that deviate from our conditions unless we have given our express written consent to their validity. Our terms and conditions shall apply even if we execute orders without reservation despite being aware of contradictory conditions or conditions that deviate from our terms and conditions. Our terms and conditions shall also apply to all future transactions with our customers. Customers can access the latest version of these Standard Terms and Conditions at www.eurotec.team at any time.

## 2. OFFERS, WRITTEN FORM

Our offers are non-binding and subject to alteration without notice until we issue our final order confirmation. Contracts and agreements, as well as transactions brokered by our representatives, shall become binding only when we issue our written order confirmation. Verbal agreements, even within the framework of contract execution, are not valid unless confirmed by us in writing.

## 3. PRICES, PACKAGING, OFFSETTING

Unless otherwise indicated by the order confirmation, our prices are ex-works and exclusive of packaging. This is billed separately. The minimum order value is $£ 50.00$. For smaller quantities, we charge a flat processing fee of $€ 30.00$.
a) Our prices are exclusive of statutory value added tax. This is stated and charged separately in the invoice at the statutory rate applicable on the date of billing.
b) Our customer may only claim a right of offsetting insofar as counterclaims are established to be legally binding or are undisputed or accepted. A right of retention may only be exercised with respect to counterclaims resulting from the same contractual relationship.

## 4. DELIVERY, DELIVERY PERIOD AND FORCE MAJEURE

Unless otherwise agreed in writing, the place of performance shall be our company premises. The goods are shipped at the customer's risk and expense by third parties acting on our behalf. From the time at which the goods are made ready for delivery and the customer has been informed of their readiness for shipping, the customer shall bear the risk of accidental loss or deterioration of the item. This shall apply even if shipping is delayed as a result of circumstances for which we are not responsible. Punctual handing over of the goods to a shipping company requires that the order be placed on time by our customer. If the goods are handed over to the appointed shipping company punctually, we will not be liable for delayed delivery to the customer. This shall apply even if a delivery deadline was agreed with the customer, especially in the case of delivery to a construction site. The customer may be exempted from rush charges incurred in relation to this if there is a legal basis for deducting this surcharge from the forwarder's bill.
Statements relating to delivery periods are always to be seen only as approximate and non-binding. They shall begin on the date of our order confirmation but not before all of the order details are clarified in full. They refer to the time of consignment ex-works and shall be considered met when the goods are reported to be ready for dispatch. Without prejudice to our rights arising due to the customer's default, they shall be extended by the period for which the customer is in arrears to us with respect to their obligations arising from this or other orders.
Even if they arise at our suppliers, the following grounds are among those that shall release us from the obligation to adhere to the delivery period and shall entitle us to extend the delivery periods, to make partial deliveries or to wholly or partially withdraw from the part of the contract that is not yet fuffilled without becoming liable to pay damages as a result, unless we are guilty of intent or gross negligence: interruptions of operations and difficulties in delivery of any kind, e. g. shortages of machinery, goods, materials or fuels, or incidents of force majeure, e. g. export and import embargos, fires, strikes, lock-outs or new official measures that adversely affect production costs and shipping.

## 5. SHIPPING

Goods are shipped at the expense and risk of the customer even if prepaid delivery was agreed. Additional costs for express shipping shall always be borne by the customer. Freight costs paid by us are to be seen only as an advancement of freight charges on behalf of the customer. Additional freight costs for urgent and express parcels shall be borne by the customer, even if we have borne the transport costs on individual occasions. Goods reported as ready for shipping must be accepted immediately and will be charged as exworks. If the goods are to be shipped abroad or passed directly to third parties, they must be examined and accepted in our factory; otherwise, the goods shall be deemed to have been delivered in accordance with the contract to the exclusion of any complaints. The risk, including that of confiscation, shall be transferred to the customer when the goods are handed over to the forwarder or freight carrier and, at the latest, when they leave our facility. Return shipments always require prior consultation with our internal sales depariment. Goods that are free of defects are only taken back with our express consent. A credit note is then issued for the value of the goods with deduction of a $25 \%$ return fee per item or against a minimum fee of $€ 50$ for returning the goods to storage. Strictly no debit notes are accepted.
6. DESIGN AND PROPERTY RIGHTS

The customer shall bear sole responsibility and be liable for ensuring that the goods it orders do not violate thirdparty property rights. No verification is performed on our part in this respect. The customer shall indemnify us against injunctions or claims for damages by third parties. If an iniunction is requested against us, the customer shall meet the legal costs and shall compensate us for the damages we have incurred.

## 7. ACCEPTANCE, QUANTITY TOLERANCES AND CALL-OFFS

For contracts with ongoing deliveries, the goods are to be accepted in monthly quantities that are as consistent as possible over the course of the contractual period. If a call-off is not made on time, we shall be entitled, after the expiry of a grace period that we have granted, to divide the order at our own discretion, withdraw from the part of the contract that has not yet been executed, or make a claim for damages due to non-performance. In the case of call-off orders, the call-offs must always be made within 12 calendar months. Over- or under-shipment by up to $10 \%$ of the order shall be permissible.

### 8.1 PAYMENT TERMS FOR INVOICES, RIGHT OF RETENTION

Invoices shall be payable with a $2 \%$ discount within 10 days of the invoice date or net within 30 days, regardless of when the goods are received and without prejudice to the right to make a complaint for defects. Payment by means of acceptance or customer's bill of exchange shall require special written agreement in advance. Discount charges will be charged in the case of payment by means of acceptance, which must have a term no longer than 3 months and be issued within 1 week of the invoice date. Credit notes for bills of exchange or cheques shall apply subject to receipt and regardless of the purchase price's earlier due date in the event of default by the customer. They shall be issued with the value at the date on which the equivalent amount will be available to us; the discount charges will be charged at the respective bank rate. In the event that the payment term is exceeded, interest and commissions
may be charged without prejudice to other rights at the respective bank rate for overdrafts but at a rate at least $5 \%$ above the respective discount rate of the Deutsche Bundesbank [German Federal Bank]. If the payment terms are not adhered to or we become aware of circumstances that, in our view, are sufficient to reduce the customer's credit worthiness, all of our claims shall become payable immediately regardless of the term of any bills of exchange that have been accepted or credited.
We shall then also be entitled to perform outstanding deliveries only in exchange for advance payment, to withdraw from the contract after a reasonable grace period, and to demand compensation for default. We may also prohibit the resale or processing of the delivered goods and demand their return or the transfer of indirect possession of the delivered goods at the customer's expense. The customer hereby already authorises us to enter its premises and confiscate the delivered goods in the above cases. We shall be entitled to the usual securities for our claims according to their nature and extent, even if they are subbect to conditions or of limited duration. Offsetting or withholding payments as a result of any counterclaims or notifications of defects shall be prohibited, except where claims are undisputed or established to be legally binding.

### 8.2 TERMS OF PAYMENT FOR WEB-SHOP CUSTOMERS

Payment shall be made exclusively in advance. Once the order process in our online shop is complete, you will receive an email with the bank details for our business account. The invoiced amount must be transferred to our account within 7 days. We cannot carry out your order until the payment arrives.

## 9. RETENTION OF TITLE

Until all liabilities arising from the business relationship are paid in full and, in particular, until all bills of exchange and cheques, including finance bills, given as payment are cashed, the goods delivered by us shall remain our property and may be taken back by us at the customer's expense in the event of default in payment. Until this point, the customer shall not be entitled to pledge or assign the goods to third parties as a security; it may sell them on or process them only within the framework of its ongoing business transactions. The customer shall be obliged to inform us immediately of any seizure by third parties of the goods delivered subject to retention of fitle.
In the event of further processing, the customer shall not acquire ownership of the goods delivered by us as set out in section 950 of the German Civil Code (BGB), as any processing is carried out by the customer on our behalf. Without prejudice to the rights of third-party suppliers, the newly created thing shall serve as security for us up to the amount of our total claims arising from the business relationship. It shall be kept safe for us by the customer and shall be regarded as goods for the purpose of these terms and conditions. If the item is intermixed or otherwise combined with other objects that to do not belong to us, we shall acquire at least co-ownership of the new thing in proportion to the value of the contract item to that of other objects that have been processed with it. If the customer sells the goods delivered by us, regardless of their condition, it hereby already assigns to us all claims against its customers arising from sales, as well as all ancillary rights, until all of our claims arising from delivery of goods are paid in full. At our request, the customer shall be obliged to notify its downstream customers of the assignment and to hand over the information and documents we require in order to assert our rights against its downstream customers.
If the total value of the securities given to us exceeds our claims arising from delivery by more than $20 \%$, we shall be obliged to retransfer securities to this extent at the customer's request. If the retention of fitle or assignment is invalid in the territory in which the goods are located, a security corresponding to the retention of title or assignment in this territory shall be deemed to be agreed. If the customer's cooperation is required in this process, it shall take all necessary measures to establish such rights.

## 10. NOTIFICATION OF DEFECTS, LIABILITY

Our customer shall be entitled to a warranty only if they have properly fulfilled their legal obligations under sections 377 and 378 of the German Commercial Code (HGB) with respect to the duties of examination and notification. If defects are present, we shall be entitled at our choice to either repair the defects or provide a replacement; if we are not prepared or not able to do so, and especially if repair/replacement is delayed beyond reasonable deadlines for reasons that we are responsible for, or if repair/replacement otherwise fails, our customer shall be entitled at its choice to withdraw from the contract or to demand a corresponding reduction in the price. Unless otherwise stipulated below, further claims of the customer shall be excluded regardless of their legal basis. We shall not be liable for damage that did not occur to the delivered item itself. In particular, we shall not be liable for lost profit or other pecuniary losses of the customer. The above exemption from liability shall not apply if the damage is caused by intent or gross negligence; it shall also not apply if the customer asserts claims for damages for non-performance due to the lack of a warranted characteristic. If we breach an essential contractual duty through negligence, our duty of reimbursement for property damage or personal injury shall be restricted to the level of cover provided by our product liability insurance.
We are prepared to allow the customer to view our policy. The warranty period is 6 months calculated from the date of transfer of risk. This period is a limitation period. The period shall also apply to claims under sections 1 and 4 of the German Product Liability Act (ProdHaftG). Insofar as our liability is excluded or restricted, this shall also apply to the personal liability of our employees, workers, staff, representatives and agents. Goods that are subject to a complaint must not be sent back without obtaining our prior written consent, as otherwise we may refuse to accept them at the sender's expense. Goods that have been partially or wholly processed will not be taken back under any circumstances. The customer is obliged to make sure that the purchased product is suitable for the intended application using technical descriptions, where available, and based on their specialist knowledge and to familiarise themselves with the application of this product. If they are not familiar with the product's application, our company staff are available to provide advice. All information and advice from our staff is provided carefully and conscientiously. Under no circumstances does this information and advice replace the indispensable consultancy services of architects and specialist planning companies or the services they provide during construction. Only the authorised professional groups are entitled to provide these services.

## 11. PLACE OF PERFORMANCE AND JURISDICTION, MISCELLANEOUS

Our company's registered office shall be the place of performance for all obligations arising from this contract, including liabilities from cheques and bills of exchange. Provided our customer is a merchant, the place of jurisdiction for all disputes arising from the contractual relationship shall be, at our choice, the Local Court of Hagen. Contracts with our customer shall be governed exclusively by German law to the exclusion of the UN Convention on Contracts for the International Sale of Goods of 11 April 1980. The language of the contract shall be German.

Hagen, 16. February 2018
E.,.r.o. Tec GmbH

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Gurotec | Construction with CLT

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## E.u.r.o.Tec GmbH

Unter dem Hofe 5- D-58099 Hagen


[^0]:    Suitable for use with:
    Shearing angle, Shearing plate, Shearing angle HB flat, Shrearing angle HH flat, Tension strap HB / HH

[^1]:    Suitable for use with:
    KonstruX, Angle-bracket screw
    Paneltwistec, Rock concrete screw,
    EST dowel bar, Dowel bar

[^2]:    Calculation according to EAA- $11 / 0024$. Wood density $\mathrm{pk}=380 \mathrm{~kg} / \mathrm{m}^{3}$. All mechanical values provided should be viewed as subject to the assumptions that have been made and represent example calculations.
    All values are calculated minimum values and are subject to typographical and printing errors.
    a) The characterisic values of the load bearing capacity Rk cannot be treated as equivalent to the max. possible load (the max. force). Characteristic values of the load-bearing capacity Rk should be reduced to dimensioning valves
    $R_{d}$ with regard to the usage class and class of the load duration: $\mathrm{Rd}=\mathrm{Rk}_{\mathrm{k}} \cdot \mathrm{Kmod}^{\mathrm{m}} / \gamma_{\mathrm{M}}$. The dimensioning values of the lood-bearing capacity Rd should be contrasted with the dimensioning values of the loads $\left(\mathrm{R}_{\mathrm{d}} \geq \mathrm{E}_{\mathrm{d}}\right)$.

[^3]:    Calculction according to EA-11/024. Wood density $\mathrm{pk}=380 \mathrm{~kg} / \mathrm{m}^{3}$. All mechanical values provided should be vieved as subject to the assumpions that have been made and represent example calulutions.
    All values sre calculcted minimum values and dre subjedt to typographical and p pirining errors.
    
    
    
    
    Plesse note: These cre planning iids. Projets must only be calculated by uuthorised pessons.

[^4]:    Calculation according to EAA- $11 / 0024$. Wood density $\rho \mathrm{pk}=380 \mathrm{~kg} / \mathrm{m}^{3}$. All mechanical values provided should be viewed as subject to the assumptions that have been made and represent example calculations.
    All values are calculcted minimum values and are subject to typographical and printing errors.
    a) The characteristic values of the load-bearing capacity Rk cannot be treated as equivalent to the max. possible load (the max. force). Characterisicic values of the load-bearing capacity Rk should be reduced to dimensioning valves Rd
    

[^5]:    Calculation according to $\operatorname{ETA}-11 / 0024$. Wood density $\rho \mathrm{\rho k}=350 \mathrm{~kg} / \mathrm{m}^{3}$. All mechanical values provided should be viewed as subject to the assumptions that have been made and represent example calculations.
    All values are calculated minimum values. Typesetting and printing errors are excepted.
    a) The characterisicic values of the lood-bearing capacity Rh should not be treated as equivalent to the max. possible load (the max. force). Characteristic valves of the load-bearing capacity Rk are to be reduced to the design values
    $R_{d}$ as regards the service class and class of the load duration: $\mathbb{R d}_{d}=R_{k} \cdot K_{m o d} / \gamma m$. The design values of the load-bearing capacity $R_{d}$ should be compared to the design values of the loads $\left(\mathbb{R}_{d} \geq E_{d}\right)$.

[^6]:    Calculation according to ETA- $\mathrm{II} / 0024$. Wood density $\rho_{\mathrm{k}}=350 \mathrm{~kg} / \mathrm{m}^{3}$. All mechanical values provided should be viewed as subject to the assumptions that have been made and represent example calculations.
    All values are calculated minimum values and are subject to typographical and printing errors.
    a) The characteristic valves of the load-bearing capacity Rk cannot be treated as equivalent to the max. possible load (the max. force). Characterisicic values of the load-bearing capacity Rk should be reduced to dimensioning values Rd

[^7]:    a) Ouantity dways refers to the less favourable value from Topduo Washer head and Cylinder-head
    b) Topduo Washer head only, c) Includes snow load zones 1,2 and $2^{*}$ each with snow guard, d) Includes all wind zones apart from North Sea islands
    e) Use of our proiect assessment service is recommended. The design examples listed here represent unfavourable, i.e. statically safe, instances.
    f) Incudedes snow lood zones 1,2 and 3, g) Includes wind zones 1 and 2 (inland)

[^8]:    *only for compression-proof insulations with compression strength $\geq 50 \mathrm{kPa}$ **also for non-compression-proof insulations

[^9]:    Please note: Verity the assumptions made. The stated values, and type and number of joining devices are based on preliminary measurements. Projects are to be dimensioned exclusively by authorised persons in accordance with the State Building Code. As per LBauO, please contact a qualified structural engineer for a paid proof of stability. We will be happy to refer you to someone.

[^10]:    de is the diameter and the total height of the connector
    ag is the metric connection thread of the connector
    V cis the height of the integrated anti-wwist system - Fully threaded screw, GoFix ${ }^{\circledR}$ FK
    IF $305,0 \times 40 \mathrm{~mm} \cdot$ IF $406,0 \times 60 \mathrm{~mm} \cdot$ IF $508,0 \times 90 \mathrm{~mm}$
    The connection is drawn together using a threaded rod or constructionscrew with a DIN 440 R washer
    MB-SB connection as a mortise joint with simultaneous absorption of tensile forces
    Rk characteristic value calculated according to DIN $1052: 2004-08$ Timber pk $380 \mathrm{~kg} / \mathrm{m}^{3} \mathrm{Nze}$. recommended permissible lood $\mathrm{R}, \mathrm{k} \times 0,8 \mathrm{kmod}: 1,3 \mathrm{ym}: 1,4$. Favtor 1,4 average load safery factor
    Please note: The stated values are planning ciids. Projects must only be calculated by authorised persons.

[^11]:    
    
    
    
    d $\gamma$ - Inctindion angle of line (chain, rope, lititing strap etic.) ; oflesst. $60^{\circ}$ according to BGR 500
    d) $\alpha$ - Angle between grain direction ond screwing oxis

    Please not: These are planning aids. Proiects must only be calculced by outhorised persons.

[^12]:    CLT system angle for fixing a wall to the wooden floor of the upper level.

[^13]:    *Not regulated by ETA-14/0409
    Please note: Verify the assumptions made. The stated values, and type and number of joining devices are based on preliminary measurements. Projects are to be dimensioned exclusively by authorised
    persons in accordance with the State Building Code. As per LBuoO, please contact a qualified structural engineer for a paid proof of stability. We will be happy to refer you to someone.

[^14]:    Please note: Verify the assumptions made. The stated values, and type and number of joining devices are bassed on preliminary measurements. Projects are to be dimensioned exdusively by authorised persons in accordance with the State Building Code. As per LBauO, please contact a qualified structural engineer for a paid proof of stability. We will be happy to refer you to someone.

